

D.07_FASCICOLO DEI CALCOLI D

Ubicazione: Località CASACCE, Comune di MOLLIA (VC)
Provincia di VERCELLI (Regione PIEMONTE)

Progetto: Mollia_MLL

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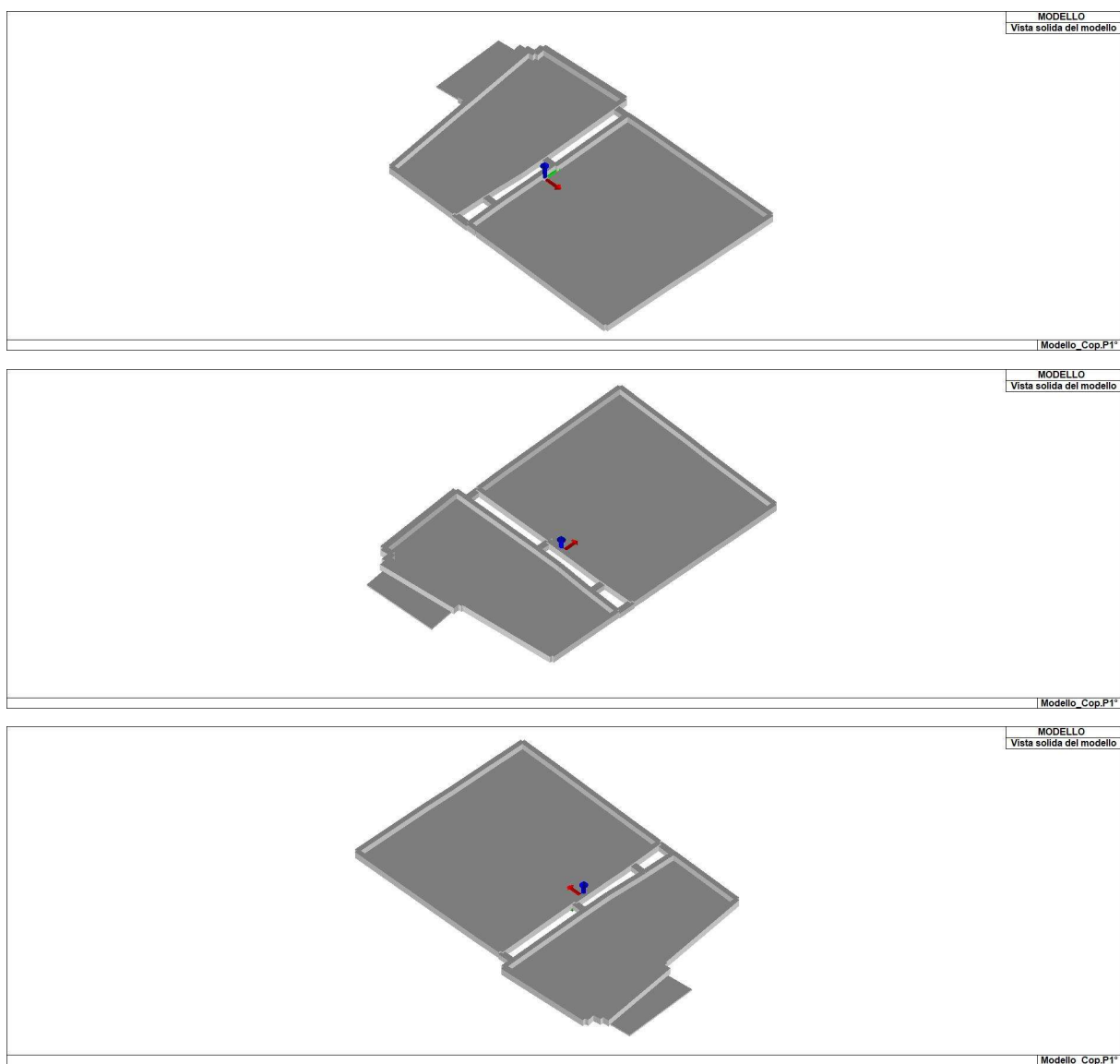
PREMESSA

Nel presente Fascicolo dei calcoli viene presentato il progetto e la verifica delle soluzioni adottate per gli interventi locali di rifacimento del solaio di copertura del piano primo.

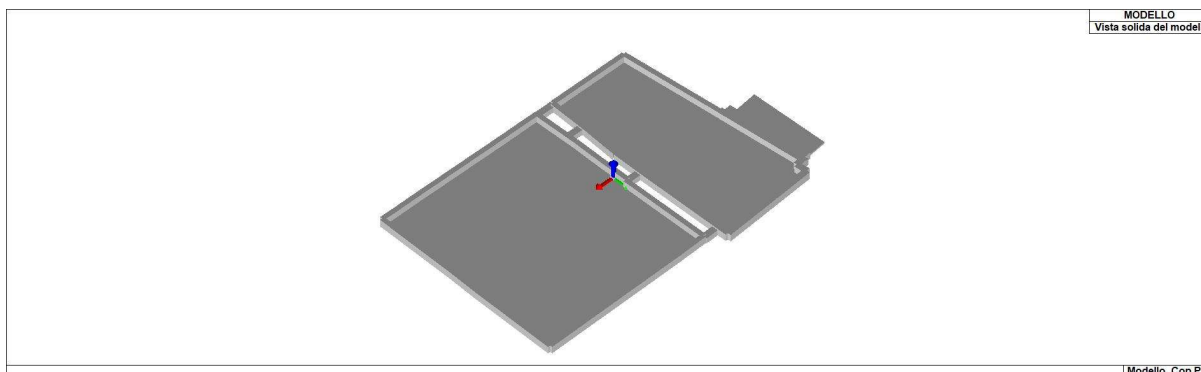
In particolare si è ipotizzato di realizzare un solaio in tecnica mista legno-calcestruzzo.

Dello stesso si riportano nel seguito le relative verifiche:

VERIFICHE DEI CORDOLI/TRAVI IN C.A.



INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



MODELLAZIONE

L'analisi strutturale è condotta con il metodo degli spostamenti per la valutazione dello stato tenso-deformativo indotto da carichi statici. L'analisi strutturale è condotta con il metodo dell'analisi modale e dello spettro di risposta in termini di accelerazione per la valutazione dello stato tenso-deformativo indotto da carichi dinamici (tra cui quelli di tipo sismico).

L'analisi strutturale viene effettuata con il metodo degli elementi finiti. Il metodo sopraindicato si basa sulla schematizzazione della struttura in elementi connessi solo in corrispondenza di un numero prefissato di punti denominati nodi. I nodi sono definiti dalle tre coordinate cartesiane in un sistema di riferimento globale. Le incognite del problema (nell'ambito del metodo degli spostamenti) sono le componenti di spostamento dei nodi riferite al sistema di riferimento globale (traslazioni secondo X, Y, Z, rotazioni attorno X, Y, Z). La soluzione del problema si ottiene con un sistema di equazioni algebriche lineari i cui termini noti sono costituiti dai carichi agenti sulla struttura opportunamente concentrati ai nodi:

$$\mathbf{K} * \mathbf{u} = \mathbf{F} \quad \text{dove} \quad \mathbf{K} = \text{matrice di rigidezza}$$

$$\mathbf{u} = \text{vettore spostamenti nodali}$$

$$\mathbf{F} = \text{vettore forze nodali}$$

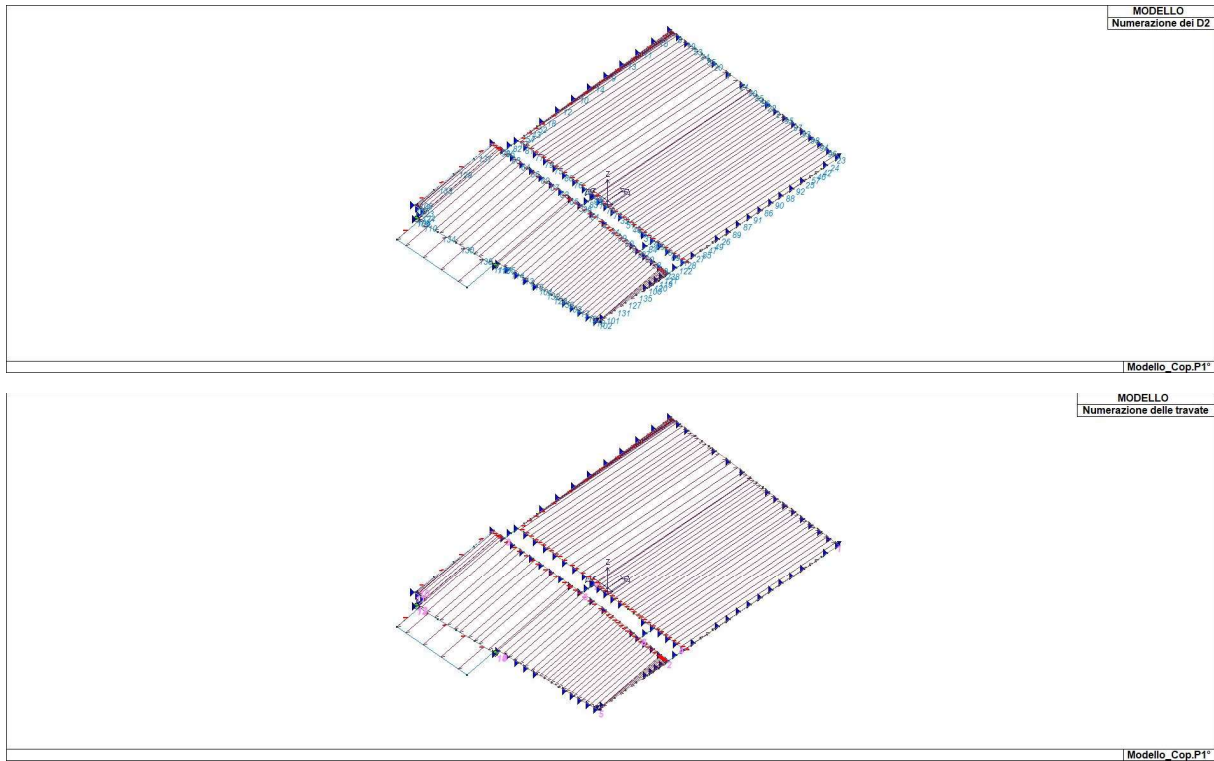
Dagli spostamenti ottenuti con la risoluzione del sistema vengono quindi dedotte le sollecitazioni e/o le tensioni di ogni elemento, riferite generalmente a una terna locale all'elemento stesso.

Il sistema di riferimento utilizzato è costituito da una terna cartesiana destrorsa XYZ. Si assume l'asse Z verticale ed orientato verso l'alto.

ELEMENTI FINITI – SEZIONI E SPESSORI

A seguire si riportano le immagini relative alle numerazioni di interesse:

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



Si riportano di seguito le caratteristiche di sezioni e spessori degli elementi strutturali, in formato tabellare e immagini:

TABELLA_SEZIONI

Id	Tipo SEZ	Area	A V2	A V3	Jt	J 2-2	J 3-3	W 2-2	W 3-3	Wp 2-2	Wp 3-3
--		cm2	cm2	cm2	cm4	cm4	cm4	cm3	cm3	cm3	cm3
1	CORDOLI_15x23 Rettangolare: b=15 h=23	345.0	287.5	287.5	1.524e+04	6468.75	1.521e+04	862.5	1322.5	1293.7	1983.7
2	SPERONI_20x18 Rettangolare: b=20 h=18	360.0	300.0	300.0	1.812e+04	1.200e+04	9720.00	1200.0	1080.0	1800.0	1620.0
3	TRAVE_40x23	920.0	766.6	766.6	1.035e+05	1.227e+05	4.056e+04	6133.3	3526.6	9200.0	5290.0

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

TABELLA_SEZIONI

Id	Tipo SEZ	Area	A V2	A V3	Jt	J 2-2	J 3-3	W 2-2	W 3-3	Wp 2-2	Wp 3-3
-	--	cm2	cm2	cm2	cm4	cm4	cm4	cm3	cm3	cm3	cm3
	Rettangolare: b=40 h=23										
4	LINK RIGIDO - Rettangolare: b=15 h=15	225.00	187.50	187.50	7116.25	4218.75	4218.75	562.50	562.50	843.75	843.75
5	CORDOLI_20 x23 Rettangolare: b=20 h=23	460.00	383.33	383.33	2.942e+04	1.533e+04	2.028e+04	1533.33	1763.33	2300.00	2645.00

Legenda

Tipo SEZ Indica il nome identificativo e la tipologia di sezione

Area Area della sezione

A V2 Area della sezione/Fattore di taglio (direzione 2)

A V3 Area della sezione/Fattore di taglio (direzione 3)

Jt Momento di inerzia torsionale della sezione

J 2-2 Momento di inerzia della sezione riferito all'Asse 2

J 3-3 Momento di inerzia della sezione riferito all'Asse 3

W 2-2 Modulo di resistenza della sezione riferito all'Asse 2

W 3-3 Modulo di resistenza della sezione riferito all'Asse 3

Wp 2-2 Modulo di resistenza plastico della sezione riferito all'Asse 2

Wp 3-3 Modulo di resistenza plastico della sezione riferito all'Asse 3

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

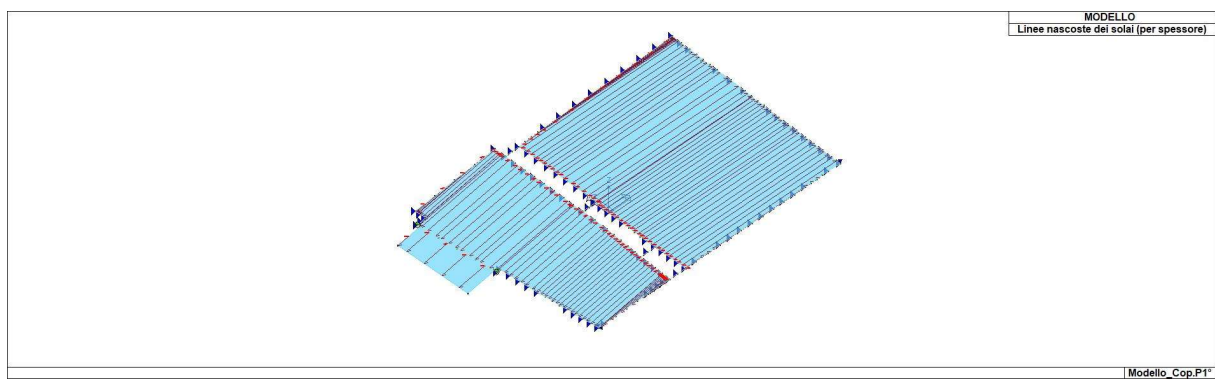
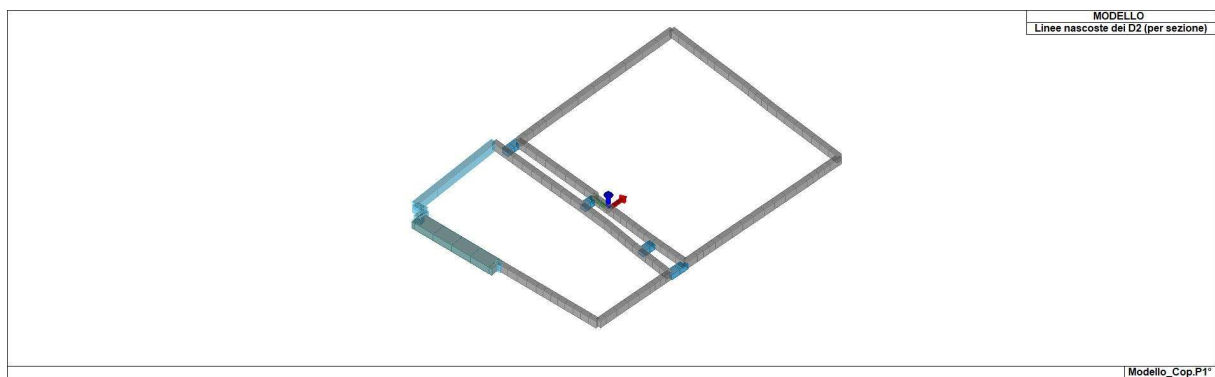
TABELLA_SPESSORI

Id	Spessore Gusci	Spessore Setti	Sp. solai piano rigido
-	cm	cm	cm
1	-	-	5.00

Legenda

Spessore Gusci Spessore degli elementi shell con sviluppo orizzontale

Spessore Setti Spessore degli elementi shell con sviluppo verticale



CARATTERISTICHE MATERIALI UTILIZZATI

Nell'esecuzione delle opere oggetto della presente relazione è previsto l'utilizzo dei seguenti materiali con le relative caratteristiche:

ELENCO DEI MATERIALI IMPIEGATI

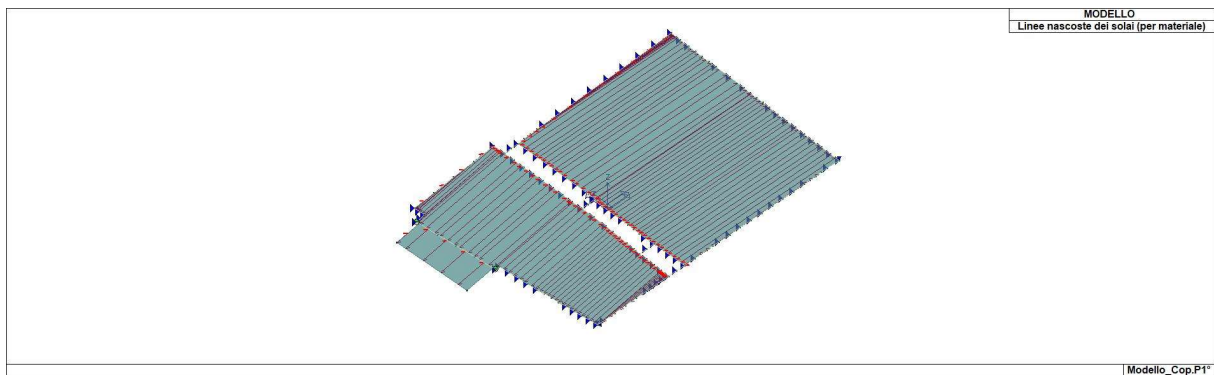
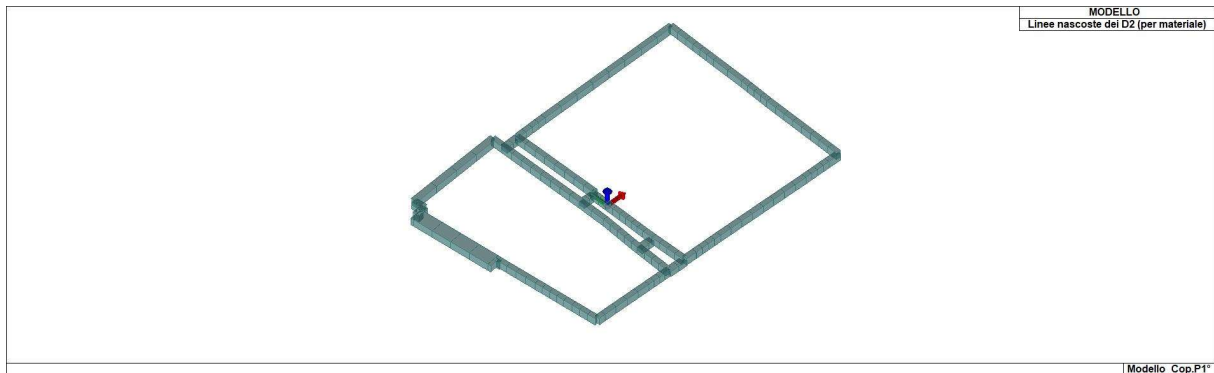
[3]- MATERIALE PER ELEVAZIONE -

	Calcestruzzo Classe C28/35		
Id	-	-	u.m.
3	< MATERIALE NUOVO >		
	Resistenza caratteristica cubica Rck	350.0	daN/cm2
	Resistenza caratteristica cilindrica fck	290.5	daN/cm2
	Resistenza fctm	28.4	daN/cm2
	Tensione caratteristica di snervamento acciaio	4500.0	daN/cm2
	Tipo acciaio	tipo C	
	Coefficiente gamma c	1.5	
	Coefficiente gamma s	1.1	
	Rapporto Rfessurata (assiale)	1.00	
	Rapporto Rfessurata (flessione)	1.00	
	Rapporto Rfessurata (taglio)	1.00	

[158]- MATERIALE PER ELEVAZIONE -

	materiale E = 2.100e+11 [w= 0.0]		
Id	-	-	u.m.
158	< MATERIALE NUOVO >		

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



ANALISI DEI CARICHI DEI SOLAI

Si riportano di seguito l'analisi dei carichi relative ai solai presenti nella struttura in oggetto:

TABELLA_CARICHI_SOLAI

ID Arch.	Tipo SOL	G1	G2	Q	Fatt. A	s sis.	Psi 0	Psi 1	Psi 2	Psi S 2	Fatt. Fi
-	-	kN/ m2	kN/ m2	kN/ m2	-	-	-	-	-	-	-
1	Variab.	1.54	2.50	2.00		1.00	0.70	0.50	0.30	0.30	1.00
2	Variab.	1.54	1.50	4.00		1.00	0.70	0.50	0.30	0.30	1.00

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Legenda

Tipo
SOL Indica la destinazione d'uso sulla base del carico variabile

G1 Carichi permanenti

G2 Carichi permanenti non strutturali

Q Carichi variabili e neve

Fatt. A Fattore di riduzione dell'area caricata (solo per solai speciali)

s sis. Coefficiente di riduzione del sovraccarico accidentale -(DM 96)-

Psi 0 Coefficiente di combinazione -(tab. 2.5.I NTC2018)-

Psi 1 Coefficiente di combinazione -(tab. 2.5.I NTC2018)-

Psi 2 Coefficiente di combinazione -(tab. 2.5.I NTC2018)-

Psi S 2 Coefficiente di combinazione che fornisce il valore Quasi Permanente dell'azione variabile Q_i -(OPCM 3274)-

Fatt. Fi Coefficiente che tiene conto della probabilità che tutti i carichi siano presenti sull'intera struttura durante l'azione sismica -(OPCM 3274)-

1 - SOLAIO INTERPIANO

Per il solaio si adottano i seguenti carichi in daN/mq:

Permanente G1	Permanente G2	Variabile Q
154.0	250.0	200.0

Categoria carichi variabili: A - Ambienti ad uso residenziale - Aree per attività domestiche e residenziali.

Coefficienti di combinazione: $\psi_0 = 0.70$, $\psi_1 = 0.50$, $\psi_2 = 0.30$

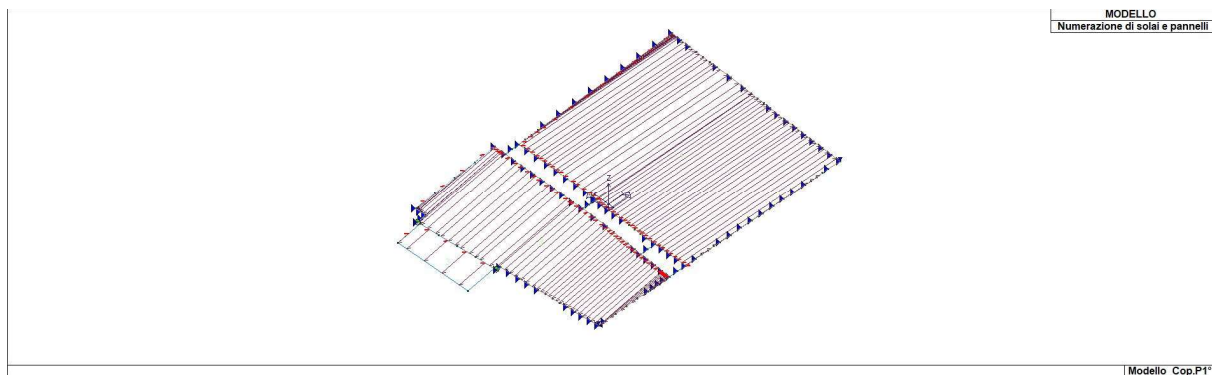
2 - BALCONE

Per il solaio si adottano i seguenti carichi in daN/mq:

Permanente G1	Permanente G2	Variabile Q
154.0	150.0	400.0

Categoria carichi variabili: A - Ambienti ad uso residenziale - Aree per attività domestiche e residenziali.

Coefficienti di combinazione: $\psi_0 = 0.70$, $\psi_1 = 0.50$, $\psi_2 = 0.30$



SCHEMATIZZAZIONE DEI CASI DI CARICO

E' possibile definire i casi di carico scegliendo fra le dodici tipologie elencate nella tabella seguente:

	Tipo CDC	Descrizione
1	Ggk	caso di carico comprensivo del peso proprio struttura
2	Gk	caso di carico con azioni permanenti
3	Qk	caso di carico con azioni variabili
4	Gsk	caso di carico comprensivo dei carichi permanenti sui solai e sulle coperture
5	Qsk	caso di carico comprensivo dei carichi variabili sui solai
6	Qnk	caso di carico comprensivo dei carichi di neve sulle coperture
7	Qtk	caso di carico comprensivo di una variazione termica agente sulla struttura
8	Qvk	caso di carico comprensivo di azioni da vento sulla struttura

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9	Esk	caso di carico sismico con analisi statica equivalente
10	Edk	caso di carico sismico con analisi dinamica
11	Etk	caso di carico comprensivo di azioni derivanti dall' incremento di spinta delle terre in condizione sismica
12	Pk	caso di carico comprensivo di azioni derivanti da coazioni, cedimenti e precompressioni

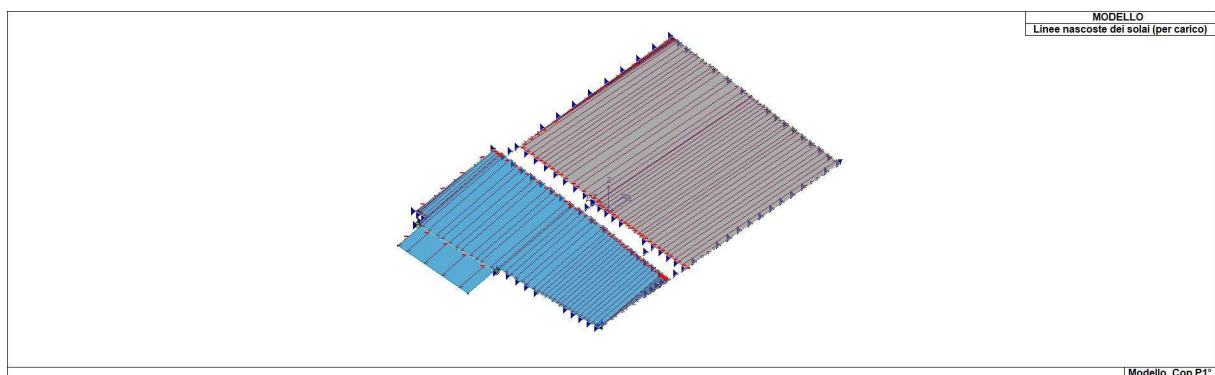
I casi di carico utilizzati nella modellazione oggetto della presente relazione sono i seguenti:

TABELLA_CASI_DI_CARICO

CDC	Tipo CDC	Sigla Id	Note
1	Ggk	CDC=Ggk (peso proprio della struttura)	
2	Gsk	CDC=G1sk (permanente solai-coperture)	
3	Gsk	CDC=G2sk (permanente solai-coperture n.c.d.)	
4	Qsk	CDC=Qsk (variabile solai)	

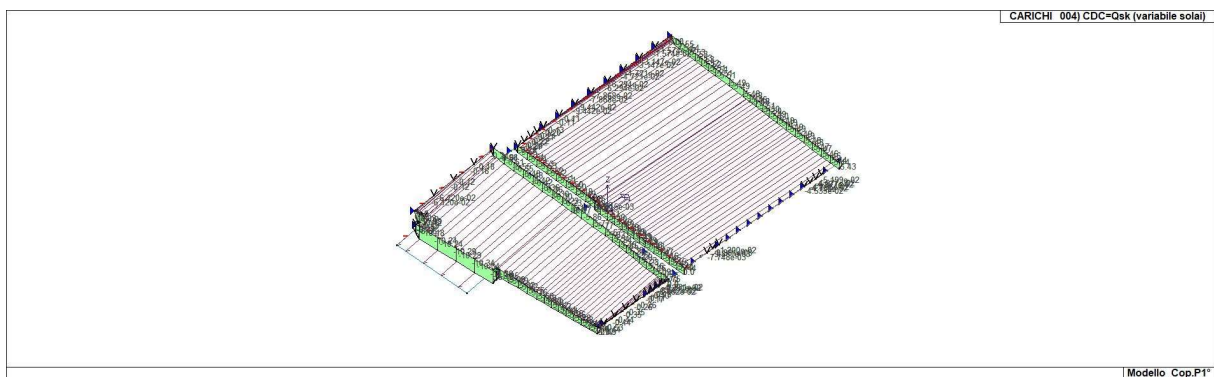
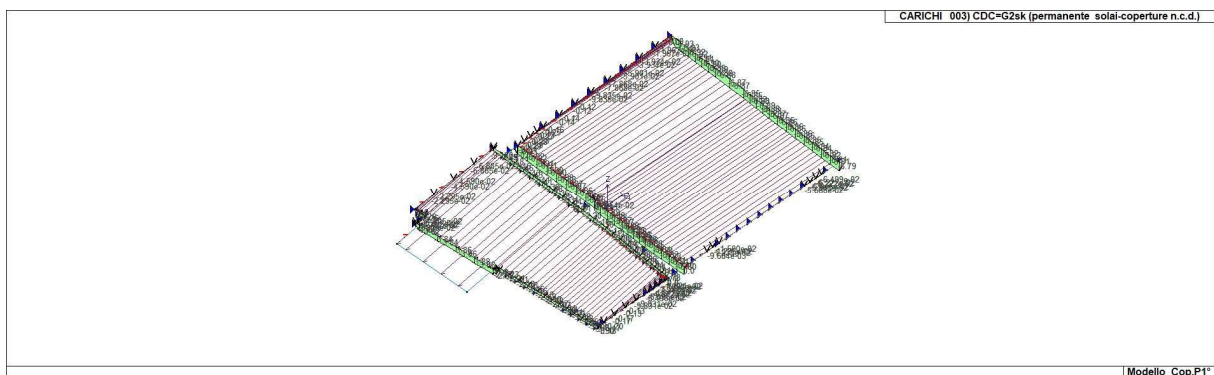
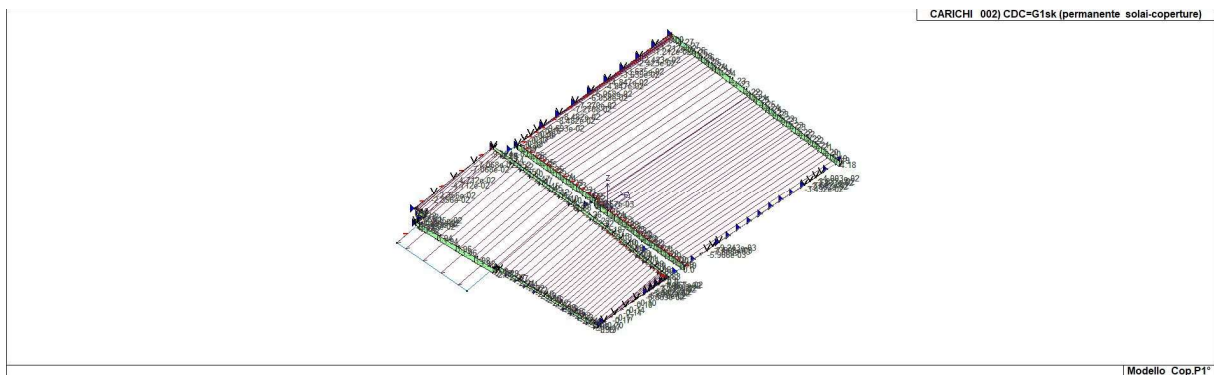
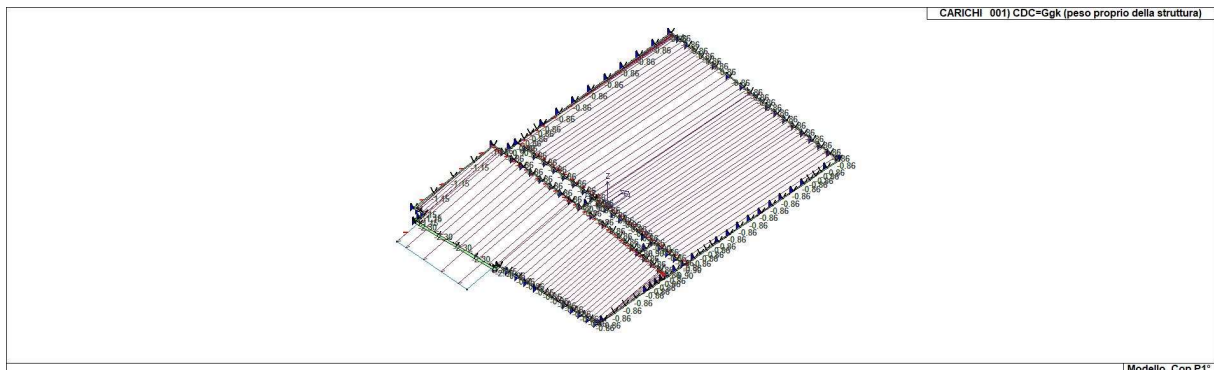
Legenda

Tipo CDC Indica il tipo di caso di carico



RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



DEFINIZIONE DELLE COMBINAZIONI

Le combinazioni previste per i diversi casi di carico (CDC) seguono le regole previste dalla Normativa vigente e sono destinate al controllo di sicurezza della struttura e alla verifica degli spostamenti e delle sollecitazioni.

Ai fini delle verifiche degli stati limite si definiscono le seguenti combinazioni delle azioni:

Combinazione fondamentale SLU

$$\gamma G_1 \cdot G_1 + \gamma G_2 \cdot G_2 + \gamma P \cdot P + \gamma Q_1 \cdot Q_{k1} + \gamma Q_2 \cdot \psi_{02} \cdot Q_{k2} + \gamma Q_3 \cdot \psi_{03} \cdot Q_{k3} + \dots$$

Combinazione caratteristica (rara) SLE

$$G_1 + G_2 + P + Q_{k1} + \psi_{02} \cdot Q_{k2} + \psi_{03} \cdot Q_{k3} + \dots$$

Combinazione frequente SLE

$$G_1 + G_2 + P + \psi_{11} \cdot Q_{k1} + \psi_{22} \cdot Q_{k2} + \psi_{23} \cdot Q_{k3} + \dots$$

Combinazione quasi permanente SLE

$$G_1 + G_2 + P + \psi_{21} \cdot Q_{k1} + \psi_{22} \cdot Q_{k2} + \psi_{23} \cdot Q_{k3} + \dots$$

Combinazione sismica, impiegata per gli stati limite ultimi e di esercizio connessi all'azione sismica E

$$E + G_1 + G_2 + P + \psi_{21} \cdot Q_{k1} + \psi_{22} \cdot Q_{k2} + \psi_{23} \cdot Q_{k3} + \dots$$

Combinazione eccezionale, impiegata per gli stati limite connessi alle azioni eccezionali

$$A_d + G_1 + G_2 + P + \psi_{21} \cdot Q_{k1} + \psi_{22} \cdot Q_{k2} + \psi_{23} \cdot Q_{k3} + \dots$$

Dove:

NTC 2018 Tabella 2.5.I

Destinazione d'uso/azione	ψ_0	ψ_1	ψ_2
Categoria A residenziali	0,70	0,50	0,30
Categoria B uffici	0,70	0,50	0,30
Categoria C ambienti suscettibili di affollamento	0,70	0,70	0,60
Categoria D ambienti ad uso commerciale	0,70	0,70	0,60
Categoria E biblioteche, archivi, magazzini,...	1,00	0,90	0,80
Categoria F Rimesse e parcheggi (autoveicoli ≤ 30 kN)	0,70	0,70	0,60
Categoria G Rimesse e parcheggi (autoveicoli > 30 kN)	0,70	0,50	0,30

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Categoria H Coperture	0,00	0,00	0,00
Vento	0,60	0,20	0,00
Neve a quota <= 1000 m	0,50	0,20	0,00
Neve a quota > 1000 m	0,70	0,50	0,20
Variazioni Termiche	0,60	0,50	0,00

Nelle verifiche possono essere adottati in alternativa due diversi approcci progettuali:

- per l'approccio 1 si considerano due diverse combinazioni di gruppi di coefficienti di sicurezza parziali per le azioni, per i materiali e per la resistenza globale (combinazione 1 con coefficienti A1 e combinazione 2 con coefficienti A2),
- per l'approccio 2 si definisce un'unica combinazione per le azioni, per la resistenza dei materiali e per la resistenza globale (con coefficienti A1).

NTC 2018 Tabella 2.6.I

		Coefficiente γ_F	EQU	A1	A2
Carichi permanenti	Favorevoli	γ_{G1}	0,9	1,0	1,0
	Sfavorevoli		1,1	1,3	1,0
Carichi permanenti non strutturali (Non compiutamente definiti)	Favorevoli	γ_{G2}	0,8	0,8	0,8
	Sfavorevoli		1,5	1,5	1,3
Carichi variabili	Favorevoli	γ_{Qi}	0,0	0,0	0,0
	Sfavorevoli		1,5	1,5	1,3

TIPO DI ANALISI EFFETTUATE

TABELLA_COMBINAZIONI

Tipo CMB	Da	Da	A	A
-	Id	Nome	Id	Nome
SLU	1	SLU		
SLE rara	2	SLE_Rare		
SLE frequente	3	SLE_Freq.		
SLE quasi permanente	4	SLE_Q.Perm.		

RISULTATI PRINCIPALI

LEGENDA RISULTATI ELEMENTI TIPO TRAVE

Il controllo dei risultati delle analisi condotte, per quanto concerne gli elementi tipo trave, è possibile in relazione alle tabelle sotto riportate.

Gli elementi vengono suddivisi in relazione alle proprietà in elementi:

tipo **pilastro**

tipo **trave in elevazione**

tipo **trave in fondazione**

Per ogni elemento e per ogni combinazione (o caso di carico) vengono riportati i risultati più significativi.

Per gli elementi tipo *pilastro* sono riportati in tabella i seguenti valori:

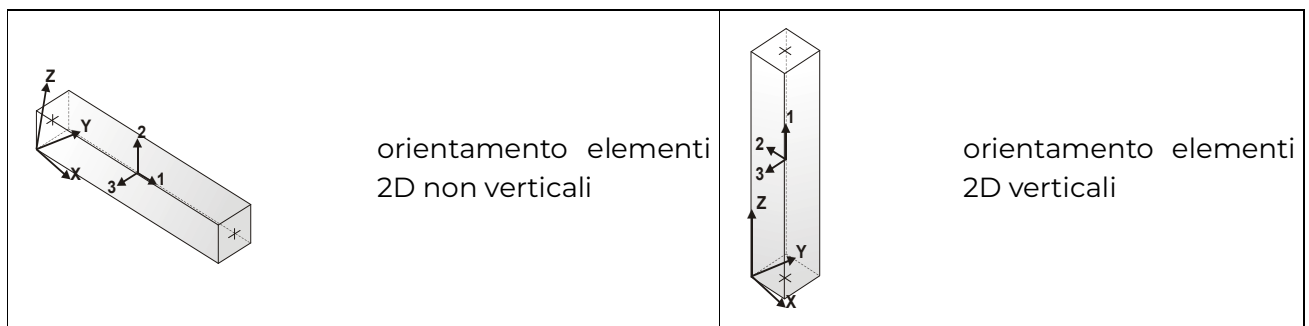
Pilas.	numero dell'elemento pilastro
Cmb	combinazione in cui si verificano i valori riportati
M3 mx/mn	momento flettente in campata M3 max (prima riga) / min (seconda riga)
M2 mx/mn	momento flettente in campata M2 max (prima riga) / min (seconda riga)
D2/D3	freccia massima in direzione 2 (prima riga) / direzione 3 (seconda riga)

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Q2/Q3	carico totale in direzione 2 (prima riga) / direzione 3 (seconda riga)
Pos.	ascissa del punto iniziale e finale dell'elemento
N, V2, ecc..	sei componenti di sollecitazione al piede ed in sommità dell'elemento

Per gli elementi tipo *trave in elevazione* sono riportati, oltre al numero dell'elemento, i medesimi risultati visti per i pilastri.

Per gli elementi tipo *trave in fondazione* (trave f.) sono riportati, oltre al numero dell'elemento, i medesimi risultati visti per i pilastri e la massima pressione sul terreno.



Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		kN m	kN m	m	kN	cm	kN	kN	kN	kN m	kN m	kN m
1	1	0.04	0.0	0.0	-13.10	0.0	0.0	3.68	0.0	8.59e-03	0.0	-0.23
		-1.72	0.0	0.0	0.0	26.0	0.0	-2.87	0.0	8.59e-03	0.0	-0.12
						52.0	0.0	-9.42	0.0	8.59e-03	0.0	-1.72
1	2	0.03	0.0	0.0	-9.09	0.0	0.0	2.55	0.0	5.93e-03	0.0	-0.16
		-1.19	0.0	0.0	0.0	26.0	0.0	-1.99	0.0	5.93e-03	0.0	-0.08
						52.0	0.0	-6.54	0.0	5.93e-03	0.0	-1.19
1	3	0.02	0.0	0.0	-7.66	0.0	0.0	2.15	0.0	4.96e-03	0.0	-0.13
		-1.00	0.0	0.0	0.0	26.0	0.0	-1.68	0.0	4.96e-03	0.0	-0.07
						52.0	0.0	-5.51	0.0	4.96e-03	0.0	-1.00
1	4	0.02	0.0	0.0	-7.09	0.0	0.0	1.99	0.0	4.57e-03	0.0	-0.12
		-0.93	0.0	0.0	0.0	26.0	0.0	-1.55	0.0	4.57e-03	0.0	-0.07
						52.0	0.0	-5.10	0.0	4.57e-03	0.0	-0.93

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
2	1	-0.15	0.0	0.0	-8.14	0.0	0.0	-0.16	0.0	5.14e-03	0.0	-0.15
		-1.52	0.0	0.0	0.0	16.2	0.0	-4.23	0.0	5.14e-03	0.0	-0.51
						32.4	0.0	-8.30	0.0	5.14e-03	0.0	-1.52
2	2	-0.11	0.0	0.0	-5.65	0.0	0.0	-0.11	0.0	3.54e-03	0.0	-0.11
		-1.06	0.0	0.0	0.0	16.2	0.0	-2.93	0.0	3.54e-03	0.0	-0.35
						32.4	0.0	-5.76	0.0	3.54e-03	0.0	-1.06
2	3	-0.09	0.0	0.0	-4.76	0.0	0.0	-0.09	0.0	2.95e-03	0.0	-0.09
		-0.89	0.0	0.0	0.0	16.2	0.0	-2.47	0.0	2.95e-03	0.0	-0.30
						32.4	0.0	-4.85	0.0	2.95e-03	0.0	-0.89
2	4	-0.08	0.0	0.0	-4.40	0.0	0.0	-0.08	0.0	2.71e-03	0.0	-0.08
		-0.82	0.0	0.0	0.0	16.2	0.0	-2.29	0.0	2.71e-03	0.0	-0.28
						32.4	0.0	-4.49	0.0	2.71e-03	0.0	-0.82
3	1	0.05	0.0	0.0	-5.66	0.0	0.0	3.00	0.0	-0.02	0.0	-0.23
		-0.23	0.0	0.0	0.0	17.4	0.0	0.18	0.0	-0.02	0.0	0.05
						34.8	0.0	-2.66	0.0	-0.02	0.0	-0.16
3	2	0.04	0.0	0.0	-3.92	0.0	0.0	2.08	0.0	-0.01	0.0	-0.16
		-0.16	0.0	0.0	0.0	17.4	0.0	0.13	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.84	0.0	-0.01	0.0	-0.11
3	3	0.03	0.0	0.0	-2.89	0.0	0.0	1.53	0.0	-0.01	0.0	-0.12
		-0.12	0.0	0.0	0.0	17.4	0.0	0.09	0.0	-0.01	0.0	0.03
						34.8	0.0	-1.36	0.0	-0.01	0.0	-0.08
3	4	0.02	0.0	0.0	-2.48	0.0	0.0	1.31	0.0	-0.01	0.0	-0.10
		-0.10	0.0	0.0	0.0	17.4	0.0	0.08	0.0	-0.01	0.0	0.02
						34.8	0.0	-1.17	0.0	-0.01	0.0	-0.07
4	1	-0.10	0.0	0.0	-6.96	0.0	0.0	5.79	0.0	0.04	0.0	-1.17
		-1.17	0.0	0.0	0.0	22.1	0.0	2.32	0.0	0.04	0.0	-0.27
						44.1	0.0	-1.17	0.0	0.04	0.0	-0.15
4	2	-0.07	0.0	0.0	-4.82	0.0	0.0	4.01	0.0	0.03	0.0	-0.81

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.81	0.0	0.0	0.0	22.1	0.0	1.61	0.0	0.03	0.0	-0.19
						44.1	0.0	-0.81	0.0	0.03	0.0	-0.10
4	3	-0.05	0.0	0.0	-3.56	0.0	0.0	2.96	0.0	0.02	0.0	-0.60
		-0.60	0.0	0.0	0.0	22.1	0.0	1.19	0.0	0.02	0.0	-0.14
						44.1	0.0	-0.60	0.0	0.02	0.0	-0.07
4	4	-0.05	0.0	0.0	-3.05	0.0	0.0	2.54	0.0	0.02	0.0	-0.51
		-0.51	0.0	0.0	0.0	22.1	0.0	1.02	0.0	0.02	0.0	-0.12
						44.1	0.0	-0.51	0.0	0.02	0.0	-0.06
5	1	0.45	0.0	-1.65e-05	-3.99	0.0	0.0	8.12	0.0	-0.03	0.0	-1.21
		-1.21	0.0	0.0	0.0	13.5	0.0	6.13	0.0	-0.03	0.0	-0.24
						27.0	0.0	4.13	0.0	-0.03	0.0	0.45
5	2	0.31	0.0	-1.15e-05	-2.76	0.0	0.0	5.62	0.0	-0.02	0.0	-0.84
		-0.84	0.0	0.0	0.0	13.5	0.0	4.25	0.0	-0.02	0.0	-0.17
						27.0	0.0	2.86	0.0	-0.02	0.0	0.31
5	3	0.23	0.0	-8.47e-06	-2.05	0.0	0.0	4.16	0.0	-0.02	0.0	-0.62
		-0.62	0.0	0.0	0.0	13.5	0.0	3.14	0.0	-0.02	0.0	-0.12
						27.0	0.0	2.11	0.0	-0.02	0.0	0.23
5	4	0.20	0.0	-7.28e-06	-1.76	0.0	0.0	3.57	0.0	-0.01	0.0	-0.53
		-0.53	0.0	0.0	0.0	13.5	0.0	2.70	0.0	-0.01	0.0	-0.11
						27.0	0.0	1.82	0.0	-0.01	0.0	0.20
6	1	-0.15	0.0	0.0	-3.35	0.0	0.0	-2.91	0.0	-4.70e-03	0.0	-0.15
		-1.21	0.0	0.0	0.0	11.5	0.0	-4.58	0.0	-4.70e-03	0.0	-0.58
						23.0	0.0	-6.26	0.0	-4.70e-03	0.0	-1.21
6	2	-0.11	0.0	0.0	-2.32	0.0	0.0	-2.02	0.0	-3.29e-03	0.0	-0.11
		-0.84	0.0	0.0	0.0	11.5	0.0	-3.17	0.0	-3.29e-03	0.0	-0.40
						23.0	0.0	-4.34	0.0	-3.29e-03	0.0	-0.84
6	3	-0.08	0.0	0.0	-1.72	0.0	0.0	-1.49	0.0	-2.51e-03	0.0	-0.08
		-0.62	0.0	0.0	0.0	11.5	0.0	-2.35	0.0	-2.51e-03	0.0	-0.30

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						23.0	0.0	-3.21	0.0	-2.51e-03	0.0	-0.62
6	4	-0.07	0.0	0.0	-1.48	0.0	0.0	-1.28	0.0	-2.21e-03	0.0	-0.07
		-0.53	0.0	0.0	0.0	11.5	0.0	-2.02	0.0	-2.21e-03	0.0	-0.26
						23.0	0.0	-2.76	0.0	-2.21e-03	0.0	-0.53
7	1	0.04	0.0	0.0	-1.96	0.0	0.0	0.41	0.0	1.09e-04	0.0	0.01
		-0.07	0.0	0.0	0.0	13.6	0.0	-0.16	0.0	1.09e-04	0.0	0.04
						27.3	0.0	-1.55	0.0	1.09e-04	0.0	-0.07
7	2	0.03	0.0	0.0	-1.37	0.0	0.0	0.29	0.0	9.53e-05	0.0	7.94e-03
		-0.05	0.0	0.0	0.0	13.6	0.0	-0.11	0.0	9.53e-05	0.0	0.03
						27.3	0.0	-1.08	0.0	9.53e-05	0.0	-0.05
7	3	0.02	0.0	0.0	-1.05	0.0	0.0	0.23	0.0	1.25e-04	0.0	6.78e-03
		-0.04	0.0	0.0	0.0	13.6	0.0	-0.09	0.0	1.25e-04	0.0	0.02
						27.3	0.0	-0.82	0.0	1.25e-04	0.0	-0.04
7	4	0.02	0.0	0.0	-0.92	0.0	0.0	0.21	0.0	1.37e-04	0.0	6.32e-03
		-0.03	0.0	0.0	0.0	13.6	0.0	-0.08	0.0	1.37e-04	0.0	0.02
						27.3	0.0	-0.71	0.0	1.37e-04	0.0	-0.03
8	1	0.20	0.0	-1.10e-06	-6.25	0.0	0.0	4.22	0.0	4.38e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	16.0	0.0	0.16	0.0	4.38e-03	0.0	0.20
						32.0	0.0	-2.03	0.0	4.38e-03	0.0	9.52e-04
8	2	0.14	0.0	0.0	-4.34	0.0	0.0	2.93	0.0	3.01e-03	0.0	-0.10
		-0.10	0.0	0.0	0.0	16.0	0.0	0.11	0.0	3.01e-03	0.0	0.14
						32.0	0.0	-1.42	0.0	3.01e-03	0.0	6.75e-04
8	3	0.12	0.0	0.0	-3.67	0.0	0.0	2.47	0.0	2.50e-03	0.0	-0.09
		-0.09	0.0	0.0	0.0	16.0	0.0	0.10	0.0	2.50e-03	0.0	0.12
						32.0	0.0	-1.20	0.0	2.50e-03	0.0	5.96e-04
8	4	0.11	0.0	0.0	-3.40	0.0	0.0	2.29	0.0	2.29e-03	0.0	-0.08
		-0.08	0.0	0.0	0.0	16.0	0.0	0.09	0.0	2.29e-03	0.0	0.11
						32.0	0.0	-1.12	0.0	2.29e-03	0.0	5.65e-04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
9	1	0.02	0.0	0.0	-0.79	0.0	0.0	0.40	0.0	-1.11e-03	0.0	-0.04
		-0.04	0.0	0.0	0.0	28.8	0.0	1.40e-03	0.0	-1.11e-03	0.0	0.02
						57.6	0.0	-0.39	0.0	-1.11e-03	0.0	-0.04
9	2	0.01	0.0	0.0	-0.59	0.0	0.0	0.30	0.0	-7.82e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	9.45e-04	0.0	-7.82e-04	0.0	0.01
						57.6	0.0	-0.29	0.0	-7.82e-04	0.0	-0.03
9	3	0.01	0.0	0.0	-0.58	0.0	0.0	0.29	0.0	-6.85e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	7.53e-04	0.0	-6.85e-04	0.0	0.01
						57.6	0.0	-0.28	0.0	-6.85e-04	0.0	-0.03
9	4	0.01	0.0	0.0	-0.57	0.0	0.0	0.29	0.0	-6.46e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	6.76e-04	0.0	-6.46e-04	0.0	0.01
						57.6	0.0	-0.28	0.0	-6.46e-04	0.0	-0.03
10	1	0.02	0.0	0.0	-0.86	0.0	0.0	0.44	0.0	-1.11e-03	0.0	-0.04
		-0.04	0.0	0.0	0.0	28.8	0.0	6.62e-03	0.0	-1.11e-03	0.0	0.02
						57.6	0.0	-0.42	0.0	-1.11e-03	0.0	-0.04
10	2	0.01	0.0	0.0	-0.65	0.0	0.0	0.33	0.0	-7.82e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	4.77e-03	0.0	-7.82e-04	0.0	0.01
						57.6	0.0	-0.32	0.0	-7.82e-04	0.0	-0.03
10	3	0.01	0.0	0.0	-0.62	0.0	0.0	0.32	0.0	-6.85e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	4.29e-03	0.0	-6.85e-04	0.0	0.01
						57.6	0.0	-0.30	0.0	-6.85e-04	0.0	-0.03
10	4	0.01	0.0	0.0	-0.61	0.0	0.0	0.31	0.0	-6.46e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	4.10e-03	0.0	-6.46e-04	0.0	0.01
						57.6	0.0	-0.30	0.0	-6.46e-04	0.0	-0.03
11	1	0.01	0.0	0.0	-0.71	0.0	0.0	0.35	0.0	-1.11e-03	0.0	-0.03
		-0.04	0.0	0.0	0.0	28.8	0.0	-0.01	0.0	-1.11e-03	0.0	0.01
						57.6	0.0	-0.36	0.0	-1.11e-03	0.0	-0.04
11	2	0.01	0.0	0.0	-0.54	0.0	0.0	0.26	0.0	-7.82e-04	0.0	-0.03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.03	0.0	0.0	0.0	28.8	0.0	-8.65e-03	0.0	-7.82e-04	0.0	0.01
						57.6	0.0	-0.27	0.0	-7.82e-04	0.0	-0.03
11	3	0.01	0.0	0.0	-0.53	0.0	0.0	0.26	0.0	-6.85e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	-8.65e-03	0.0	-6.85e-04	0.0	0.01
						57.6	0.0	-0.27	0.0	-6.85e-04	0.0	-0.03
11	4	0.01	0.0	0.0	-0.53	0.0	0.0	0.26	0.0	-6.46e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	-8.65e-03	0.0	-6.46e-04	0.0	0.01
						57.6	0.0	-0.27	0.0	-6.46e-04	0.0	-0.03
12	1	0.03	0.0	0.0	-0.90	0.0	0.0	0.43	0.0	-1.11e-03	0.0	-0.03
		-0.04	0.0	0.0	0.0	28.8	0.0	-0.03	0.0	-1.11e-03	0.0	0.03
						57.6	0.0	-0.47	0.0	-1.11e-03	0.0	-0.04
12	2	0.02	0.0	0.0	-0.68	0.0	0.0	0.32	0.0	-7.82e-04	0.0	-0.02
		-0.03	0.0	0.0	0.0	28.8	0.0	-0.02	0.0	-7.82e-04	0.0	0.02
						57.6	0.0	-0.35	0.0	-7.82e-04	0.0	-0.03
12	3	0.02	0.0	0.0	-0.65	0.0	0.0	0.31	0.0	-6.85e-04	0.0	-0.02
		-0.03	0.0	0.0	0.0	28.8	0.0	-0.02	0.0	-6.85e-04	0.0	0.02
						57.6	0.0	-0.34	0.0	-6.85e-04	0.0	-0.03
12	4	0.02	0.0	0.0	-0.63	0.0	0.0	0.30	0.0	-6.46e-04	0.0	-0.02
		-0.03	0.0	0.0	0.0	28.8	0.0	-0.02	0.0	-6.46e-04	0.0	0.02
						57.6	0.0	-0.33	0.0	-6.46e-04	0.0	-0.03
13	1	0.02	0.0	0.0	-0.75	0.0	0.0	0.38	0.0	-1.11e-03	0.0	-0.04
		-0.04	0.0	0.0	0.0	28.8	0.0	3.92e-03	0.0	-1.11e-03	0.0	0.02
						57.6	0.0	-0.36	0.0	-1.11e-03	0.0	-0.03
13	2	0.01	0.0	0.0	-0.57	0.0	0.0	0.29	0.0	-7.82e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	2.87e-03	0.0	-7.82e-04	0.0	0.01
						57.6	0.0	-0.28	0.0	-7.82e-04	0.0	-0.03
13	3	0.01	0.0	0.0	-0.55	0.0	0.0	0.28	0.0	-6.85e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	2.65e-03	0.0	-6.85e-04	0.0	0.01

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						57.6	0.0	-0.27	0.0	-6.85e-04	0.0	-0.03
13	4	0.01	0.0	0.0	-0.55	0.0	0.0	0.28	0.0	-6.46e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	2.56e-03	0.0	-6.46e-04	0.0	0.01
						57.6	0.0	-0.27	0.0	-6.46e-04	0.0	-0.03
14	1	0.02	0.0	0.0	-0.82	0.0	0.0	0.42	0.0	-1.11e-03	0.0	-0.04
		-0.04	0.0	0.0	0.0	28.8	0.0	8.38e-04	0.0	-1.11e-03	0.0	0.02
						57.6	0.0	-0.41	0.0	-1.11e-03	0.0	-0.04
14	2	0.02	0.0	0.0	-0.62	0.0	0.0	0.31	0.0	-7.82e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	5.49e-04	0.0	-7.82e-04	0.0	0.02
						57.6	0.0	-0.31	0.0	-7.82e-04	0.0	-0.03
14	3	0.01	0.0	0.0	-0.60	0.0	0.0	0.30	0.0	-6.85e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	4.10e-04	0.0	-6.85e-04	0.0	0.01
						57.6	0.0	-0.30	0.0	-6.85e-04	0.0	-0.03
14	4	0.01	0.0	0.0	-0.59	0.0	0.0	0.30	0.0	-6.46e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	3.55e-04	0.0	-6.46e-04	0.0	0.01
						57.6	0.0	-0.29	0.0	-6.46e-04	0.0	-0.03
15	1	0.03	0.0	0.0	-0.67	0.0	0.0	0.41	0.0	-1.11e-03	0.0	-0.04
		-0.04	0.0	0.0	0.0	28.8	0.0	0.07	0.0	-1.11e-03	0.0	0.03
						57.6	0.0	-0.25	0.0	-1.11e-03	0.0	4.34e-03
15	2	0.03	0.0	0.0	-0.51	0.0	0.0	0.31	0.0	-7.82e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	0.06	0.0	-7.82e-04	0.0	0.02
						57.6	0.0	-0.20	0.0	-7.82e-04	0.0	2.99e-03
15	3	0.02	0.0	0.0	-0.51	0.0	0.0	0.31	0.0	-6.85e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	0.05	0.0	-6.85e-04	0.0	0.02
						57.6	0.0	-0.20	0.0	-6.85e-04	0.0	2.48e-03
15	4	0.02	0.0	0.0	-0.51	0.0	0.0	0.31	0.0	-6.46e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	28.8	0.0	0.05	0.0	-6.46e-04	0.0	0.02
						57.6	0.0	-0.20	0.0	-6.46e-04	0.0	2.27e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
16	1	1.11	0.0	-6.23e-06	-5.40	0.0	0.0	5.40	0.0	-0.01	0.0	0.53
		0.53	0.0	0.0	0.0	10.8	0.0	2.70	0.0	-0.01	0.0	0.97
						21.5	0.0	-1.83e-03	0.0	-0.01	0.0	1.11
16	2	0.77	0.0	-4.32e-06	-3.75	0.0	0.0	3.75	0.0	-8.73e-03	0.0	0.37
		0.37	0.0	0.0	0.0	10.8	0.0	1.87	0.0	-8.73e-03	0.0	0.67
						21.5	0.0	-1.26e-03	0.0	-8.73e-03	0.0	0.77
16	3	0.65	0.0	-3.64e-06	-3.16	0.0	0.0	3.16	0.0	-7.34e-03	0.0	0.31
		0.31	0.0	0.0	0.0	10.8	0.0	1.58	0.0	-7.34e-03	0.0	0.57
						21.5	0.0	-1.06e-03	0.0	-7.34e-03	0.0	0.65
16	4	0.60	0.0	-3.37e-06	-2.92	0.0	0.0	2.92	0.0	-6.78e-03	0.0	0.29
		0.29	0.0	0.0	0.0	10.8	0.0	1.46	0.0	-6.78e-03	0.0	0.52
						21.5	0.0	-9.75e-04	0.0	-6.78e-03	0.0	0.60
17	1	0.06	0.0	-1.56e-06	-0.51	0.0	0.0	0.99	0.0	9.74e-04	0.0	-0.11
		-0.11	0.0	0.0	0.0	11.8	0.0	0.74	0.0	9.74e-04	0.0	-0.01
						23.5	0.0	0.48	0.0	9.74e-04	0.0	0.06
17	2	0.04	0.0	-1.14e-06	-0.37	0.0	0.0	0.73	0.0	7.47e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	11.8	0.0	0.54	0.0	7.47e-04	0.0	-9.19e-03
						23.5	0.0	0.35	0.0	7.47e-04	0.0	0.04
17	3	0.04	0.0	-1.06e-06	-0.35	0.0	0.0	0.67	0.0	7.39e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	11.8	0.0	0.50	0.0	7.39e-04	0.0	-8.61e-03
						23.5	0.0	0.33	0.0	7.39e-04	0.0	0.04
17	4	0.04	0.0	-1.02e-06	-0.33	0.0	0.0	0.65	0.0	7.36e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	11.8	0.0	0.48	0.0	7.36e-04	0.0	-8.37e-03
						23.5	0.0	0.32	0.0	7.36e-04	0.0	0.04
18	1	-1.25e-03	0.0	0.0	-0.94	0.0	0.0	0.64	0.0	-1.11e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	28.8	0.0	0.16	0.0	-1.11e-03	0.0	-8.83e-03
						57.6	0.0	-0.31	0.0	-1.11e-03	0.0	-0.03
18	2	-7.23e-04	0.0	0.0	-0.70	0.0	0.0	0.47	0.0	-7.82e-04	0.0	-0.09

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.09	0.0	0.0	0.0	28.8	0.0	0.12	0.0	-7.82e-04	0.0	-6.10e-03
						57.6	0.0	-0.23	0.0	-7.82e-04	0.0	-0.02
18	3	-3.76e-04	0.0	0.0	-0.67	0.0	0.0	0.44	0.0	-6.85e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	28.8	0.0	0.11	0.0	-6.85e-04	0.0	-5.09e-03
						57.6	0.0	-0.23	0.0	-6.85e-04	0.0	-0.02
18	4	-2.37e-04	0.0	0.0	-0.65	0.0	0.0	0.43	0.0	-6.46e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	28.8	0.0	0.10	0.0	-6.46e-04	0.0	-4.69e-03
						57.6	0.0	-0.22	0.0	-6.46e-04	0.0	-0.02
19	1	-0.15	0.0	0.0	-8.12	0.0	0.0	8.85	0.0	4.38e-03	0.0	-1.68
		-1.68	0.0	0.0	0.0	16.0	0.0	4.80	0.0	4.38e-03	0.0	-0.59
						32.0	0.0	0.74	0.0	4.38e-03	0.0	-0.15
19	2	-0.10	0.0	0.0	-5.63	0.0	0.0	6.14	0.0	3.01e-03	0.0	-1.17
		-1.17	0.0	0.0	0.0	16.0	0.0	3.33	0.0	3.01e-03	0.0	-0.41
						32.0	0.0	0.51	0.0	3.01e-03	0.0	-0.10
19	3	-0.09	0.0	0.0	-4.74	0.0	0.0	5.17	0.0	2.50e-03	0.0	-0.98
		-0.98	0.0	0.0	0.0	16.0	0.0	2.80	0.0	2.50e-03	0.0	-0.34
						32.0	0.0	0.43	0.0	2.50e-03	0.0	-0.09
19	4	-0.08	0.0	0.0	-4.39	0.0	0.0	4.79	0.0	2.29e-03	0.0	-0.91
		-0.91	0.0	0.0	0.0	16.0	0.0	2.59	0.0	2.29e-03	0.0	-0.32
						32.0	0.0	0.40	0.0	2.29e-03	0.0	-0.08
20	1	0.67	0.0	-2.16e-05	-6.33	0.0	0.0	12.70	0.0	4.39e-03	0.0	-1.72
		-1.72	0.0	0.0	0.0	12.5	0.0	9.54	0.0	4.39e-03	0.0	-0.32
						25.1	0.0	6.38	0.0	4.39e-03	0.0	0.67
20	2	0.47	0.0	-1.50e-05	-4.39	0.0	0.0	8.81	0.0	3.02e-03	0.0	-1.19
		-1.19	0.0	0.0	0.0	12.5	0.0	6.62	0.0	3.02e-03	0.0	-0.22
						25.1	0.0	4.42	0.0	3.02e-03	0.0	0.47
20	3	0.39	0.0	-1.26e-05	-3.70	0.0	0.0	7.42	0.0	2.51e-03	0.0	-1.00
		-1.00	0.0	0.0	0.0	12.5	0.0	5.58	0.0	2.51e-03	0.0	-0.19

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						25.1	0.0	3.73	0.0	2.51e-03	0.0	0.39
20	4	0.36	0.0	-1.17e-05	-3.42	0.0	0.0	6.87	0.0	2.30e-03	0.0	-0.93
		-0.93	0.0	0.0	0.0	12.5	0.0	5.16	0.0	2.30e-03	0.0	-0.18
						25.1	0.0	3.45	0.0	2.30e-03	0.0	0.36
21	1	0.08	0.0	-1.08e-06	-13.07	0.0	0.0	9.13	0.0	8.59e-03	0.0	-1.58
		-1.58	0.0	0.0	0.0	26.0	0.0	2.60	0.0	8.59e-03	0.0	-0.05
						52.0	0.0	-3.94	0.0	8.59e-03	0.0	-0.23
21	2	0.06	0.0	0.0	-9.07	0.0	0.0	6.34	0.0	5.93e-03	0.0	-1.10
		-1.10	0.0	0.0	0.0	26.0	0.0	1.80	0.0	5.93e-03	0.0	-0.04
						52.0	0.0	-2.73	0.0	5.93e-03	0.0	-0.16
21	3	0.05	0.0	0.0	-7.64	0.0	0.0	5.34	0.0	4.96e-03	0.0	-0.92
		-0.92	0.0	0.0	0.0	26.0	0.0	1.52	0.0	4.96e-03	0.0	-0.03
						52.0	0.0	-2.30	0.0	4.96e-03	0.0	-0.13
21	4	0.04	0.0	0.0	-7.07	0.0	0.0	4.94	0.0	4.57e-03	0.0	-0.85
		-0.85	0.0	0.0	0.0	26.0	0.0	1.41	0.0	4.57e-03	0.0	-0.03
						52.0	0.0	-2.13	0.0	4.57e-03	0.0	-0.12
22	1	0.64	0.0	-1.88e-05	-6.05	0.0	0.0	12.04	0.0	-0.03	0.0	-1.52
		-1.52	0.0	0.0	0.0	12.0	0.0	9.02	0.0	-0.03	0.0	-0.26
						24.0	0.0	5.99	0.0	-0.03	0.0	0.64
22	2	0.45	0.0	-1.30e-05	-4.20	0.0	0.0	8.35	0.0	-0.02	0.0	-1.06
		-1.06	0.0	0.0	0.0	12.0	0.0	6.26	0.0	-0.02	0.0	-0.18
						24.0	0.0	4.16	0.0	-0.02	0.0	0.45
22	3	0.38	0.0	-1.10e-05	-3.54	0.0	0.0	7.04	0.0	-0.02	0.0	-0.89
		-0.89	0.0	0.0	0.0	12.0	0.0	5.27	0.0	-0.02	0.0	-0.15
						24.0	0.0	3.50	0.0	-0.02	0.0	0.38
22	4	0.35	0.0	-1.02e-05	-3.27	0.0	0.0	6.51	0.0	-0.02	0.0	-0.82
		-0.82	0.0	0.0	0.0	12.0	0.0	4.88	0.0	-0.02	0.0	-0.14
						24.0	0.0	3.24	0.0	-0.02	0.0	0.35

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
23	1	0.22	0.0	-1.25e-06	-8.06	0.0	0.0	3.33	0.0	5.14e-03	0.0	-1.23e-03
		-0.23	0.0	0.0	0.0	16.2	0.0	-0.70	0.0	5.14e-03	0.0	0.21
						32.4	0.0	-4.74	0.0	5.14e-03	0.0	-0.23
23	2	0.15	0.0	0.0	-5.59	0.0	0.0	2.31	0.0	3.54e-03	0.0	-8.74e-04
		-0.16	0.0	0.0	0.0	16.2	0.0	-0.49	0.0	3.54e-03	0.0	0.15
						32.4	0.0	-3.29	0.0	3.54e-03	0.0	-0.16
23	3	0.13	0.0	0.0	-4.71	0.0	0.0	1.95	0.0	2.95e-03	0.0	-7.67e-04
		-0.13	0.0	0.0	0.0	16.2	0.0	-0.41	0.0	2.95e-03	0.0	0.12
						32.4	0.0	-2.77	0.0	2.95e-03	0.0	-0.13
23	4	0.12	0.0	0.0	-4.36	0.0	0.0	1.80	0.0	2.71e-03	0.0	-7.25e-04
		-0.12	0.0	0.0	0.0	16.2	0.0	-0.38	0.0	2.71e-03	0.0	0.11
						32.4	0.0	-2.56	0.0	2.71e-03	0.0	-0.12
24	1	-6.53e-04	0.0	0.0	-0.46	0.0	0.0	0.36	0.0	-1.20e-03	0.0	-0.06
		-0.06	0.0	0.0	0.0	20.5	0.0	0.13	0.0	-1.20e-03	0.0	-7.94e-03
						41.0	0.0	-0.10	0.0	-1.20e-03	0.0	-5.15e-03
24	2	-4.88e-05	0.0	0.0	-0.35	0.0	0.0	0.28	0.0	-8.53e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	20.5	0.0	0.10	0.0	-8.53e-04	0.0	-5.62e-03
						41.0	0.0	-0.08	0.0	-8.53e-04	0.0	-3.55e-03
24	3	6.06e-04	0.0	0.0	-0.35	0.0	0.0	0.27	0.0	-7.50e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	20.5	0.0	0.10	0.0	-7.50e-04	0.0	-4.90e-03
						41.0	0.0	-0.08	0.0	-7.50e-04	0.0	-2.95e-03
24	4	8.68e-04	0.0	0.0	-0.35	0.0	0.0	0.27	0.0	-7.09e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	20.5	0.0	0.10	0.0	-7.09e-04	0.0	-4.62e-03
						41.0	0.0	-0.08	0.0	-7.09e-04	0.0	-2.71e-03
25	1	0.03	0.0	0.0	-0.27	0.0	0.0	0.54	0.0	-4.98e-04	0.0	-0.06
		-0.06	0.0	0.0	0.0	10.3	0.0	0.40	0.0	-4.98e-04	0.0	-7.56e-03
						20.5	0.0	0.27	0.0	-4.98e-04	0.0	0.03
25	2	0.02	0.0	0.0	-0.21	0.0	0.0	0.41	0.0	-3.17e-04	0.0	-0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.04	0.0	0.0	0.0	10.3	0.0	0.30	0.0	-3.17e-04	0.0	-5.82e-03
						20.5	0.0	0.20	0.0	-3.17e-04	0.0	0.02
25	3	0.02	0.0	0.0	-0.20	0.0	0.0	0.40	0.0	-2.25e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	10.3	0.0	0.30	0.0	-2.25e-04	0.0	-5.83e-03
						20.5	0.0	0.20	0.0	-2.25e-04	0.0	0.02
25	4	0.02	0.0	0.0	-0.20	0.0	0.0	0.39	0.0	-1.88e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	10.3	0.0	0.30	0.0	-1.88e-04	0.0	-5.84e-03
						20.5	0.0	0.20	0.0	-1.88e-04	0.0	0.02
26	1	-6.00e-03	0.0	0.0	-0.42	0.0	0.0	0.33	0.0	-1.10e-03	0.0	-0.06
		-0.06	0.0	0.0	0.0	18.7	0.0	0.12	0.0	-1.10e-03	0.0	-0.01
						37.4	0.0	-0.09	0.0	-1.10e-03	0.0	-9.22e-03
26	2	-4.60e-03	0.0	0.0	-0.32	0.0	0.0	0.26	0.0	-7.74e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	18.7	0.0	0.10	0.0	-7.74e-04	0.0	-9.85e-03
						37.4	0.0	-0.07	0.0	-7.74e-04	0.0	-7.11e-03
26	3	-4.57e-03	0.0	0.0	-0.32	0.0	0.0	0.26	0.0	-6.73e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	18.7	0.0	0.09	0.0	-6.73e-04	0.0	-9.75e-03
						37.4	0.0	-0.07	0.0	-6.73e-04	0.0	-7.13e-03
26	4	-4.56e-03	0.0	0.0	-0.32	0.0	0.0	0.26	0.0	-6.32e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	18.7	0.0	0.09	0.0	-6.32e-04	0.0	-9.71e-03
						37.4	0.0	-0.07	0.0	-6.32e-04	0.0	-7.13e-03
27	1	0.02	0.0	0.0	-0.24	0.0	0.0	0.48	0.0	-5.45e-04	0.0	-0.05
		-0.05	0.0	0.0	0.0	10.7	0.0	0.36	0.0	-5.45e-04	0.0	-8.32e-03
						21.4	0.0	0.24	0.0	-5.45e-04	0.0	0.02
27	2	0.02	0.0	0.0	-0.18	0.0	0.0	0.37	0.0	-3.49e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	10.7	0.0	0.28	0.0	-3.49e-04	0.0	-6.43e-03
						21.4	0.0	0.19	0.0	-3.49e-04	0.0	0.02
27	3	0.02	0.0	0.0	-0.18	0.0	0.0	0.37	0.0	-2.50e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	10.7	0.0	0.28	0.0	-2.50e-04	0.0	-6.47e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						21.4	0.0	0.19	0.0	-2.50e-04	0.0	0.02
27	4	0.02	0.0	0.0	-0.18	0.0	0.0	0.37	0.0	-2.10e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	10.7	0.0	0.28	0.0	-2.10e-04	0.0	-6.49e-03
						21.4	0.0	0.19	0.0	-2.10e-04	0.0	0.02
28	1	-5.33e-03	0.0	0.0	-0.39	0.0	0.0	0.06	0.0	-1.38e-03	0.0	-7.07e-03
		-0.05	0.0	0.0	0.0	17.5	0.0	-0.13	0.0	-1.38e-03	0.0	-0.01
						35.0	0.0	-0.33	0.0	-1.38e-03	0.0	-0.05
28	2	-4.09e-03	0.0	0.0	-0.30	0.0	0.0	0.05	0.0	-9.89e-04	0.0	-5.42e-03
		-0.04	0.0	0.0	0.0	17.5	0.0	-0.10	0.0	-9.89e-04	0.0	-0.01
						35.0	0.0	-0.25	0.0	-9.89e-04	0.0	-0.04
28	3	-4.08e-03	0.0	0.0	-0.30	0.0	0.0	0.05	0.0	-8.90e-04	0.0	-5.41e-03
		-0.04	0.0	0.0	0.0	17.5	0.0	-0.10	0.0	-8.90e-04	0.0	-0.01
						35.0	0.0	-0.25	0.0	-8.90e-04	0.0	-0.04
28	4	-4.08e-03	0.0	0.0	-0.30	0.0	0.0	0.05	0.0	-8.51e-04	0.0	-5.40e-03
		-0.04	0.0	0.0	0.0	17.5	0.0	-0.10	0.0	-8.51e-04	0.0	-0.01
						35.0	0.0	-0.25	0.0	-8.51e-04	0.0	-0.04
29	1	0.08	0.0	0.0	-6.82	0.0	0.0	3.38	0.0	8.53e-04	0.0	-0.14
		-0.15	0.0	0.0	0.0	13.6	0.0	-0.04	0.0	8.53e-04	0.0	0.08
						27.3	0.0	-3.45	0.0	8.53e-04	0.0	-0.15
29	2	0.06	0.0	0.0	-4.73	0.0	0.0	2.34	0.0	5.68e-04	0.0	-0.10
		-0.11	0.0	0.0	0.0	13.6	0.0	-0.03	0.0	5.68e-04	0.0	0.06
						27.3	0.0	-2.39	0.0	5.68e-04	0.0	-0.11
29	3	0.05	0.0	0.0	-3.99	0.0	0.0	1.97	0.0	4.41e-04	0.0	-0.08
		-0.09	0.0	0.0	0.0	13.6	0.0	-0.02	0.0	4.41e-04	0.0	0.05
						27.3	0.0	-2.02	0.0	4.41e-04	0.0	-0.09
29	4	0.04	0.0	0.0	-3.69	0.0	0.0	1.82	0.0	3.89e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	13.6	0.0	-0.02	0.0	3.89e-04	0.0	0.04
						27.3	0.0	-1.87	0.0	3.89e-04	0.0	-0.08

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
30	1	-0.13	0.0	0.0	-7.78	0.0	0.0	0.42	0.0	0.02	0.0	-0.14
		-1.21	0.0	0.0	0.0	15.5	0.0	-3.47	0.0	0.02	0.0	-0.37
						31.0	0.0	-7.36	0.0	0.02	0.0	-1.21
30	2	-0.09	0.0	0.0	-5.40	0.0	0.0	0.29	0.0	0.01	0.0	-0.09
		-0.84	0.0	0.0	0.0	15.5	0.0	-2.41	0.0	0.01	0.0	-0.26
						31.0	0.0	-5.10	0.0	0.01	0.0	-0.84
30	3	-0.08	0.0	0.0	-4.55	0.0	0.0	0.25	0.0	0.01	0.0	-0.08
		-0.71	0.0	0.0	0.0	15.5	0.0	-2.03	0.0	0.01	0.0	-0.22
						31.0	0.0	-4.30	0.0	0.01	0.0	-0.71
30	4	-0.07	0.0	0.0	-4.21	0.0	0.0	0.23	0.0	0.01	0.0	-0.07
		-0.65	0.0	0.0	0.0	15.5	0.0	-1.87	0.0	0.01	0.0	-0.20
						31.0	0.0	-3.98	0.0	0.01	0.0	-0.65
31	1	0.53	0.0	-1.33e-05	-5.40	0.0	0.0	10.81	0.0	-0.01	0.0	-1.21
		-1.21	0.0	0.0	0.0	10.8	0.0	8.10	0.0	-0.01	0.0	-0.19
						21.5	0.0	5.40	0.0	-0.01	0.0	0.53
31	2	0.37	0.0	-9.20e-06	-3.75	0.0	0.0	7.50	0.0	-8.73e-03	0.0	-0.84
		-0.84	0.0	0.0	0.0	10.8	0.0	5.62	0.0	-8.73e-03	0.0	-0.13
						21.5	0.0	3.75	0.0	-8.73e-03	0.0	0.37
31	3	0.31	0.0	-7.75e-06	-3.16	0.0	0.0	6.32	0.0	-7.34e-03	0.0	-0.71
		-0.71	0.0	0.0	0.0	10.8	0.0	4.74	0.0	-7.34e-03	0.0	-0.11
						21.5	0.0	3.16	0.0	-7.34e-03	0.0	0.31
31	4	0.29	0.0	-7.17e-06	-2.92	0.0	0.0	5.84	0.0	-6.78e-03	0.0	-0.65
		-0.65	0.0	0.0	0.0	10.8	0.0	4.38	0.0	-6.78e-03	0.0	-0.11
						21.5	0.0	2.92	0.0	-6.78e-03	0.0	0.29
32	1	-0.17	0.0	0.0	-6.41	0.0	0.0	7.31	0.0	5.28e-03	0.0	-1.21
		-1.21	0.0	0.0	0.0	12.8	0.0	4.11	0.0	5.28e-03	0.0	-0.48
						25.5	0.0	0.90	0.0	5.28e-03	0.0	-0.17
32	2	-0.11	0.0	0.0	-4.45	0.0	0.0	5.07	0.0	3.67e-03	0.0	-0.84

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.84	0.0	0.0	0.0	12.8	0.0	2.85	0.0	3.67e-03	0.0	-0.34
						25.5	0.0	0.62	0.0	3.67e-03	0.0	-0.11
32	3	-0.10	0.0	0.0	-3.75	0.0	0.0	4.28	0.0	3.11e-03	0.0	-0.71
		-0.71	0.0	0.0	0.0	12.8	0.0	2.40	0.0	3.11e-03	0.0	-0.28
						25.5	0.0	0.53	0.0	3.11e-03	0.0	-0.10
32	4	-0.09	0.0	0.0	-3.47	0.0	0.0	3.96	0.0	2.89e-03	0.0	-0.66
		-0.66	0.0	0.0	0.0	12.8	0.0	2.22	0.0	2.89e-03	0.0	-0.26
						25.5	0.0	0.49	0.0	2.89e-03	0.0	-0.09
33	1	0.02	0.0	0.0	-0.07	0.0	0.0	0.11	0.0	-0.14	0.0	0.01
		0.01	0.0	0.0	0.0	3.0	0.0	0.07	0.0	-0.14	0.0	0.02
						6.1	0.0	0.04	0.0	-0.14	0.0	0.02
33	2	0.01	0.0	0.0	-0.05	0.0	0.0	0.08	0.0	-0.09	0.0	0.01
		0.01	0.0	0.0	0.0	3.0	0.0	0.05	0.0	-0.09	0.0	0.01
						6.1	0.0	0.03	0.0	-0.09	0.0	0.01
33	3	0.01	0.0	0.0	-0.05	0.0	0.0	0.07	0.0	-0.08	0.0	8.17e-03
		8.17e-03	0.0	0.0	0.0	3.0	0.0	0.05	0.0	-0.08	0.0	0.01
						6.1	0.0	0.02	0.0	-0.08	0.0	0.01
33	4	0.01	0.0	0.0	-0.05	0.0	0.0	0.07	0.0	-0.07	0.0	7.39e-03
		7.39e-03	0.0	0.0	0.0	3.0	0.0	0.05	0.0	-0.07	0.0	9.18e-03
						6.1	0.0	0.02	0.0	-0.07	0.0	0.01
34	1	0.19	0.0	-1.36e-06	-8.81	0.0	0.0	4.03	0.0	2.60e-03	0.0	-0.14
		-0.27	0.0	0.0	0.0	17.6	0.0	-0.37	0.0	2.60e-03	0.0	0.18
						35.1	0.0	-4.78	0.0	2.60e-03	0.0	-0.27
34	2	0.13	0.0	0.0	-6.11	0.0	0.0	2.80	0.0	1.91e-03	0.0	-0.09
		-0.19	0.0	0.0	0.0	17.6	0.0	-0.26	0.0	1.91e-03	0.0	0.13
						35.1	0.0	-3.31	0.0	1.91e-03	0.0	-0.19
34	3	0.11	0.0	0.0	-5.15	0.0	0.0	2.35	0.0	1.77e-03	0.0	-0.08
		-0.16	0.0	0.0	0.0	17.6	0.0	-0.22	0.0	1.77e-03	0.0	0.11

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						35.1	0.0	-2.79	0.0	1.77e-03	0.0	-0.16
34	4	0.10	0.0	0.0	-4.76	0.0	0.0	2.18	0.0	1.71e-03	0.0	-0.07
		-0.14	0.0	0.0	0.0	17.6	0.0	-0.20	0.0	1.71e-03	0.0	0.10
						35.1	0.0	-2.58	0.0	1.71e-03	0.0	-0.14
35	1	0.02	0.0	0.0	-0.29	0.0	0.0	2.77	0.0	0.04	0.0	-0.62
		-0.62	0.0	0.0	0.0	12.3	0.0	2.63	0.0	0.04	0.0	-0.29
						24.5	0.0	2.49	0.0	0.04	0.0	0.02
35	2	0.02	0.0	0.0	-0.22	0.0	0.0	2.09	0.0	0.03	0.0	-0.47
		-0.47	0.0	0.0	0.0	12.3	0.0	1.98	0.0	0.03	0.0	-0.22
						24.5	0.0	1.87	0.0	0.03	0.0	0.02
35	3	0.02	0.0	0.0	-0.22	0.0	0.0	1.99	0.0	0.02	0.0	-0.45
		-0.45	0.0	0.0	0.0	12.3	0.0	1.88	0.0	0.02	0.0	-0.21
						24.5	0.0	1.77	0.0	0.02	0.0	0.02
35	4	0.02	0.0	0.0	-0.22	0.0	0.0	1.95	0.0	0.02	0.0	-0.44
		-0.44	0.0	0.0	0.0	12.3	0.0	1.84	0.0	0.02	0.0	-0.20
						24.5	0.0	1.73	0.0	0.02	0.0	0.02
36	1	-4.58e-03	0.0	0.0	-0.25	0.0	0.0	0.37	0.0	-9.62e-03	0.0	-0.06
		-0.06	0.0	0.0	0.0	10.8	0.0	0.24	0.0	-9.62e-03	0.0	-0.02
						21.5	0.0	0.12	0.0	-9.62e-03	0.0	-4.58e-03
36	2	-3.52e-03	0.0	0.0	-0.19	0.0	0.0	0.27	0.0	-6.66e-03	0.0	-0.04
		-0.04	0.0	0.0	0.0	10.8	0.0	0.17	0.0	-6.66e-03	0.0	-0.02
						21.5	0.0	0.08	0.0	-6.66e-03	0.0	-3.52e-03
36	3	-3.51e-03	0.0	0.0	-0.19	0.0	0.0	0.23	0.0	-4.89e-03	0.0	-0.03
		-0.03	0.0	0.0	0.0	10.8	0.0	0.13	0.0	-4.89e-03	0.0	-0.01
						21.5	0.0	0.04	0.0	-4.89e-03	0.0	-3.51e-03
36	4	-3.50e-03	0.0	0.0	-0.19	0.0	0.0	0.22	0.0	-4.19e-03	0.0	-0.03
		-0.03	0.0	0.0	0.0	10.8	0.0	0.12	0.0	-4.19e-03	0.0	-0.01
						21.5	0.0	0.02	0.0	-4.19e-03	0.0	-3.50e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
37	1	5.11e-03	0.0	0.0	-0.30	0.0	0.0	0.14	0.0	8.68e-03	0.0	-3.12e-03
		-5.72e-03	0.0	0.0	0.0	12.8	0.0	-0.01	0.0	8.68e-03	0.0	5.10e-03
						25.5	0.0	-0.16	0.0	8.68e-03	0.0	-5.72e-03
37	2	4.00e-03	0.0	0.0	-0.23	0.0	0.0	0.11	0.0	6.02e-03	0.0	-2.21e-03
		-4.49e-03	0.0	0.0	0.0	12.8	0.0	-8.93e-03	0.0	6.02e-03	0.0	3.97e-03
						25.5	0.0	-0.12	0.0	6.02e-03	0.0	-4.49e-03
37	3	4.18e-03	0.0	0.0	-0.23	0.0	0.0	0.10	0.0	4.56e-03	0.0	-1.77e-03
		-4.63e-03	0.0	0.0	0.0	12.8	0.0	-0.01	0.0	4.56e-03	0.0	4.12e-03
						25.5	0.0	-0.13	0.0	4.56e-03	0.0	-4.63e-03
37	4	4.26e-03	0.0	0.0	-0.23	0.0	0.0	0.10	0.0	3.98e-03	0.0	-1.59e-03
		-4.69e-03	0.0	0.0	0.0	12.8	0.0	-0.01	0.0	3.98e-03	0.0	4.18e-03
						25.5	0.0	-0.13	0.0	3.98e-03	0.0	-4.69e-03
38	1	4.68e-03	0.0	0.0	-0.33	0.0	0.0	0.16	0.0	-0.01	0.0	-6.34e-03
		-8.07e-03	0.0	0.0	0.0	14.3	0.0	-6.09e-03	0.0	-0.01	0.0	4.68e-03
						28.5	0.0	-0.17	0.0	-0.01	0.0	-8.07e-03
38	2	3.61e-03	0.0	0.0	-0.26	0.0	0.0	0.12	0.0	-7.81e-03	0.0	-4.85e-03
		-6.22e-03	0.0	0.0	0.0	14.3	0.0	-4.80e-03	0.0	-7.81e-03	0.0	3.61e-03
						28.5	0.0	-0.13	0.0	-7.81e-03	0.0	-6.22e-03
38	3	3.62e-03	0.0	0.0	-0.26	0.0	0.0	0.12	0.0	-6.77e-03	0.0	-4.80e-03
		-6.23e-03	0.0	0.0	0.0	14.3	0.0	-5.02e-03	0.0	-6.77e-03	0.0	3.62e-03
						28.5	0.0	-0.13	0.0	-6.77e-03	0.0	-6.23e-03
38	4	3.63e-03	0.0	0.0	-0.26	0.0	0.0	0.12	0.0	-6.35e-03	0.0	-4.78e-03
		-6.24e-03	0.0	0.0	0.0	14.3	0.0	-5.11e-03	0.0	-6.35e-03	0.0	3.63e-03
						28.5	0.0	-0.13	0.0	-6.35e-03	0.0	-6.24e-03
39	1	1.02	0.0	8.13e-06	-4.11	0.0	0.0	0.08	0.0	-0.03	0.0	1.02
		0.49	0.0	0.0	0.0	13.5	0.0	-1.97	0.0	-0.03	0.0	0.89
						27.0	0.0	-4.03	0.0	-0.03	0.0	0.49
39	2	0.71	0.0	5.63e-06	-2.85	0.0	0.0	0.05	0.0	-0.02	0.0	0.71

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		0.34	0.0	0.0	0.0	13.5	0.0	-1.37	0.0	-0.02	0.0	0.62
						27.0	0.0	-2.79	0.0	-0.02	0.0	0.34
39	3	0.52	0.0	4.16e-06	-2.11	0.0	0.0	0.04	0.0	-0.02	0.0	0.52
		0.25	0.0	0.0	0.0	13.5	0.0	-1.01	0.0	-0.02	0.0	0.46
						27.0	0.0	-2.07	0.0	-0.02	0.0	0.25
39	4	0.45	0.0	3.58e-06	-1.81	0.0	0.0	0.03	0.0	-0.01	0.0	0.45
		0.21	0.0	0.0	0.0	13.5	0.0	-0.87	0.0	-0.01	0.0	0.39
						27.0	0.0	-1.77	0.0	-0.01	0.0	0.21
40	1	0.61	0.0	1.86e-05	-6.02	0.0	0.0	-6.12	0.0	-0.03	0.0	0.61
		-1.58	0.0	0.0	0.0	12.0	0.0	-9.13	0.0	-0.03	0.0	-0.30
						24.0	0.0	-12.14	0.0	-0.03	0.0	-1.58
40	2	0.43	0.0	1.29e-05	-4.17	0.0	0.0	-4.25	0.0	-0.02	0.0	0.43
		-1.10	0.0	0.0	0.0	12.0	0.0	-6.33	0.0	-0.02	0.0	-0.21
						24.0	0.0	-8.42	0.0	-0.02	0.0	-1.10
40	3	0.36	0.0	1.09e-05	-3.52	0.0	0.0	-3.58	0.0	-0.02	0.0	0.36
		-0.92	0.0	0.0	0.0	12.0	0.0	-5.33	0.0	-0.02	0.0	-0.18
						24.0	0.0	-7.09	0.0	-0.02	0.0	-0.92
40	4	0.33	0.0	1.01e-05	-3.26	0.0	0.0	-3.31	0.0	-0.02	0.0	0.33
		-0.85	0.0	0.0	0.0	12.0	0.0	-4.94	0.0	-0.02	0.0	-0.16
						24.0	0.0	-6.56	0.0	-0.02	0.0	-0.85
41	1	0.49	0.0	1.68e-05	-4.17	0.0	0.0	-4.03	0.0	-0.03	0.0	0.49
		-1.17	0.0	0.0	0.0	13.5	0.0	-6.11	0.0	-0.03	0.0	-0.20
						27.0	0.0	-8.21	0.0	-0.03	0.0	-1.17
41	2	0.34	0.0	1.17e-05	-2.89	0.0	0.0	-2.79	0.0	-0.02	0.0	0.34
		-0.81	0.0	0.0	0.0	13.5	0.0	-4.23	0.0	-0.02	0.0	-0.14
						27.0	0.0	-5.68	0.0	-0.02	0.0	-0.81
41	3	0.25	0.0	8.62e-06	-2.14	0.0	0.0	-2.07	0.0	-0.02	0.0	0.25
		-0.60	0.0	0.0	0.0	13.5	0.0	-3.13	0.0	-0.02	0.0	-0.10

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						27.0	0.0	-4.20	0.0	-0.02	0.0	-0.60
41	4	0.21	0.0	7.40e-06	-1.83	0.0	0.0	-1.77	0.0	-0.01	0.0	0.21
		-0.51	0.0	0.0	0.0	13.5	0.0	-2.69	0.0	-0.01	0.0	-0.09
						27.0	0.0	-3.61	0.0	-0.01	0.0	-0.51
42	1	0.02	0.0	0.0	-0.23	0.0	0.0	-0.28	0.0	-4.98e-04	0.0	0.02
		-0.06	0.0	0.0	0.0	10.3	0.0	-0.40	0.0	-4.98e-04	0.0	-0.01
						20.5	0.0	-0.51	0.0	-4.98e-04	0.0	-0.06
42	2	0.02	0.0	0.0	-0.18	0.0	0.0	-0.21	0.0	-3.17e-04	0.0	0.02
		-0.04	0.0	0.0	0.0	10.3	0.0	-0.30	0.0	-3.17e-04	0.0	-8.48e-03
						20.5	0.0	-0.39	0.0	-3.17e-04	0.0	-0.04
42	3	0.02	0.0	0.0	-0.18	0.0	0.0	-0.21	0.0	-2.25e-04	0.0	0.02
		-0.04	0.0	0.0	0.0	10.3	0.0	-0.30	0.0	-2.25e-04	0.0	-8.24e-03
						20.5	0.0	-0.38	0.0	-2.25e-04	0.0	-0.04
42	4	0.02	0.0	0.0	-0.18	0.0	0.0	-0.21	0.0	-1.88e-04	0.0	0.02
		-0.04	0.0	0.0	0.0	10.3	0.0	-0.29	0.0	-1.88e-04	0.0	-8.15e-03
						20.5	0.0	-0.38	0.0	-1.88e-04	0.0	-0.04
43	1	0.11	0.0	0.0	-0.49	0.0	0.0	-0.02	0.0	9.74e-04	0.0	0.11
		0.05	0.0	0.0	0.0	11.8	0.0	-0.26	0.0	9.74e-04	0.0	0.10
						23.5	0.0	-0.51	0.0	9.74e-04	0.0	0.05
43	2	0.08	0.0	0.0	-0.36	0.0	0.0	-0.01	0.0	7.47e-04	0.0	0.08
		0.04	0.0	0.0	0.0	11.8	0.0	-0.19	0.0	7.47e-04	0.0	0.07
						23.5	0.0	-0.37	0.0	7.47e-04	0.0	0.04
43	3	0.08	0.0	0.0	-0.33	0.0	0.0	-0.01	0.0	7.39e-04	0.0	0.08
		0.04	0.0	0.0	0.0	11.8	0.0	-0.18	0.0	7.39e-04	0.0	0.07
						23.5	0.0	-0.34	0.0	7.39e-04	0.0	0.04
43	4	0.07	0.0	0.0	-0.32	0.0	0.0	-0.01	0.0	7.36e-04	0.0	0.07
		0.03	0.0	0.0	0.0	11.8	0.0	-0.17	0.0	7.36e-04	0.0	0.06
						23.5	0.0	-0.33	0.0	7.36e-04	0.0	0.03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
44	1	1.48	0.0	1.05e-05	-6.34	0.0	0.0	0.04	0.0	4.39e-03	0.0	1.48
		0.69	0.0	0.0	0.0	12.5	0.0	-3.13	0.0	4.39e-03	0.0	1.28
						25.1	0.0	-6.30	0.0	4.39e-03	0.0	0.69
44	2	1.03	0.0	7.29e-06	-4.40	0.0	0.0	0.03	0.0	3.02e-03	0.0	1.03
		0.48	0.0	0.0	0.0	12.5	0.0	-2.17	0.0	3.02e-03	0.0	0.89
						25.1	0.0	-4.37	0.0	3.02e-03	0.0	0.48
44	3	0.86	0.0	6.14e-06	-3.71	0.0	0.0	0.02	0.0	2.51e-03	0.0	0.86
		0.41	0.0	0.0	0.0	12.5	0.0	-1.83	0.0	2.51e-03	0.0	0.75
						25.1	0.0	-3.68	0.0	2.51e-03	0.0	0.41
44	4	0.80	0.0	5.68e-06	-3.43	0.0	0.0	0.02	0.0	2.30e-03	0.0	0.80
		0.38	0.0	0.0	0.0	12.5	0.0	-1.69	0.0	2.30e-03	0.0	0.69
						25.1	0.0	-3.41	0.0	2.30e-03	0.0	0.38
45	1	1.36	0.0	9.14e-06	-6.06	0.0	0.0	-0.06	0.0	-0.03	0.0	1.36
		0.61	0.0	0.0	0.0	12.0	0.0	-3.10	0.0	-0.03	0.0	1.17
						24.0	0.0	-6.12	0.0	-0.03	0.0	0.61
45	2	0.94	0.0	6.34e-06	-4.20	0.0	0.0	-0.04	0.0	-0.02	0.0	0.94
		0.43	0.0	0.0	0.0	12.0	0.0	-2.15	0.0	-0.02	0.0	0.81
						24.0	0.0	-4.25	0.0	-0.02	0.0	0.43
45	3	0.79	0.0	5.34e-06	-3.54	0.0	0.0	-0.04	0.0	-0.02	0.0	0.79
		0.36	0.0	0.0	0.0	12.0	0.0	-1.81	0.0	-0.02	0.0	0.68
						24.0	0.0	-3.58	0.0	-0.02	0.0	0.36
45	4	0.73	0.0	4.95e-06	-3.27	0.0	0.0	-0.03	0.0	-0.02	0.0	0.73
		0.33	0.0	0.0	0.0	12.0	0.0	-1.68	0.0	-0.02	0.0	0.63
						24.0	0.0	-3.31	0.0	-0.02	0.0	0.33
46	1	0.05	0.0	0.0	-0.28	0.0	0.0	-7.21e-03	0.0	-4.98e-04	0.0	0.05
		0.02	0.0	0.0	0.0	10.3	0.0	-0.14	0.0	-4.98e-04	0.0	0.05
						20.5	0.0	-0.28	0.0	-4.98e-04	0.0	0.02
46	2	0.04	0.0	0.0	-0.21	0.0	0.0	-5.17e-03	0.0	-3.17e-04	0.0	0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		0.02	0.0	0.0	0.0	10.3	0.0	-0.11	0.0	-3.17e-04	0.0	0.03
						20.5	0.0	-0.21	0.0	-3.17e-04	0.0	0.02
46	3	0.04	0.0	0.0	-0.20	0.0	0.0	-4.61e-03	0.0	-2.25e-04	0.0	0.04
		0.02	0.0	0.0	0.0	10.3	0.0	-0.11	0.0	-2.25e-04	0.0	0.03
						20.5	0.0	-0.21	0.0	-2.25e-04	0.0	0.02
46	4	0.04	0.0	0.0	-0.20	0.0	0.0	-4.39e-03	0.0	-1.88e-04	0.0	0.04
		0.02	0.0	0.0	0.0	10.3	0.0	-0.10	0.0	-1.88e-04	0.0	0.03
						20.5	0.0	-0.21	0.0	-1.88e-04	0.0	0.02
47	1	0.05	0.0	0.0	-0.25	0.0	0.0	1.79e-03	0.0	-5.45e-04	0.0	0.05
		0.02	0.0	0.0	0.0	10.7	0.0	-0.12	0.0	-5.45e-04	0.0	0.04
						21.4	0.0	-0.25	0.0	-5.45e-04	0.0	0.02
47	2	0.04	0.0	0.0	-0.19	0.0	0.0	1.22e-03	0.0	-3.49e-04	0.0	0.04
		0.02	0.0	0.0	0.0	10.7	0.0	-0.09	0.0	-3.49e-04	0.0	0.03
						21.4	0.0	-0.19	0.0	-3.49e-04	0.0	0.02
47	3	0.04	0.0	0.0	-0.19	0.0	0.0	1.00e-03	0.0	-2.50e-04	0.0	0.04
		0.02	0.0	0.0	0.0	10.7	0.0	-0.09	0.0	-2.50e-04	0.0	0.03
						21.4	0.0	-0.19	0.0	-2.50e-04	0.0	0.02
47	4	0.04	0.0	0.0	-0.19	0.0	0.0	9.13e-04	0.0	-2.10e-04	0.0	0.04
		0.02	0.0	0.0	0.0	10.7	0.0	-0.09	0.0	-2.10e-04	0.0	0.03
						21.4	0.0	-0.19	0.0	-2.10e-04	0.0	0.02
48	1	1.11	0.0	6.23e-06	-5.41	0.0	0.0	-1.83e-03	0.0	-0.01	0.0	1.11
		0.53	0.0	0.0	0.0	10.8	0.0	-2.70	0.0	-0.01	0.0	0.97
						21.5	0.0	-5.41	0.0	-0.01	0.0	0.53
48	2	0.77	0.0	4.32e-06	-3.75	0.0	0.0	-1.26e-03	0.0	-8.73e-03	0.0	0.77
		0.37	0.0	0.0	0.0	10.8	0.0	-1.88	0.0	-8.73e-03	0.0	0.67
						21.5	0.0	-3.75	0.0	-8.73e-03	0.0	0.37
48	3	0.65	0.0	3.64e-06	-3.16	0.0	0.0	-1.06e-03	0.0	-7.34e-03	0.0	0.65
		0.31	0.0	0.0	0.0	10.8	0.0	-1.58	0.0	-7.34e-03	0.0	0.57

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						21.5	0.0	-3.16	0.0	-7.34e-03	0.0	0.31
48	4	0.60	0.0	3.37e-06	-2.92	0.0	0.0	-9.75e-04	0.0	-6.78e-03	0.0	0.60
		0.29	0.0	0.0	0.0	10.8	0.0	-1.46	0.0	-6.78e-03	0.0	0.52
						21.5	0.0	-2.92	0.0	-6.78e-03	0.0	0.29
49	1	0.02	0.0	0.0	-0.25	0.0	0.0	-0.25	0.0	-5.45e-04	0.0	0.02
		-0.06	0.0	0.0	0.0	10.7	0.0	-0.37	0.0	-5.45e-04	0.0	-9.17e-03
						21.4	0.0	-0.50	0.0	-5.45e-04	0.0	-0.06
49	2	0.02	0.0	0.0	-0.19	0.0	0.0	-0.19	0.0	-3.49e-04	0.0	0.02
		-0.04	0.0	0.0	0.0	10.7	0.0	-0.29	0.0	-3.49e-04	0.0	-7.02e-03
						21.4	0.0	-0.38	0.0	-3.49e-04	0.0	-0.04
49	3	0.02	0.0	0.0	-0.19	0.0	0.0	-0.19	0.0	-2.50e-04	0.0	0.02
		-0.04	0.0	0.0	0.0	10.7	0.0	-0.28	0.0	-2.50e-04	0.0	-6.98e-03
						21.4	0.0	-0.38	0.0	-2.50e-04	0.0	-0.04
49	4	0.02	0.0	0.0	-0.19	0.0	0.0	-0.19	0.0	-2.10e-04	0.0	0.02
		-0.04	0.0	0.0	0.0	10.7	0.0	-0.28	0.0	-2.10e-04	0.0	-6.96e-03
						21.4	0.0	-0.38	0.0	-2.10e-04	0.0	-0.04
50	1	1.02	0.0	-8.42e-06	-4.05	0.0	0.0	4.13	0.0	-0.03	0.0	0.45
		0.45	0.0	0.0	0.0	13.5	0.0	2.11	0.0	-0.03	0.0	0.87
						27.0	0.0	0.08	0.0	-0.03	0.0	1.02
50	2	0.71	0.0	-5.83e-06	-2.81	0.0	0.0	2.86	0.0	-0.02	0.0	0.31
		0.31	0.0	0.0	0.0	13.5	0.0	1.46	0.0	-0.02	0.0	0.61
						27.0	0.0	0.05	0.0	-0.02	0.0	0.71
50	3	0.52	0.0	-4.31e-06	-2.08	0.0	0.0	2.11	0.0	-0.02	0.0	0.23
		0.23	0.0	0.0	0.0	13.5	0.0	1.08	0.0	-0.02	0.0	0.45
						27.0	0.0	0.04	0.0	-0.02	0.0	0.52
50	4	0.45	0.0	-3.70e-06	-1.78	0.0	0.0	1.82	0.0	-0.01	0.0	0.20
		0.20	0.0	0.0	0.0	13.5	0.0	0.93	0.0	-0.01	0.0	0.38
						27.0	0.0	0.03	0.0	-0.01	0.0	0.45

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
51	1	0.53	0.0	1.33e-05	-5.41	0.0	0.0	-5.41	0.0	-0.01	0.0	0.53
		-1.21	0.0	0.0	0.0	10.8	0.0	-8.11	0.0	-0.01	0.0	-0.20
						21.5	0.0	-10.81	0.0	-0.01	0.0	-1.21
51	2	0.37	0.0	9.20e-06	-3.75	0.0	0.0	-3.75	0.0	-8.73e-03	0.0	0.37
		-0.84	0.0	0.0	0.0	10.8	0.0	-5.63	0.0	-8.73e-03	0.0	-0.14
						21.5	0.0	-7.50	0.0	-8.73e-03	0.0	-0.84
51	3	0.31	0.0	7.75e-06	-3.16	0.0	0.0	-3.16	0.0	-7.34e-03	0.0	0.31
		-0.71	0.0	0.0	0.0	10.8	0.0	-4.74	0.0	-7.34e-03	0.0	-0.11
						21.5	0.0	-6.32	0.0	-7.34e-03	0.0	-0.71
51	4	0.29	0.0	7.17e-06	-2.92	0.0	0.0	-2.92	0.0	-6.78e-03	0.0	0.29
		-0.66	0.0	0.0	0.0	10.8	0.0	-4.39	0.0	-6.78e-03	0.0	-0.11
						21.5	0.0	-5.85	0.0	-6.78e-03	0.0	-0.66
52	1	0.05	0.0	1.52e-06	-0.48	0.0	0.0	-0.51	0.0	9.74e-04	0.0	0.05
		-0.12	0.0	0.0	0.0	11.8	0.0	-0.75	0.0	9.74e-04	0.0	-0.02
						23.5	0.0	-0.98	0.0	9.74e-04	0.0	-0.12
52	2	0.04	0.0	1.11e-06	-0.35	0.0	0.0	-0.37	0.0	7.47e-04	0.0	0.04
		-0.09	0.0	0.0	0.0	11.8	0.0	-0.55	0.0	7.47e-04	0.0	-0.02
						23.5	0.0	-0.72	0.0	7.47e-04	0.0	-0.09
52	3	0.04	0.0	1.03e-06	-0.33	0.0	0.0	-0.34	0.0	7.39e-04	0.0	0.04
		-0.08	0.0	0.0	0.0	11.8	0.0	-0.51	0.0	7.39e-04	0.0	-0.01
						23.5	0.0	-0.67	0.0	7.39e-04	0.0	-0.08
52	4	0.03	0.0	0.0	-0.32	0.0	0.0	-0.33	0.0	7.36e-04	0.0	0.03
		-0.08	0.0	0.0	0.0	11.8	0.0	-0.49	0.0	7.36e-04	0.0	-0.01
						23.5	0.0	-0.65	0.0	7.36e-04	0.0	-0.08
53	1	0.69	0.0	2.17e-05	-6.35	0.0	0.0	-6.30	0.0	4.39e-03	0.0	0.69
		-1.68	0.0	0.0	0.0	12.5	0.0	-9.47	0.0	4.39e-03	0.0	-0.30
						25.1	0.0	-12.65	0.0	4.39e-03	0.0	-1.68
53	2	0.48	0.0	1.51e-05	-4.40	0.0	0.0	-4.37	0.0	3.02e-03	0.0	0.48

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-1.17	0.0	0.0	0.0	12.5	0.0	-6.57	0.0	3.02e-03	0.0	-0.20
						25.1	0.0	-8.78	0.0	3.02e-03	0.0	-1.17
53	3	0.41	0.0	1.27e-05	-3.71	0.0	0.0	-3.68	0.0	2.51e-03	0.0	0.41
		-0.98	0.0	0.0	0.0	12.5	0.0	-5.54	0.0	2.51e-03	0.0	-0.17
						25.1	0.0	-7.39	0.0	2.51e-03	0.0	-0.98
53	4	0.38	0.0	1.17e-05	-3.43	0.0	0.0	-3.41	0.0	2.30e-03	0.0	0.38
		-0.91	0.0	0.0	0.0	12.5	0.0	-5.12	0.0	2.30e-03	0.0	-0.16
						25.1	0.0	-6.84	0.0	2.30e-03	0.0	-0.91
54	1	0.11	0.0	0.0	-0.50	0.0	0.0	0.48	0.0	9.74e-04	0.0	0.06
		0.06	0.0	0.0	0.0	11.8	0.0	0.23	0.0	9.74e-04	0.0	0.10
						23.5	0.0	-0.02	0.0	9.74e-04	0.0	0.11
54	2	0.08	0.0	0.0	-0.37	0.0	0.0	0.35	0.0	7.47e-04	0.0	0.04
		0.04	0.0	0.0	0.0	11.8	0.0	0.17	0.0	7.47e-04	0.0	0.07
						23.5	0.0	-0.01	0.0	7.47e-04	0.0	0.08
54	3	0.08	0.0	0.0	-0.34	0.0	0.0	0.33	0.0	7.39e-04	0.0	0.04
		0.04	0.0	0.0	0.0	11.8	0.0	0.16	0.0	7.39e-04	0.0	0.07
						23.5	0.0	-0.01	0.0	7.39e-04	0.0	0.08
54	4	0.07	0.0	0.0	-0.33	0.0	0.0	0.32	0.0	7.36e-04	0.0	0.04
		0.04	0.0	0.0	0.0	11.8	0.0	0.15	0.0	7.36e-04	0.0	0.07
						23.5	0.0	-0.01	0.0	7.36e-04	0.0	0.07
55	1	1.48	0.0	-1.06e-05	-6.33	0.0	0.0	6.38	0.0	4.39e-03	0.0	0.67
		0.67	0.0	0.0	0.0	12.5	0.0	3.21	0.0	4.39e-03	0.0	1.27
						25.1	0.0	0.04	0.0	4.39e-03	0.0	1.48
55	2	1.03	0.0	-7.38e-06	-4.39	0.0	0.0	4.42	0.0	3.02e-03	0.0	0.47
		0.47	0.0	0.0	0.0	12.5	0.0	2.23	0.0	3.02e-03	0.0	0.88
						25.1	0.0	0.03	0.0	3.02e-03	0.0	1.03
55	3	0.86	0.0	-6.22e-06	-3.70	0.0	0.0	3.73	0.0	2.51e-03	0.0	0.39
		0.39	0.0	0.0	0.0	12.5	0.0	1.88	0.0	2.51e-03	0.0	0.74

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						25.1	0.0	0.02	0.0	2.51e-03	0.0	0.86
55	4	0.80	0.0	-5.75e-06	-3.43	0.0	0.0	3.45	0.0	2.30e-03	0.0	0.36
		0.36	0.0	0.0	0.0	12.5	0.0	1.74	0.0	2.30e-03	0.0	0.69
						25.1	0.0	0.02	0.0	2.30e-03	0.0	0.80
56	1	1.36	0.0	-8.96e-06	-6.06	0.0	0.0	5.99	0.0	-0.03	0.0	0.64
		0.64	0.0	0.0	0.0	12.0	0.0	2.97	0.0	-0.03	0.0	1.18
						24.0	0.0	-0.06	0.0	-0.03	0.0	1.36
56	2	0.94	0.0	-6.21e-06	-4.20	0.0	0.0	4.16	0.0	-0.02	0.0	0.45
		0.45	0.0	0.0	0.0	12.0	0.0	2.06	0.0	-0.02	0.0	0.82
						24.0	0.0	-0.04	0.0	-0.02	0.0	0.94
56	3	0.79	0.0	-5.24e-06	-3.54	0.0	0.0	3.50	0.0	-0.02	0.0	0.38
		0.38	0.0	0.0	0.0	12.0	0.0	1.73	0.0	-0.02	0.0	0.69
						24.0	0.0	-0.04	0.0	-0.02	0.0	0.79
56	4	0.73	0.0	-4.85e-06	-3.28	0.0	0.0	3.24	0.0	-0.02	0.0	0.35
		0.35	0.0	0.0	0.0	12.0	0.0	1.60	0.0	-0.02	0.0	0.64
						24.0	0.0	-0.03	0.0	-0.02	0.0	0.73
57	1	0.05	0.0	0.0	-0.27	0.0	0.0	0.27	0.0	-4.98e-04	0.0	0.03
		0.03	0.0	0.0	0.0	10.3	0.0	0.13	0.0	-4.98e-04	0.0	0.05
						20.5	0.0	-7.21e-03	0.0	-4.98e-04	0.0	0.05
57	2	0.04	0.0	0.0	-0.21	0.0	0.0	0.20	0.0	-3.17e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.3	0.0	0.10	0.0	-3.17e-04	0.0	0.04
						20.5	0.0	-5.17e-03	0.0	-3.17e-04	0.0	0.04
57	3	0.04	0.0	0.0	-0.20	0.0	0.0	0.20	0.0	-2.25e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.3	0.0	0.10	0.0	-2.25e-04	0.0	0.03
						20.5	0.0	-4.61e-03	0.0	-2.25e-04	0.0	0.04
57	4	0.04	0.0	0.0	-0.20	0.0	0.0	0.20	0.0	-1.88e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.3	0.0	0.10	0.0	-1.88e-04	0.0	0.03
						20.5	0.0	-4.39e-03	0.0	-1.88e-04	0.0	0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
58	1	0.04	0.0	0.0	-3.89	0.0	0.0	1.93	0.0	1.09e-04	0.0	-0.09
		-0.09	0.0	0.0	0.0	13.6	0.0	-3.16e-03	0.0	1.09e-04	0.0	0.04
						27.3	0.0	-1.95	0.0	1.09e-04	0.0	-0.09
58	2	0.03	0.0	0.0	-2.70	0.0	0.0	1.34	0.0	9.53e-05	0.0	-0.06
		-0.06	0.0	0.0	0.0	13.6	0.0	-2.18e-03	0.0	9.53e-05	0.0	0.03
						27.3	0.0	-1.35	0.0	9.53e-05	0.0	-0.06
58	3	0.02	0.0	0.0	-2.00	0.0	0.0	0.99	0.0	1.25e-04	0.0	-0.05
		-0.05	0.0	0.0	0.0	13.6	0.0	-1.57e-03	0.0	1.25e-04	0.0	0.02
						27.3	0.0	-1.00	0.0	1.25e-04	0.0	-0.05
58	4	0.02	0.0	0.0	-1.72	0.0	0.0	0.85	0.0	1.37e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	13.6	0.0	-1.33e-03	0.0	1.37e-04	0.0	0.02
						27.3	0.0	-0.86	0.0	1.37e-04	0.0	-0.04
59	1	0.05	0.0	0.0	-3.83	0.0	0.0	1.84	0.0	1.09e-04	0.0	-0.07
		-0.09	0.0	0.0	0.0	13.6	0.0	-0.07	0.0	1.09e-04	0.0	0.05
						27.3	0.0	-1.99	0.0	1.09e-04	0.0	-0.09
59	2	0.04	0.0	0.0	-2.66	0.0	0.0	1.28	0.0	9.53e-05	0.0	-0.05
		-0.06	0.0	0.0	0.0	13.6	0.0	-0.04	0.0	9.53e-05	0.0	0.04
						27.3	0.0	-1.38	0.0	9.53e-05	0.0	-0.06
59	3	0.03	0.0	0.0	-1.97	0.0	0.0	0.95	0.0	1.25e-04	0.0	-0.04
		-0.05	0.0	0.0	0.0	13.6	0.0	-0.03	0.0	1.25e-04	0.0	0.03
						27.3	0.0	-1.02	0.0	1.25e-04	0.0	-0.05
59	4	0.02	0.0	0.0	-1.69	0.0	0.0	0.82	0.0	1.37e-04	0.0	-0.03
		-0.04	0.0	0.0	0.0	13.6	0.0	-0.03	0.0	1.37e-04	0.0	0.02
						27.3	0.0	-0.88	0.0	1.37e-04	0.0	-0.04
60	1	0.02	0.0	0.0	-3.94	0.0	0.0	1.76	0.0	1.09e-04	0.0	-0.09
		-0.15	0.0	0.0	0.0	13.6	0.0	-0.20	0.0	1.09e-04	0.0	0.02
						27.3	0.0	-2.18	0.0	1.09e-04	0.0	-0.15
60	2	0.01	0.0	0.0	-2.73	0.0	0.0	1.22	0.0	9.53e-05	0.0	-0.06

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.10	0.0	0.0	0.0	13.6	0.0	-0.14	0.0	9.53e-05	0.0	0.01
						27.3	0.0	-1.51	0.0	9.53e-05	0.0	-0.10
60	3	9.59e-03	0.0	0.0	-2.02	0.0	0.0	0.90	0.0	1.25e-04	0.0	-0.05
		-0.07	0.0	0.0	0.0	13.6	0.0	-0.10	0.0	1.25e-04	0.0	8.90e-03
						27.3	0.0	-1.12	0.0	1.25e-04	0.0	-0.07
60	4	8.26e-03	0.0	0.0	-1.74	0.0	0.0	0.78	0.0	1.37e-04	0.0	-0.04
		-0.06	0.0	0.0	0.0	13.6	0.0	-0.09	0.0	1.37e-04	0.0	7.68e-03
						27.3	0.0	-0.96	0.0	1.37e-04	0.0	-0.06
61	1	0.20	0.0	-1.65e-06	-7.07	0.0	0.0	3.32	0.0	0.04	0.0	-0.15
		-0.23	0.0	0.0	0.0	22.1	0.0	-0.20	0.0	0.04	0.0	0.20
						44.1	0.0	-3.74	0.0	0.04	0.0	-0.23
61	2	0.14	0.0	-1.14e-06	-4.89	0.0	0.0	2.30	0.0	0.03	0.0	-0.10
		-0.16	0.0	0.0	0.0	22.1	0.0	-0.14	0.0	0.03	0.0	0.14
						44.1	0.0	-2.59	0.0	0.03	0.0	-0.16
61	3	0.10	0.0	0.0	-3.61	0.0	0.0	1.70	0.0	0.02	0.0	-0.07
		-0.12	0.0	0.0	0.0	22.1	0.0	-0.10	0.0	0.02	0.0	0.10
						44.1	0.0	-1.91	0.0	0.02	0.0	-0.12
61	4	0.09	0.0	0.0	-3.10	0.0	0.0	1.46	0.0	0.02	0.0	-0.06
		-0.10	0.0	0.0	0.0	22.1	0.0	-0.09	0.0	0.02	0.0	0.09
						44.1	0.0	-1.64	0.0	0.02	0.0	-0.10
62	1	0.09	0.0	0.0	-6.08	0.0	0.0	3.03	0.0	-0.02	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.4	0.0	-2.28e-03	0.0	-0.02	0.0	0.09
						34.8	0.0	-3.05	0.0	-0.02	0.0	-0.18
62	2	0.06	0.0	0.0	-4.21	0.0	0.0	2.10	0.0	-0.01	0.0	-0.12
		-0.12	0.0	0.0	0.0	17.4	0.0	-1.56e-03	0.0	-0.01	0.0	0.06
						34.8	0.0	-2.11	0.0	-0.01	0.0	-0.12
62	3	0.04	0.0	0.0	-3.10	0.0	0.0	1.54	0.0	-0.01	0.0	-0.09
		-0.09	0.0	0.0	0.0	17.4	0.0	-1.12e-03	0.0	-0.01	0.0	0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						34.8	0.0	-1.55	0.0	-0.01	0.0	-0.09
62	4	0.04	0.0	0.0	-2.65	0.0	0.0	1.32	0.0	-0.01	0.0	-0.08
		-0.08	0.0	0.0	0.0	17.4	0.0	-9.42e-04	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.33	0.0	-0.01	0.0	-0.08
63	1	0.09	0.0	0.0	-5.97	0.0	0.0	2.98	0.0	-0.02	0.0	-0.17
		-0.17	0.0	0.0	0.0	17.4	0.0	-6.73e-04	0.0	-0.02	0.0	0.09
						34.8	0.0	-2.99	0.0	-0.02	0.0	-0.17
63	2	0.06	0.0	0.0	-4.13	0.0	0.0	2.06	0.0	-0.01	0.0	-0.12
		-0.12	0.0	0.0	0.0	17.4	0.0	-4.57e-04	0.0	-0.01	0.0	0.06
						34.8	0.0	-2.07	0.0	-0.01	0.0	-0.12
63	3	0.04	0.0	0.0	-3.04	0.0	0.0	1.52	0.0	-0.01	0.0	-0.09
		-0.09	0.0	0.0	0.0	17.4	0.0	-3.11e-04	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.53	0.0	-0.01	0.0	-0.09
63	4	0.04	0.0	0.0	-2.61	0.0	0.0	1.30	0.0	-0.01	0.0	-0.08
		-0.08	0.0	0.0	0.0	17.4	0.0	-2.53e-04	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.31	0.0	-0.01	0.0	-0.08
64	1	0.09	0.0	0.0	-6.19	0.0	0.0	3.09	0.0	-0.02	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.4	0.0	3.99e-03	0.0	-0.02	0.0	0.09
						34.8	0.0	-3.10	0.0	-0.02	0.0	-0.18
64	2	0.06	0.0	0.0	-4.28	0.0	0.0	2.14	0.0	-0.01	0.0	-0.12
		-0.12	0.0	0.0	0.0	17.4	0.0	2.87e-03	0.0	-0.01	0.0	0.06
						34.8	0.0	-2.14	0.0	-0.01	0.0	-0.12
64	3	0.05	0.0	0.0	-3.15	0.0	0.0	1.58	0.0	-0.01	0.0	-0.09
		-0.09	0.0	0.0	0.0	17.4	0.0	2.16e-03	0.0	-0.01	0.0	0.05
						34.8	0.0	-1.58	0.0	-0.01	0.0	-0.09
64	4	0.04	0.0	0.0	-2.70	0.0	0.0	1.35	0.0	-0.01	0.0	-0.08
		-0.08	0.0	0.0	0.0	17.4	0.0	1.87e-03	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.35	0.0	-0.01	0.0	-0.08

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
65	1	0.09	0.0	0.0	-5.91	0.0	0.0	2.92	0.0	-0.02	0.0	-0.16
		-0.17	0.0	0.0	0.0	17.4	0.0	-0.03	0.0	-0.02	0.0	0.09
						34.8	0.0	-2.99	0.0	-0.02	0.0	-0.17
65	2	0.06	0.0	0.0	-4.09	0.0	0.0	2.02	0.0	-0.01	0.0	-0.11
		-0.12	0.0	0.0	0.0	17.4	0.0	-0.02	0.0	-0.01	0.0	0.06
						34.8	0.0	-2.07	0.0	-0.01	0.0	-0.12
65	3	0.05	0.0	0.0	-3.02	0.0	0.0	1.49	0.0	-0.01	0.0	-0.08
		-0.09	0.0	0.0	0.0	17.4	0.0	-0.01	0.0	-0.01	0.0	0.05
						34.8	0.0	-1.52	0.0	-0.01	0.0	-0.09
65	4	0.04	0.0	0.0	-2.58	0.0	0.0	1.28	0.0	-0.01	0.0	-0.07
		-0.08	0.0	0.0	0.0	17.4	0.0	-0.01	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.31	0.0	-0.01	0.0	-0.08
66	1	0.09	0.0	0.0	-6.14	0.0	0.0	3.06	0.0	-0.02	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.4	0.0	-2.76e-03	0.0	-0.02	0.0	0.09
						34.8	0.0	-3.08	0.0	-0.02	0.0	-0.18
66	2	0.06	0.0	0.0	-4.25	0.0	0.0	2.12	0.0	-0.01	0.0	-0.12
		-0.12	0.0	0.0	0.0	17.4	0.0	-1.91e-03	0.0	-0.01	0.0	0.06
						34.8	0.0	-2.13	0.0	-0.01	0.0	-0.12
66	3	0.05	0.0	0.0	-3.13	0.0	0.0	1.56	0.0	-0.01	0.0	-0.09
		-0.09	0.0	0.0	0.0	17.4	0.0	-1.37e-03	0.0	-0.01	0.0	0.05
						34.8	0.0	-1.57	0.0	-0.01	0.0	-0.09
66	4	0.04	0.0	0.0	-2.68	0.0	0.0	1.33	0.0	-0.01	0.0	-0.08
		-0.08	0.0	0.0	0.0	17.4	0.0	-1.16e-03	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.34	0.0	-0.01	0.0	-0.08
67	1	0.09	0.0	0.0	-6.03	0.0	0.0	3.00	0.0	-0.02	0.0	-0.17
		-0.18	0.0	0.0	0.0	17.4	0.0	-2.44e-03	0.0	-0.02	0.0	0.09
						34.8	0.0	-3.02	0.0	-0.02	0.0	-0.18
67	2	0.06	0.0	0.0	-4.17	0.0	0.0	2.08	0.0	-0.01	0.0	-0.12

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.12	0.0	0.0	0.0	17.4	0.0	-1.67e-03	0.0	-0.01	0.0	0.06
						34.8	0.0	-2.09	0.0	-0.01	0.0	-0.12
67	3	0.04	0.0	0.0	-3.07	0.0	0.0	1.53	0.0	-0.01	0.0	-0.09
		-0.09	0.0	0.0	0.0	17.4	0.0	-1.20e-03	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.54	0.0	-0.01	0.0	-0.09
67	4	0.04	0.0	0.0	-2.63	0.0	0.0	1.31	0.0	-0.01	0.0	-0.08
		-0.08	0.0	0.0	0.0	17.4	0.0	-1.01e-03	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.32	0.0	-0.01	0.0	-0.08
68	1	0.08	0.0	0.0	-6.25	0.0	0.0	3.03	0.0	-0.02	0.0	-0.18
		-0.21	0.0	0.0	0.0	17.4	0.0	-0.09	0.0	-0.02	0.0	0.08
						34.8	0.0	-3.22	0.0	-0.02	0.0	-0.21
68	2	0.05	0.0	0.0	-4.32	0.0	0.0	2.09	0.0	-0.01	0.0	-0.12
		-0.15	0.0	0.0	0.0	17.4	0.0	-0.06	0.0	-0.01	0.0	0.05
						34.8	0.0	-2.23	0.0	-0.01	0.0	-0.15
68	3	0.04	0.0	0.0	-3.18	0.0	0.0	1.54	0.0	-0.01	0.0	-0.09
		-0.11	0.0	0.0	0.0	17.4	0.0	-0.05	0.0	-0.01	0.0	0.04
						34.8	0.0	-1.64	0.0	-0.01	0.0	-0.11
68	4	0.03	0.0	0.0	-2.72	0.0	0.0	1.32	0.0	-0.01	0.0	-0.08
		-0.09	0.0	0.0	0.0	17.4	0.0	-0.04	0.0	-0.01	0.0	0.03
						34.8	0.0	-1.41	0.0	-0.01	0.0	-0.09
69	1	0.08	0.0	0.0	-6.80	0.0	0.0	3.40	0.0	8.53e-04	0.0	-0.15
		-0.15	0.0	0.0	0.0	13.6	0.0	2.84e-03	0.0	8.53e-04	0.0	0.08
						27.3	0.0	-3.39	0.0	8.53e-04	0.0	-0.15
69	2	0.05	0.0	0.0	-4.72	0.0	0.0	2.36	0.0	5.68e-04	0.0	-0.11
		-0.11	0.0	0.0	0.0	13.6	0.0	1.97e-03	0.0	5.68e-04	0.0	0.05
						27.3	0.0	-2.35	0.0	5.68e-04	0.0	-0.11
69	3	0.05	0.0	0.0	-3.97	0.0	0.0	1.99	0.0	4.41e-04	0.0	-0.09
		-0.09	0.0	0.0	0.0	13.6	0.0	1.65e-03	0.0	4.41e-04	0.0	0.05

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						27.3	0.0	-1.98	0.0	4.41e-04	0.0	-0.09
69	4	0.04	0.0	0.0	-3.68	0.0	0.0	1.84	0.0	3.89e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	13.6	0.0	1.53e-03	0.0	3.89e-04	0.0	0.04
						27.3	0.0	-1.84	0.0	3.89e-04	0.0	-0.08
70	1	0.08	0.0	0.0	-6.81	0.0	0.0	3.41	0.0	8.53e-04	0.0	-0.15
		-0.15	0.0	0.0	0.0	13.6	0.0	3.81e-04	0.0	8.53e-04	0.0	0.08
						27.3	0.0	-3.40	0.0	8.53e-04	0.0	-0.15
70	2	0.05	0.0	0.0	-4.72	0.0	0.0	2.36	0.0	5.68e-04	0.0	-0.11
		-0.11	0.0	0.0	0.0	13.6	0.0	2.62e-04	0.0	5.68e-04	0.0	0.05
						27.3	0.0	-2.36	0.0	5.68e-04	0.0	-0.11
70	3	0.05	0.0	0.0	-3.98	0.0	0.0	1.99	0.0	4.41e-04	0.0	-0.09
		-0.09	0.0	0.0	0.0	13.6	0.0	2.14e-04	0.0	4.41e-04	0.0	0.05
						27.3	0.0	-1.99	0.0	4.41e-04	0.0	-0.09
70	4	0.04	0.0	0.0	-3.68	0.0	0.0	1.84	0.0	3.89e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	13.6	0.0	1.95e-04	0.0	3.89e-04	0.0	0.04
						27.3	0.0	-1.84	0.0	3.89e-04	0.0	-0.08
71	1	0.15	0.0	0.0	-6.60	0.0	0.0	3.92	0.0	8.53e-04	0.0	-0.15
		-0.15	0.0	0.0	0.0	13.6	0.0	0.52	0.0	8.53e-04	0.0	0.15
						27.3	0.0	-2.68	0.0	8.53e-04	0.0	-9.98e-03
71	2	0.11	0.0	0.0	-4.58	0.0	0.0	2.72	0.0	5.68e-04	0.0	-0.11
		-0.11	0.0	0.0	0.0	13.6	0.0	0.36	0.0	5.68e-04	0.0	0.10
						27.3	0.0	-1.86	0.0	5.68e-04	0.0	-6.87e-03
71	3	0.09	0.0	0.0	-3.86	0.0	0.0	2.29	0.0	4.41e-04	0.0	-0.09
		-0.09	0.0	0.0	0.0	13.6	0.0	0.30	0.0	4.41e-04	0.0	0.09
						27.3	0.0	-1.57	0.0	4.41e-04	0.0	-5.93e-03
71	4	0.08	0.0	0.0	-3.57	0.0	0.0	2.12	0.0	3.89e-04	0.0	-0.08
		-0.08	0.0	0.0	0.0	13.6	0.0	0.28	0.0	3.89e-04	0.0	0.08
						27.3	0.0	-1.45	0.0	3.89e-04	0.0	-5.55e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
72	1	0.07	0.0	0.0	-6.43	0.0	0.0	3.22	0.0	5.28e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	12.8	0.0	3.56e-03	0.0	5.28e-03	0.0	0.07
						25.5	0.0	-3.21	0.0	5.28e-03	0.0	-0.14
72	2	0.05	0.0	0.0	-4.46	0.0	0.0	2.23	0.0	3.67e-03	0.0	-0.10
		-0.10	0.0	0.0	0.0	12.8	0.0	2.47e-03	0.0	3.67e-03	0.0	0.05
						25.5	0.0	-2.23	0.0	3.67e-03	0.0	-0.09
72	3	0.04	0.0	0.0	-3.76	0.0	0.0	1.88	0.0	3.11e-03	0.0	-0.08
		-0.08	0.0	0.0	0.0	12.8	0.0	2.03e-03	0.0	3.11e-03	0.0	0.04
						25.5	0.0	-1.88	0.0	3.11e-03	0.0	-0.08
72	4	0.04	0.0	0.0	-3.48	0.0	0.0	1.74	0.0	2.89e-03	0.0	-0.07
		-0.07	0.0	0.0	0.0	12.8	0.0	1.86e-03	0.0	2.89e-03	0.0	0.04
						25.5	0.0	-1.74	0.0	2.89e-03	0.0	-0.07
73	1	0.05	0.0	0.0	-6.42	0.0	0.0	3.32	0.0	5.28e-03	0.0	-0.17
		-0.17	0.0	0.0	0.0	12.8	0.0	0.11	0.0	5.28e-03	0.0	0.05
						25.5	0.0	-3.10	0.0	5.28e-03	0.0	-0.14
73	2	0.04	0.0	0.0	-4.45	0.0	0.0	2.30	0.0	3.67e-03	0.0	-0.11
		-0.11	0.0	0.0	0.0	12.8	0.0	0.08	0.0	3.67e-03	0.0	0.04
						25.5	0.0	-2.15	0.0	3.67e-03	0.0	-0.10
73	3	0.03	0.0	0.0	-3.75	0.0	0.0	1.94	0.0	3.11e-03	0.0	-0.10
		-0.10	0.0	0.0	0.0	12.8	0.0	0.06	0.0	3.11e-03	0.0	0.03
						25.5	0.0	-1.81	0.0	3.11e-03	0.0	-0.08
73	4	0.03	0.0	0.0	-3.47	0.0	0.0	1.80	0.0	2.89e-03	0.0	-0.09
		-0.09	0.0	0.0	0.0	12.8	0.0	0.06	0.0	2.89e-03	0.0	0.03
						25.5	0.0	-1.68	0.0	2.89e-03	0.0	-0.07
74	1	0.07	0.0	0.0	-6.24	0.0	0.0	3.26	0.0	5.28e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	12.8	0.0	0.04	0.0	5.28e-03	0.0	0.07
						25.5	0.0	-2.98	0.0	5.28e-03	0.0	-0.13
74	2	0.05	0.0	0.0	-4.33	0.0	0.0	2.26	0.0	3.67e-03	0.0	-0.09

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.09	0.0	0.0	0.0	12.8	0.0	0.03	0.0	3.67e-03	0.0	0.05
						25.5	0.0	-2.07	0.0	3.67e-03	0.0	-0.09
74	3	0.04	0.0	0.0	-3.65	0.0	0.0	1.90	0.0	3.11e-03	0.0	-0.08
		-0.08	0.0	0.0	0.0	12.8	0.0	0.02	0.0	3.11e-03	0.0	0.04
						25.5	0.0	-1.75	0.0	3.11e-03	0.0	-0.07
74	4	0.04	0.0	0.0	-3.38	0.0	0.0	1.76	0.0	2.89e-03	0.0	-0.07
		-0.07	0.0	0.0	0.0	12.8	0.0	0.02	0.0	2.89e-03	0.0	0.04
						25.5	0.0	-1.62	0.0	2.89e-03	0.0	-0.07
75	1	0.13	0.0	-1.19e-06	-8.86	0.0	0.0	4.43	0.0	2.60e-03	0.0	-0.26
		-0.26	0.0	0.0	0.0	17.6	0.0	-8.84e-04	0.0	2.60e-03	0.0	0.13
						35.1	0.0	-4.44	0.0	2.60e-03	0.0	-0.26
75	2	0.09	0.0	0.0	-6.15	0.0	0.0	3.07	0.0	1.91e-03	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.6	0.0	-6.11e-04	0.0	1.91e-03	0.0	0.09
						35.1	0.0	-3.08	0.0	1.91e-03	0.0	-0.18
75	3	0.08	0.0	0.0	-5.18	0.0	0.0	2.59	0.0	1.77e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	17.6	0.0	-5.12e-04	0.0	1.77e-03	0.0	0.08
						35.1	0.0	-2.59	0.0	1.77e-03	0.0	-0.15
75	4	0.07	0.0	0.0	-4.79	0.0	0.0	2.40	0.0	1.71e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	17.6	0.0	-4.72e-04	0.0	1.71e-03	0.0	0.07
						35.1	0.0	-2.40	0.0	1.71e-03	0.0	-0.14
76	1	0.13	0.0	-1.19e-06	-8.84	0.0	0.0	4.41	0.0	2.60e-03	0.0	-0.26
		-0.26	0.0	0.0	0.0	17.6	0.0	-2.55e-03	0.0	2.60e-03	0.0	0.13
						35.1	0.0	-4.42	0.0	2.60e-03	0.0	-0.26
76	2	0.09	0.0	0.0	-6.13	0.0	0.0	3.06	0.0	1.91e-03	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.6	0.0	-1.77e-03	0.0	1.91e-03	0.0	0.09
						35.1	0.0	-3.07	0.0	1.91e-03	0.0	-0.18
76	3	0.08	0.0	0.0	-5.16	0.0	0.0	2.58	0.0	1.77e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	17.6	0.0	-1.49e-03	0.0	1.77e-03	0.0	0.08

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						35.1	0.0	-2.58	0.0	1.77e-03	0.0	-0.15
76	4	0.07	0.0	0.0	-4.78	0.0	0.0	2.39	0.0	1.71e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	17.6	0.0	-1.38e-03	0.0	1.71e-03	0.0	0.07
						35.1	0.0	-2.39	0.0	1.71e-03	0.0	-0.14
77	1	0.12	0.0	-1.17e-06	-8.89	0.0	0.0	4.40	0.0	2.60e-03	0.0	-0.26
		-0.28	0.0	0.0	0.0	17.6	0.0	-0.05	0.0	2.60e-03	0.0	0.12
						35.1	0.0	-4.50	0.0	2.60e-03	0.0	-0.28
77	2	0.09	0.0	0.0	-6.17	0.0	0.0	3.05	0.0	1.91e-03	0.0	-0.18
		-0.19	0.0	0.0	0.0	17.6	0.0	-0.03	0.0	1.91e-03	0.0	0.09
						35.1	0.0	-3.12	0.0	1.91e-03	0.0	-0.19
77	3	0.07	0.0	0.0	-5.20	0.0	0.0	2.57	0.0	1.77e-03	0.0	-0.15
		-0.16	0.0	0.0	0.0	17.6	0.0	-0.03	0.0	1.77e-03	0.0	0.07
						35.1	0.0	-2.63	0.0	1.77e-03	0.0	-0.16
77	4	0.07	0.0	0.0	-4.81	0.0	0.0	2.38	0.0	1.71e-03	0.0	-0.14
		-0.15	0.0	0.0	0.0	17.6	0.0	-0.03	0.0	1.71e-03	0.0	0.07
						35.1	0.0	-2.43	0.0	1.71e-03	0.0	-0.15
78	1	0.13	0.0	-1.17e-06	-8.82	0.0	0.0	4.44	0.0	2.60e-03	0.0	-0.27
		-0.27	0.0	0.0	0.0	17.6	0.0	0.03	0.0	2.60e-03	0.0	0.13
						35.1	0.0	-4.39	0.0	2.60e-03	0.0	-0.26
78	2	0.09	0.0	0.0	-6.12	0.0	0.0	3.08	0.0	1.91e-03	0.0	-0.19
		-0.19	0.0	0.0	0.0	17.6	0.0	0.02	0.0	1.91e-03	0.0	0.09
						35.1	0.0	-3.04	0.0	1.91e-03	0.0	-0.18
78	3	0.07	0.0	0.0	-5.16	0.0	0.0	2.59	0.0	1.77e-03	0.0	-0.16
		-0.16	0.0	0.0	0.0	17.6	0.0	0.02	0.0	1.77e-03	0.0	0.07
						35.1	0.0	-2.56	0.0	1.77e-03	0.0	-0.15
78	4	0.07	0.0	0.0	-4.77	0.0	0.0	2.40	0.0	1.71e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	17.6	0.0	0.01	0.0	1.71e-03	0.0	0.07
						35.1	0.0	-2.37	0.0	1.71e-03	0.0	-0.14

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
79	1	0.13	0.0	-1.20e-06	-8.88	0.0	0.0	4.44	0.0	2.60e-03	0.0	-0.26
		-0.26	0.0	0.0	0.0	17.6	0.0	3.01e-03	0.0	2.60e-03	0.0	0.13
						35.1	0.0	-4.44	0.0	2.60e-03	0.0	-0.26
79	2	0.09	0.0	0.0	-6.16	0.0	0.0	3.08	0.0	1.91e-03	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.6	0.0	2.09e-03	0.0	1.91e-03	0.0	0.09
						35.1	0.0	-3.08	0.0	1.91e-03	0.0	-0.18
79	3	0.08	0.0	0.0	-5.19	0.0	0.0	2.60	0.0	1.77e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	17.6	0.0	1.77e-03	0.0	1.77e-03	0.0	0.08
						35.1	0.0	-2.59	0.0	1.77e-03	0.0	-0.15
79	4	0.07	0.0	0.0	-4.80	0.0	0.0	2.40	0.0	1.71e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	17.6	0.0	1.64e-03	0.0	1.71e-03	0.0	0.07
						35.1	0.0	-2.40	0.0	1.71e-03	0.0	-0.14
80	1	0.13	0.0	-1.19e-06	-8.85	0.0	0.0	4.42	0.0	2.60e-03	0.0	-0.26
		-0.26	0.0	0.0	0.0	17.6	0.0	-4.25e-04	0.0	2.60e-03	0.0	0.13
						35.1	0.0	-4.43	0.0	2.60e-03	0.0	-0.26
80	2	0.09	0.0	0.0	-6.14	0.0	0.0	3.07	0.0	1.91e-03	0.0	-0.18
		-0.18	0.0	0.0	0.0	17.6	0.0	-2.93e-04	0.0	1.91e-03	0.0	0.09
						35.1	0.0	-3.07	0.0	1.91e-03	0.0	-0.18
80	3	0.08	0.0	0.0	-5.17	0.0	0.0	2.59	0.0	1.77e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	17.6	0.0	-2.43e-04	0.0	1.77e-03	0.0	0.08
						35.1	0.0	-2.59	0.0	1.77e-03	0.0	-0.15
80	4	0.07	0.0	0.0	-4.79	0.0	0.0	2.39	0.0	1.71e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	17.6	0.0	-2.23e-04	0.0	1.71e-03	0.0	0.07
						35.1	0.0	-2.39	0.0	1.71e-03	0.0	-0.14
81	1	0.24	0.0	-1.52e-06	-8.91	0.0	0.0	5.13	0.0	2.60e-03	0.0	-0.28
		-0.28	0.0	0.0	0.0	17.6	0.0	0.67	0.0	2.60e-03	0.0	0.23
						35.1	0.0	-3.78	0.0	2.60e-03	0.0	-0.04
81	2	0.17	0.0	-1.05e-06	-6.18	0.0	0.0	3.56	0.0	1.91e-03	0.0	-0.19

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.19	0.0	0.0	0.0	17.6	0.0	0.47	0.0	1.91e-03	0.0	0.16
						35.1	0.0	-2.63	0.0	1.91e-03	0.0	-0.03
81	3	0.14	0.0	0.0	-5.21	0.0	0.0	3.00	0.0	1.77e-03	0.0	-0.16
		-0.16	0.0	0.0	0.0	17.6	0.0	0.39	0.0	1.77e-03	0.0	0.14
						35.1	0.0	-2.21	0.0	1.77e-03	0.0	-0.02
81	4	0.13	0.0	0.0	-4.82	0.0	0.0	2.77	0.0	1.71e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	17.6	0.0	0.36	0.0	1.71e-03	0.0	0.13
						35.1	0.0	-2.05	0.0	1.71e-03	0.0	-0.02
82	1	0.02	0.0	0.0	-0.29	0.0	0.0	-0.40	0.0	0.04	0.0	0.02
		-0.11	0.0	0.0	0.0	12.3	0.0	-0.55	0.0	0.04	0.0	-0.03
						24.5	0.0	-0.69	0.0	0.04	0.0	-0.11
82	2	0.02	0.0	0.0	-0.22	0.0	0.0	-0.29	0.0	0.03	0.0	0.02
		-0.08	0.0	0.0	0.0	12.3	0.0	-0.40	0.0	0.03	0.0	-0.03
						24.5	0.0	-0.51	0.0	0.03	0.0	-0.08
82	3	0.02	0.0	0.0	-0.22	0.0	0.0	-0.26	0.0	0.02	0.0	0.02
		-0.08	0.0	0.0	0.0	12.3	0.0	-0.37	0.0	0.02	0.0	-0.02
						24.5	0.0	-0.48	0.0	0.02	0.0	-0.08
82	4	0.02	0.0	0.0	-0.22	0.0	0.0	-0.25	0.0	0.02	0.0	0.02
		-0.07	0.0	0.0	0.0	12.3	0.0	-0.36	0.0	0.02	0.0	-0.02
						24.5	0.0	-0.47	0.0	0.02	0.0	-0.07
83	1	4.20e-03	0.0	0.0	-0.25	0.0	0.0	0.14	0.0	-9.62e-03	0.0	-4.58e-03
		-4.58e-03	0.0	0.0	0.0	10.8	0.0	0.02	0.0	-9.62e-03	0.0	4.07e-03
						21.5	0.0	-0.11	0.0	-9.62e-03	0.0	-8.02e-04
83	2	3.16e-03	0.0	0.0	-0.19	0.0	0.0	0.11	0.0	-6.66e-03	0.0	-3.52e-03
		-3.52e-03	0.0	0.0	0.0	10.8	0.0	0.01	0.0	-6.66e-03	0.0	3.07e-03
						21.5	0.0	-0.08	0.0	-6.66e-03	0.0	-7.46e-04
83	3	3.05e-03	0.0	0.0	-0.19	0.0	0.0	0.11	0.0	-4.89e-03	0.0	-3.51e-03
		-3.51e-03	0.0	0.0	0.0	10.8	0.0	0.01	0.0	-4.89e-03	0.0	2.97e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						21.5	0.0	-0.08	0.0	-4.89e-03	0.0	-9.58e-04
83	4	3.00e-03	0.0	0.0	-0.19	0.0	0.0	0.11	0.0	-4.19e-03	0.0	-3.50e-03
		-3.50e-03	0.0	0.0	0.0	10.8	0.0	0.01	0.0	-4.19e-03	0.0	2.93e-03
						21.5	0.0	-0.09	0.0	-4.19e-03	0.0	-1.04e-03
84	1	-2.51e-03	0.0	0.0	-0.30	0.0	0.0	0.09	0.0	8.68e-03	0.0	-5.72e-03
		-0.02	0.0	0.0	0.0	12.8	0.0	-0.06	0.0	8.68e-03	0.0	-4.15e-03
						25.5	0.0	-0.21	0.0	8.68e-03	0.0	-0.02
84	2	-1.49e-03	0.0	0.0	-0.23	0.0	0.0	0.07	0.0	6.02e-03	0.0	-4.49e-03
		-0.02	0.0	0.0	0.0	12.8	0.0	-0.04	0.0	6.02e-03	0.0	-2.44e-03
						25.5	0.0	-0.16	0.0	6.02e-03	0.0	-0.02
84	3	-8.00e-04	0.0	0.0	-0.23	0.0	0.0	0.08	0.0	4.56e-03	0.0	-4.63e-03
		-0.01	0.0	0.0	0.0	12.8	0.0	-0.03	0.0	4.56e-03	0.0	-1.35e-03
						25.5	0.0	-0.15	0.0	4.56e-03	0.0	-0.01
84	4	-4.88e-04	0.0	0.0	-0.23	0.0	0.0	0.09	0.0	3.98e-03	0.0	-4.69e-03
		-0.01	0.0	0.0	0.0	12.8	0.0	-0.03	0.0	3.98e-03	0.0	-9.16e-04
						25.5	0.0	-0.14	0.0	3.98e-03	0.0	-0.01
85	1	0.05	0.0	0.0	-0.24	0.0	0.0	0.24	0.0	-5.45e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.7	0.0	0.12	0.0	-5.45e-04	0.0	0.04
						21.4	0.0	1.79e-03	0.0	-5.45e-04	0.0	0.05
85	2	0.04	0.0	0.0	-0.18	0.0	0.0	0.19	0.0	-3.49e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.7	0.0	0.09	0.0	-3.49e-04	0.0	0.03
						21.4	0.0	1.22e-03	0.0	-3.49e-04	0.0	0.04
85	3	0.04	0.0	0.0	-0.18	0.0	0.0	0.19	0.0	-2.50e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.7	0.0	0.09	0.0	-2.50e-04	0.0	0.03
						21.4	0.0	1.00e-03	0.0	-2.50e-04	0.0	0.04
85	4	0.04	0.0	0.0	-0.18	0.0	0.0	0.19	0.0	-2.10e-04	0.0	0.02
		0.02	0.0	0.0	0.0	10.7	0.0	0.09	0.0	-2.10e-04	0.0	0.03
						21.4	0.0	9.13e-04	0.0	-2.10e-04	0.0	0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
86	1	6.55e-03	0.0	0.0	-0.42	0.0	0.0	0.21	0.0	-1.10e-03	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	9.91e-05	0.0	-1.10e-03	0.0	6.55e-03
						37.4	0.0	-0.21	0.0	-1.10e-03	0.0	-0.01
86	2	5.04e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-7.74e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	7.51e-05	0.0	-7.74e-04	0.0	5.04e-03
						37.4	0.0	-0.16	0.0	-7.74e-04	0.0	-0.01
86	3	5.04e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.73e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	7.34e-05	0.0	-6.73e-04	0.0	5.04e-03
						37.4	0.0	-0.16	0.0	-6.73e-04	0.0	-0.01
86	4	5.04e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.32e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	7.27e-05	0.0	-6.32e-04	0.0	5.04e-03
						37.4	0.0	-0.16	0.0	-6.32e-04	0.0	-0.01
87	1	6.38e-03	0.0	0.0	-0.42	0.0	0.0	0.21	0.0	-1.10e-03	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	1.02e-03	0.0	-1.10e-03	0.0	6.38e-03
						37.4	0.0	-0.21	0.0	-1.10e-03	0.0	-0.01
87	2	4.91e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-7.74e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	7.78e-04	0.0	-7.74e-04	0.0	4.91e-03
						37.4	0.0	-0.16	0.0	-7.74e-04	0.0	-0.01
87	3	4.91e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.73e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	7.73e-04	0.0	-6.73e-04	0.0	4.91e-03
						37.4	0.0	-0.16	0.0	-6.73e-04	0.0	-0.01
87	4	4.91e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.32e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	7.71e-04	0.0	-6.32e-04	0.0	4.91e-03
						37.4	0.0	-0.16	0.0	-6.32e-04	0.0	-0.01
88	1	8.29e-03	0.0	0.0	-0.42	0.0	0.0	0.22	0.0	-1.10e-03	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	0.01	0.0	-1.10e-03	0.0	8.29e-03
						37.4	0.0	-0.20	0.0	-1.10e-03	0.0	-9.23e-03
88	2	6.36e-03	0.0	0.0	-0.32	0.0	0.0	0.17	0.0	-7.74e-04	0.0	-0.01

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.01	0.0	0.0	0.0	18.7	0.0	8.49e-03	0.0	-7.74e-04	0.0	6.36e-03
						37.4	0.0	-0.15	0.0	-7.74e-04	0.0	-7.15e-03
88	3	6.32e-03	0.0	0.0	-0.32	0.0	0.0	0.17	0.0	-6.73e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	8.27e-03	0.0	-6.73e-04	0.0	6.32e-03
						37.4	0.0	-0.15	0.0	-6.73e-04	0.0	-7.22e-03
88	4	6.31e-03	0.0	0.0	-0.32	0.0	0.0	0.17	0.0	-6.32e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	8.19e-03	0.0	-6.32e-04	0.0	6.31e-03
						37.4	0.0	-0.15	0.0	-6.32e-04	0.0	-7.25e-03
89	1	8.29e-03	0.0	0.0	-0.42	0.0	0.0	0.20	0.0	-1.10e-03	0.0	-9.22e-03
		-0.01	0.0	0.0	0.0	18.7	0.0	-0.01	0.0	-1.10e-03	0.0	8.29e-03
						37.4	0.0	-0.22	0.0	-1.10e-03	0.0	-0.01
89	2	6.37e-03	0.0	0.0	-0.32	0.0	0.0	0.15	0.0	-7.74e-04	0.0	-7.11e-03
		-0.01	0.0	0.0	0.0	18.7	0.0	-8.61e-03	0.0	-7.74e-04	0.0	6.37e-03
						37.4	0.0	-0.17	0.0	-7.74e-04	0.0	-0.01
89	3	6.36e-03	0.0	0.0	-0.32	0.0	0.0	0.15	0.0	-6.73e-04	0.0	-7.13e-03
		-0.01	0.0	0.0	0.0	18.7	0.0	-8.55e-03	0.0	-6.73e-04	0.0	6.36e-03
						37.4	0.0	-0.17	0.0	-6.73e-04	0.0	-0.01
89	4	6.36e-03	0.0	0.0	-0.32	0.0	0.0	0.15	0.0	-6.32e-04	0.0	-7.13e-03
		-0.01	0.0	0.0	0.0	18.7	0.0	-8.53e-03	0.0	-6.32e-04	0.0	6.36e-03
						37.4	0.0	-0.17	0.0	-6.32e-04	0.0	-0.01
90	1	6.38e-03	0.0	0.0	-0.42	0.0	0.0	0.21	0.0	-1.10e-03	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-1.01e-03	0.0	-1.10e-03	0.0	6.38e-03
						37.4	0.0	-0.21	0.0	-1.10e-03	0.0	-0.01
90	2	4.91e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-7.74e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-7.66e-04	0.0	-7.74e-04	0.0	4.91e-03
						37.4	0.0	-0.16	0.0	-7.74e-04	0.0	-0.01
90	3	4.91e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.73e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-7.47e-04	0.0	-6.73e-04	0.0	4.91e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						37.4	0.0	-0.16	0.0	-6.73e-04	0.0	-0.01
90	4	4.91e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.32e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-7.39e-04	0.0	-6.32e-04	0.0	4.91e-03
						37.4	0.0	-0.16	0.0	-6.32e-04	0.0	-0.01
91	1	6.55e-03	0.0	0.0	-0.42	0.0	0.0	0.21	0.0	-1.10e-03	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-1.00e-04	0.0	-1.10e-03	0.0	6.55e-03
						37.4	0.0	-0.21	0.0	-1.10e-03	0.0	-0.01
91	2	5.04e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-7.74e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-7.68e-05	0.0	-7.74e-04	0.0	5.04e-03
						37.4	0.0	-0.16	0.0	-7.74e-04	0.0	-0.01
91	3	5.04e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.73e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-7.61e-05	0.0	-6.73e-04	0.0	5.04e-03
						37.4	0.0	-0.16	0.0	-6.73e-04	0.0	-0.01
91	4	5.04e-03	0.0	0.0	-0.32	0.0	0.0	0.16	0.0	-6.32e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	18.7	0.0	-7.59e-05	0.0	-6.32e-04	0.0	5.04e-03
						37.4	0.0	-0.16	0.0	-6.32e-04	0.0	-0.01
92	1	-5.99e-03	0.0	0.0	-0.42	0.0	0.0	0.09	0.0	-1.10e-03	0.0	-9.23e-03
		-0.06	0.0	0.0	0.0	18.7	0.0	-0.12	0.0	-1.10e-03	0.0	-0.01
						37.4	0.0	-0.33	0.0	-1.10e-03	0.0	-0.06
92	2	-4.55e-03	0.0	0.0	-0.32	0.0	0.0	0.07	0.0	-7.74e-04	0.0	-7.15e-03
		-0.04	0.0	0.0	0.0	18.7	0.0	-0.09	0.0	-7.74e-04	0.0	-9.64e-03
						37.4	0.0	-0.26	0.0	-7.74e-04	0.0	-0.04
92	3	-4.45e-03	0.0	0.0	-0.32	0.0	0.0	0.07	0.0	-6.73e-04	0.0	-7.22e-03
		-0.04	0.0	0.0	0.0	18.7	0.0	-0.09	0.0	-6.73e-04	0.0	-9.27e-03
						37.4	0.0	-0.25	0.0	-6.73e-04	0.0	-0.04
92	4	-4.42e-03	0.0	0.0	-0.32	0.0	0.0	0.07	0.0	-6.32e-04	0.0	-7.25e-03
		-0.04	0.0	0.0	0.0	18.7	0.0	-0.09	0.0	-6.32e-04	0.0	-9.13e-03
						37.4	0.0	-0.25	0.0	-6.32e-04	0.0	-0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
93	1	0.11	0.0	0.0	-8.13	0.0	0.0	4.06	0.0	5.14e-03	0.0	-0.22
		-0.22	0.0	0.0	0.0	16.2	0.0	8.05e-05	0.0	5.14e-03	0.0	0.11
						32.4	0.0	-4.07	0.0	5.14e-03	0.0	-0.22
93	2	0.08	0.0	0.0	-5.64	0.0	0.0	2.82	0.0	3.54e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	16.2	0.0	5.74e-05	0.0	3.54e-03	0.0	0.08
						32.4	0.0	-2.82	0.0	3.54e-03	0.0	-0.15
93	3	0.06	0.0	0.0	-4.75	0.0	0.0	2.38	0.0	2.95e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	16.2	0.0	5.09e-05	0.0	2.95e-03	0.0	0.06
						32.4	0.0	-2.38	0.0	2.95e-03	0.0	-0.13
93	4	0.06	0.0	0.0	-4.40	0.0	0.0	2.20	0.0	2.71e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	16.2	0.0	4.82e-05	0.0	2.71e-03	0.0	0.06
						32.4	0.0	-2.20	0.0	2.71e-03	0.0	-0.12
94	1	0.11	0.0	0.0	-8.10	0.0	0.0	4.05	0.0	5.14e-03	0.0	-0.22
		-0.22	0.0	0.0	0.0	16.2	0.0	-2.49e-03	0.0	5.14e-03	0.0	0.11
						32.4	0.0	-4.05	0.0	5.14e-03	0.0	-0.22
94	2	0.08	0.0	0.0	-5.62	0.0	0.0	2.81	0.0	3.54e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	16.2	0.0	-1.72e-03	0.0	3.54e-03	0.0	0.08
						32.4	0.0	-2.81	0.0	3.54e-03	0.0	-0.15
94	3	0.06	0.0	0.0	-4.73	0.0	0.0	2.36	0.0	2.95e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	16.2	0.0	-1.45e-03	0.0	2.95e-03	0.0	0.06
						32.4	0.0	-2.37	0.0	2.95e-03	0.0	-0.13
94	4	0.06	0.0	0.0	-4.38	0.0	0.0	2.19	0.0	2.71e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	16.2	0.0	-1.34e-03	0.0	2.71e-03	0.0	0.06
						32.4	0.0	-2.19	0.0	2.71e-03	0.0	-0.12
95	1	0.14	0.0	-1.07e-06	-8.14	0.0	0.0	4.28	0.0	5.14e-03	0.0	-0.22
		-0.22	0.0	0.0	0.0	16.2	0.0	0.21	0.0	5.14e-03	0.0	0.14
						32.4	0.0	-3.86	0.0	5.14e-03	0.0	-0.15
95	2	0.10	0.0	0.0	-5.64	0.0	0.0	2.97	0.0	3.54e-03	0.0	-0.15

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.15	0.0	0.0	0.0	16.2	0.0	0.15	0.0	3.54e-03	0.0	0.10
						32.4	0.0	-2.68	0.0	3.54e-03	0.0	-0.11
95	3	0.08	0.0	0.0	-4.76	0.0	0.0	2.50	0.0	2.95e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	16.2	0.0	0.12	0.0	2.95e-03	0.0	0.08
						32.4	0.0	-2.25	0.0	2.95e-03	0.0	-0.09
95	4	0.08	0.0	0.0	-4.40	0.0	0.0	2.31	0.0	2.71e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	16.2	0.0	0.11	0.0	2.71e-03	0.0	0.08
						32.4	0.0	-2.09	0.0	2.71e-03	0.0	-0.08
96	1	0.10	0.0	0.0	-8.08	0.0	0.0	4.07	0.0	5.14e-03	0.0	-0.23
		-0.23	0.0	0.0	0.0	16.2	0.0	0.03	0.0	5.14e-03	0.0	0.10
						32.4	0.0	-4.01	0.0	5.14e-03	0.0	-0.22
96	2	0.07	0.0	0.0	-5.61	0.0	0.0	2.83	0.0	3.54e-03	0.0	-0.16
		-0.16	0.0	0.0	0.0	16.2	0.0	0.02	0.0	3.54e-03	0.0	0.07
						32.4	0.0	-2.78	0.0	3.54e-03	0.0	-0.15
96	3	0.06	0.0	0.0	-4.72	0.0	0.0	2.38	0.0	2.95e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	16.2	0.0	0.02	0.0	2.95e-03	0.0	0.06
						32.4	0.0	-2.34	0.0	2.95e-03	0.0	-0.13
96	4	0.06	0.0	0.0	-4.37	0.0	0.0	2.20	0.0	2.71e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	16.2	0.0	0.02	0.0	2.71e-03	0.0	0.06
						32.4	0.0	-2.17	0.0	2.71e-03	0.0	-0.12
97	1	0.11	0.0	0.0	-8.13	0.0	0.0	4.06	0.0	5.14e-03	0.0	-0.22
		-0.22	0.0	0.0	0.0	16.2	0.0	-0.01	0.0	5.14e-03	0.0	0.11
						32.4	0.0	-4.08	0.0	5.14e-03	0.0	-0.22
97	2	0.07	0.0	0.0	-5.64	0.0	0.0	2.81	0.0	3.54e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	16.2	0.0	-7.43e-03	0.0	3.54e-03	0.0	0.07
						32.4	0.0	-2.83	0.0	3.54e-03	0.0	-0.15
97	3	0.06	0.0	0.0	-4.75	0.0	0.0	2.37	0.0	2.95e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	16.2	0.0	-6.26e-03	0.0	2.95e-03	0.0	0.06

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						32.4	0.0	-2.38	0.0	2.95e-03	0.0	-0.13
97	4	0.06	0.0	0.0	-4.40	0.0	0.0	2.19	0.0	2.71e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	16.2	0.0	-5.79e-03	0.0	2.71e-03	0.0	0.06
						32.4	0.0	-2.21	0.0	2.71e-03	0.0	-0.12
98	1	0.11	0.0	0.0	-8.12	0.0	0.0	4.06	0.0	5.14e-03	0.0	-0.22
		-0.22	0.0	0.0	0.0	16.2	0.0	-5.66e-04	0.0	5.14e-03	0.0	0.11
						32.4	0.0	-4.06	0.0	5.14e-03	0.0	-0.22
98	2	0.08	0.0	0.0	-5.63	0.0	0.0	2.81	0.0	3.54e-03	0.0	-0.15
		-0.15	0.0	0.0	0.0	16.2	0.0	-3.90e-04	0.0	3.54e-03	0.0	0.08
						32.4	0.0	-2.82	0.0	3.54e-03	0.0	-0.15
98	3	0.06	0.0	0.0	-4.74	0.0	0.0	2.37	0.0	2.95e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	16.2	0.0	-3.25e-04	0.0	2.95e-03	0.0	0.06
						32.4	0.0	-2.37	0.0	2.95e-03	0.0	-0.13
98	4	0.06	0.0	0.0	-4.39	0.0	0.0	2.19	0.0	2.71e-03	0.0	-0.12
		-0.12	0.0	0.0	0.0	16.2	0.0	-2.99e-04	0.0	2.71e-03	0.0	0.06
						32.4	0.0	-2.20	0.0	2.71e-03	0.0	-0.12
99	1	0.23	0.0	-1.47e-06	-6.69	0.0	0.0	4.12	0.0	-0.64	0.0	-0.24
		-0.24	0.0	0.0	0.0	19.5	0.0	0.64	0.0	-0.64	0.0	0.22
						39.0	0.0	-2.57	0.0	-0.64	0.0	0.03
99	2	0.16	0.0	-1.02e-06	-4.63	0.0	0.0	2.86	0.0	-0.48	0.0	-0.17
		-0.17	0.0	0.0	0.0	19.5	0.0	0.45	0.0	-0.48	0.0	0.15
						39.0	0.0	-1.77	0.0	-0.48	0.0	0.03
99	3	0.12	0.0	0.0	-3.41	0.0	0.0	2.12	0.0	-0.46	0.0	-0.12
		-0.12	0.0	0.0	0.0	19.5	0.0	0.35	0.0	-0.46	0.0	0.12
						39.0	0.0	-1.29	0.0	-0.46	0.0	0.03
99	4	0.11	0.0	0.0	-2.92	0.0	0.0	1.83	0.0	-0.45	0.0	-0.11
		-0.11	0.0	0.0	0.0	19.5	0.0	0.31	0.0	-0.45	0.0	0.10
						39.0	0.0	-1.09	0.0	-0.45	0.0	0.03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
100	1	-0.05	0.0	0.0	-0.28	0.0	0.0	1.65	0.0	2.22e-04	0.0	-0.33
		-0.33	0.0	0.0	0.0	9.4	0.0	1.51	0.0	2.22e-04	0.0	-0.19
						18.8	0.0	1.37	0.0	2.22e-04	0.0	-0.05
100	2	-0.04	0.0	0.0	-0.21	0.0	0.0	1.21	0.0	1.31e-04	0.0	-0.25
		-0.25	0.0	0.0	0.0	9.4	0.0	1.10	0.0	1.31e-04	0.0	-0.14
						18.8	0.0	1.00	0.0	1.31e-04	0.0	-0.04
100	3	-0.03	0.0	0.0	-0.20	0.0	0.0	1.08	0.0	1.32e-05	0.0	-0.22
		-0.22	0.0	0.0	0.0	9.4	0.0	0.98	0.0	1.32e-05	0.0	-0.12
						18.8	0.0	0.88	0.0	1.32e-05	0.0	-0.03
100	4	-0.03	0.0	0.0	-0.19	0.0	0.0	1.03	0.0	-3.38e-05	0.0	-0.21
		-0.21	0.0	0.0	0.0	9.4	0.0	0.93	0.0	-3.38e-05	0.0	-0.11
						18.8	0.0	0.83	0.0	-3.38e-05	0.0	-0.03
101	1	0.12	0.0	-7.21e-06	-0.91	0.0	0.0	1.69	0.0	2.22e-04	0.0	-0.35
		-0.35	0.0	0.0	0.0	19.3	0.0	1.22	0.0	2.22e-04	0.0	-0.07
						38.5	0.0	0.78	0.0	2.22e-04	0.0	0.12
101	2	0.09	0.0	-5.27e-06	-0.66	0.0	0.0	1.23	0.0	1.31e-04	0.0	-0.26
		-0.26	0.0	0.0	0.0	19.3	0.0	0.89	0.0	1.31e-04	0.0	-0.05
						38.5	0.0	0.57	0.0	1.31e-04	0.0	0.09
101	3	0.07	0.0	-4.63e-06	-0.57	0.0	0.0	1.07	0.0	1.34e-05	0.0	-0.23
		-0.23	0.0	0.0	0.0	19.3	0.0	0.78	0.0	1.34e-05	0.0	-0.05
						38.5	0.0	0.50	0.0	1.34e-05	0.0	0.07
101	4	0.07	0.0	-4.37e-06	-0.53	0.0	0.0	1.00	0.0	-3.36e-05	0.0	-0.21
		-0.21	0.0	0.0	0.0	19.3	0.0	0.73	0.0	-3.36e-05	0.0	-0.05
						38.5	0.0	0.47	0.0	-3.36e-05	0.0	0.07
102	1	-1.75e-03	0.0	0.0	-0.51	0.0	0.0	-1.34	0.0	6.71e-03	0.0	-1.75e-03
		-0.35	0.0	0.0	0.0	11.0	0.0	-1.60	0.0	6.71e-03	0.0	-0.16
						22.0	0.0	-1.85	0.0	6.71e-03	0.0	-0.35
102	2	-1.27e-03	0.0	0.0	-0.37	0.0	0.0	-0.98	0.0	4.87e-03	0.0	-1.27e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.26	0.0	0.0	0.0	11.0	0.0	-1.17	0.0	4.87e-03	0.0	-0.12
						22.0	0.0	-1.35	0.0	4.87e-03	0.0	-0.26
102	3	-1.09e-03	0.0	0.0	-0.32	0.0	0.0	-0.86	0.0	4.17e-03	0.0	-1.09e-03
		-0.23	0.0	0.0	0.0	11.0	0.0	-1.02	0.0	4.17e-03	0.0	-0.10
						22.0	0.0	-1.18	0.0	4.17e-03	0.0	-0.23
102	4	-1.01e-03	0.0	0.0	-0.30	0.0	0.0	-0.82	0.0	3.89e-03	0.0	-1.01e-03
		-0.21	0.0	0.0	0.0	11.0	0.0	-0.97	0.0	3.89e-03	0.0	-0.10
						22.0	0.0	-1.11	0.0	3.89e-03	0.0	-0.21
103	1	-0.10	0.0	0.0	-3.83	0.0	0.0	5.07	0.0	-1.15e-03	0.0	-0.92
		-0.92	0.0	0.0	0.0	13.0	0.0	3.15	0.0	-1.15e-03	0.0	-0.38
						25.9	0.0	1.24	0.0	-1.15e-03	0.0	-0.10
103	2	-0.07	0.0	0.0	-2.65	0.0	0.0	3.51	0.0	-8.32e-04	0.0	-0.64
		-0.64	0.0	0.0	0.0	13.0	0.0	2.18	0.0	-8.32e-04	0.0	-0.27
						25.9	0.0	0.86	0.0	-8.32e-04	0.0	-0.07
103	3	-0.05	0.0	0.0	-1.96	0.0	0.0	2.60	0.0	-7.12e-04	0.0	-0.47
		-0.47	0.0	0.0	0.0	13.0	0.0	1.61	0.0	-7.12e-04	0.0	-0.20
						25.9	0.0	0.63	0.0	-7.12e-04	0.0	-0.05
103	4	-0.04	0.0	0.0	-1.69	0.0	0.0	2.23	0.0	-6.64e-04	0.0	-0.40
		-0.40	0.0	0.0	0.0	13.0	0.0	1.38	0.0	-6.64e-04	0.0	-0.17
						25.9	0.0	0.54	0.0	-6.64e-04	0.0	-0.04
104	1	0.39	0.0	-1.12e-05	-3.71	0.0	0.0	7.34	0.0	-1.15e-03	0.0	-0.92
		-0.92	0.0	0.0	0.0	11.9	0.0	5.48	0.0	-1.15e-03	0.0	-0.16
						23.8	0.0	3.63	0.0	-1.15e-03	0.0	0.39
104	2	0.27	0.0	-7.75e-06	-2.57	0.0	0.0	5.09	0.0	-8.31e-04	0.0	-0.64
		-0.64	0.0	0.0	0.0	11.9	0.0	3.80	0.0	-8.31e-04	0.0	-0.11
						23.8	0.0	2.52	0.0	-8.31e-04	0.0	0.27
104	3	0.20	0.0	-5.73e-06	-1.90	0.0	0.0	3.76	0.0	-7.12e-04	0.0	-0.47
		-0.47	0.0	0.0	0.0	11.9	0.0	2.81	0.0	-7.12e-04	0.0	-0.08

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						23.8	0.0	1.86	0.0	-7.12e-04	0.0	0.20
104	4	0.17	0.0	-4.92e-06	-1.63	0.0	0.0	3.23	0.0	-6.64e-04	0.0	-0.40
		-0.40	0.0	0.0	0.0	11.9	0.0	2.41	0.0	-6.64e-04	0.0	-0.07
						23.8	0.0	1.60	0.0	-6.64e-04	0.0	0.17
105	1	0.14	0.0	0.0	-5.10	0.0	0.0	2.11	0.0	-1.15e-03	0.0	-2.62e-05
		-0.14	0.0	0.0	0.0	15.7	0.0	-0.44	0.0	-1.15e-03	0.0	0.13
						31.3	0.0	-2.99	0.0	-1.15e-03	0.0	-0.14
105	2	0.09	0.0	0.0	-3.53	0.0	0.0	1.46	0.0	-8.31e-04	0.0	-1.98e-05
		-0.10	0.0	0.0	0.0	15.7	0.0	-0.31	0.0	-8.31e-04	0.0	0.09
						31.3	0.0	-2.07	0.0	-8.31e-04	0.0	-0.10
105	3	0.07	0.0	0.0	-2.61	0.0	0.0	1.08	0.0	-7.12e-04	0.0	-1.88e-05
		-0.07	0.0	0.0	0.0	15.7	0.0	-0.23	0.0	-7.12e-04	0.0	0.07
						31.3	0.0	-1.53	0.0	-7.12e-04	0.0	-0.07
105	4	0.06	0.0	0.0	-2.23	0.0	0.0	0.93	0.0	-6.64e-04	0.0	-1.85e-05
		-0.06	0.0	0.0	0.0	15.7	0.0	-0.19	0.0	-6.64e-04	0.0	0.06
						31.3	0.0	-1.31	0.0	-6.64e-04	0.0	-0.06
106	1	0.58	0.0	-1.00e-04	-1.19	0.0	0.0	2.69	0.0	2.84e-03	0.0	-1.02
		-1.02	0.0	0.0	0.0	37.7	0.0	2.12	0.0	2.84e-03	0.0	-0.11
						75.4	0.0	1.51	0.0	2.84e-03	0.0	0.58
106	2	0.44	0.0	-7.58e-05	-0.91	0.0	0.0	2.05	0.0	1.66e-03	0.0	-0.77
		-0.77	0.0	0.0	0.0	37.7	0.0	1.60	0.0	1.66e-03	0.0	-0.08
						75.4	0.0	1.14	0.0	1.66e-03	0.0	0.44
106	3	0.42	0.0	-7.26e-05	-0.90	0.0	0.0	1.98	0.0	4.05e-04	0.0	-0.74
		-0.74	0.0	0.0	0.0	37.7	0.0	1.54	0.0	4.05e-04	0.0	-0.07
						75.4	0.0	1.09	0.0	4.05e-04	0.0	0.42
106	4	0.42	0.0	-7.13e-05	-0.89	0.0	0.0	1.96	0.0	-9.70e-05	0.0	-0.72
		-0.72	0.0	0.0	0.0	37.7	0.0	1.52	0.0	-9.70e-05	0.0	-0.07
						75.4	0.0	1.06	0.0	-9.70e-05	0.0	0.42

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
107	1	0.05	0.0	0.0	-1.94	0.0	0.0	0.38	0.0	1.02	0.0	0.03
		-0.02	0.0	0.0	0.0	8.5	0.0	-0.21	0.0	1.02	0.0	0.05
						17.1	0.0	-1.57	0.0	1.02	0.0	-0.02
107	2	0.04	0.0	0.0	-1.36	0.0	0.0	0.26	0.0	0.77	0.0	0.03
		-0.02	0.0	0.0	0.0	8.5	0.0	-0.16	0.0	0.77	0.0	0.03
						17.1	0.0	-1.10	0.0	0.77	0.0	-0.02
107	3	0.03	0.0	0.0	-1.03	0.0	0.0	0.18	0.0	0.74	0.0	0.02
		-0.01	0.0	0.0	0.0	8.5	0.0	-0.15	0.0	0.74	0.0	0.03
						17.1	0.0	-0.85	0.0	0.74	0.0	-0.01
107	4	0.03	0.0	0.0	-0.90	0.0	0.0	0.15	0.0	0.72	0.0	0.02
		-0.01	0.0	0.0	0.0	8.5	0.0	-0.14	0.0	0.72	0.0	0.03
						17.1	0.0	-0.75	0.0	0.72	0.0	-0.01
108	1	-0.01	0.0	0.0	-0.31	0.0	0.0	-0.83	0.0	3.74e-04	0.0	-0.01
		-0.18	0.0	0.0	0.0	8.6	0.0	-0.99	0.0	3.74e-04	0.0	-0.09
						17.1	0.0	-1.15	0.0	3.74e-04	0.0	-0.18
108	2	-8.90e-03	0.0	0.0	-0.24	0.0	0.0	-0.63	0.0	2.69e-04	0.0	-8.90e-03
		-0.14	0.0	0.0	0.0	8.6	0.0	-0.75	0.0	2.69e-04	0.0	-0.07
						17.1	0.0	-0.87	0.0	2.69e-04	0.0	-0.14
108	3	-7.42e-03	0.0	0.0	-0.22	0.0	0.0	-0.61	0.0	2.24e-04	0.0	-7.42e-03
		-0.13	0.0	0.0	0.0	8.6	0.0	-0.73	0.0	2.24e-04	0.0	-0.06
						17.1	0.0	-0.84	0.0	2.24e-04	0.0	-0.13
108	4	-6.82e-03	0.0	0.0	-0.22	0.0	0.0	-0.60	0.0	2.06e-04	0.0	-6.82e-03
		-0.13	0.0	0.0	0.0	8.6	0.0	-0.71	0.0	2.06e-04	0.0	-0.06
						17.1	0.0	-0.82	0.0	2.06e-04	0.0	-0.13
109	1	3.36	0.0	-1.58e-04	-1.82	0.0	0.0	37.98	0.0	0.01	0.0	0.0
		0.0	0.0	0.0	0.0	4.5	0.0	37.07	0.0	0.01	0.0	1.70
						9.1	0.0	36.16	0.0	0.01	0.0	3.36
109	2	2.33	0.0	-1.10e-04	-1.27	0.0	0.0	26.40	0.0	8.90e-03	0.0	0.0

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		0.0	0.0	0.0	0.0	4.5	0.0	25.77	0.0	8.90e-03	0.0	1.18
						9.1	0.0	25.13	0.0	8.90e-03	0.0	2.33
109	3	1.75	0.0	-8.23e-05	-0.97	0.0	0.0	19.77	0.0	7.42e-03	0.0	0.0
		0.0	0.0	0.0	0.0	4.5	0.0	19.28	0.0	7.42e-03	0.0	0.88
						9.1	0.0	18.80	0.0	7.42e-03	0.0	1.75
109	4	1.51	0.0	-7.12e-05	-0.85	0.0	0.0	17.11	0.0	6.83e-03	0.0	0.0
		0.0	0.0	0.0	0.0	4.5	0.0	16.69	0.0	6.83e-03	0.0	0.77
						9.1	0.0	16.27	0.0	6.83e-03	0.0	1.51
110	1	20.06	0.0	-9.23e-04	-17.91	0.0	0.0	36.13	0.0	5.02e-03	0.0	3.36
		3.36	0.0	0.0	0.0	30.7	0.0	27.19	0.0	5.02e-03	0.0	13.08
						61.4	0.0	18.22	0.0	5.02e-03	0.0	20.06
110	2	13.94	0.0	-6.42e-04	-12.45	0.0	0.0	25.12	0.0	3.84e-03	0.0	2.33
		2.33	0.0	0.0	0.0	30.7	0.0	18.90	0.0	3.84e-03	0.0	9.09
						61.4	0.0	12.66	0.0	3.84e-03	0.0	13.94
110	3	10.43	0.0	-4.80e-04	-9.32	0.0	0.0	18.79	0.0	3.80e-03	0.0	1.75
		1.75	0.0	0.0	0.0	30.7	0.0	14.13	0.0	3.80e-03	0.0	6.80
						61.4	0.0	9.47	0.0	3.80e-03	0.0	10.43
110	4	9.02	0.0	-4.15e-04	-8.06	0.0	0.0	16.26	0.0	3.78e-03	0.0	1.51
		1.51	0.0	0.0	0.0	30.7	0.0	12.23	0.0	3.78e-03	0.0	5.89
						61.4	0.0	8.19	0.0	3.78e-03	0.0	9.02
111	1	3.76	0.0	1.78e-04	-1.92	0.0	0.0	-36.02	0.0	5.03e-03	0.0	3.76
		0.0	0.0	0.0	0.0	5.1	0.0	-36.98	0.0	5.03e-03	0.0	1.91
						10.2	0.0	-37.94	0.0	5.03e-03	0.0	0.0
111	2	2.62	0.0	1.24e-04	-1.34	0.0	0.0	-25.04	0.0	3.85e-03	0.0	2.62
		0.0	0.0	0.0	0.0	5.1	0.0	-25.71	0.0	3.85e-03	0.0	1.33
						10.2	0.0	-26.38	0.0	3.85e-03	0.0	0.0
111	3	1.96	0.0	9.25e-05	-1.03	0.0	0.0	-18.72	0.0	3.81e-03	0.0	1.96
		0.0	0.0	0.0	0.0	5.1	0.0	-19.24	0.0	3.81e-03	0.0	0.99

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						10.2	0.0	-19.75	0.0	3.81e-03	0.0	0.0
111	4	1.69	0.0	8.01e-05	-0.90	0.0	0.0	-16.20	0.0	3.79e-03	0.0	1.69
		0.0	0.0	0.0	0.0	5.1	0.0	-16.65	0.0	3.79e-03	0.0	0.86
						10.2	0.0	-17.10	0.0	3.79e-03	0.0	0.0
112	1	5.53e-03	0.0	0.0	-0.18	0.0	0.0	0.04	0.0	1.89e-05	0.0	5.03e-03
		-1.15e-03	0.0	0.0	0.0	6.0	0.0	-0.05	0.0	1.89e-05	0.0	4.65e-03
						12.0	0.0	-0.14	0.0	1.89e-05	0.0	-1.15e-03
112	2	4.24e-03	0.0	0.0	-0.14	0.0	0.0	0.03	0.0	1.45e-05	0.0	3.85e-03
		-8.31e-04	0.0	0.0	0.0	6.0	0.0	-0.04	0.0	1.45e-05	0.0	3.60e-03
						12.0	0.0	-0.11	0.0	1.45e-05	0.0	-8.31e-04
112	3	4.24e-03	0.0	0.0	-0.14	0.0	0.0	0.03	0.0	1.43e-05	0.0	3.81e-03
		-7.12e-04	0.0	0.0	0.0	6.0	0.0	-0.04	0.0	1.43e-05	0.0	3.63e-03
						12.0	0.0	-0.11	0.0	1.43e-05	0.0	-7.12e-04
112	4	4.24e-03	0.0	0.0	-0.14	0.0	0.0	0.03	0.0	1.42e-05	0.0	3.79e-03
		-6.64e-04	0.0	0.0	0.0	6.0	0.0	-0.04	0.0	1.42e-05	0.0	3.65e-03
						12.0	0.0	-0.11	0.0	1.42e-05	0.0	-6.64e-04
113	1	0.08	0.0	0.0	-5.00	0.0	0.0	2.61	0.0	-1.15e-03	0.0	-0.13
		-0.13	0.0	0.0	0.0	15.7	0.0	0.11	0.0	-1.15e-03	0.0	0.08
						31.3	0.0	-2.38	0.0	-1.15e-03	0.0	-0.10
113	2	0.06	0.0	0.0	-3.46	0.0	0.0	1.81	0.0	-8.31e-04	0.0	-0.09
		-0.09	0.0	0.0	0.0	15.7	0.0	0.07	0.0	-8.31e-04	0.0	0.06
						31.3	0.0	-1.65	0.0	-8.31e-04	0.0	-0.07
113	3	0.04	0.0	0.0	-2.55	0.0	0.0	1.34	0.0	-7.12e-04	0.0	-0.07
		-0.07	0.0	0.0	0.0	15.7	0.0	0.05	0.0	-7.12e-04	0.0	0.04
						31.3	0.0	-1.22	0.0	-7.12e-04	0.0	-0.05
113	4	0.04	0.0	0.0	-2.19	0.0	0.0	1.15	0.0	-6.64e-04	0.0	-0.06
		-0.06	0.0	0.0	0.0	15.7	0.0	0.05	0.0	-6.64e-04	0.0	0.04
						31.3	0.0	-1.05	0.0	-6.64e-04	0.0	-0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
114	1	0.06	0.0	0.0	-5.05	0.0	0.0	2.55	0.0	-1.15e-03	0.0	-0.14
		-0.14	0.0	0.0	0.0	15.7	0.0	0.02	0.0	-1.15e-03	0.0	0.06
						31.3	0.0	-2.50	0.0	-1.15e-03	0.0	-0.13
114	2	0.04	0.0	0.0	-3.50	0.0	0.0	1.76	0.0	-8.31e-04	0.0	-0.10
		-0.10	0.0	0.0	0.0	15.7	0.0	0.01	0.0	-8.31e-04	0.0	0.04
						31.3	0.0	-1.73	0.0	-8.31e-04	0.0	-0.09
114	3	0.03	0.0	0.0	-2.58	0.0	0.0	1.30	0.0	-7.12e-04	0.0	-0.07
		-0.07	0.0	0.0	0.0	15.7	0.0	7.99e-03	0.0	-7.12e-04	0.0	0.03
						31.3	0.0	-1.28	0.0	-7.12e-04	0.0	-0.07
114	4	0.03	0.0	0.0	-2.21	0.0	0.0	1.12	0.0	-6.64e-04	0.0	-0.06
		-0.06	0.0	0.0	0.0	15.7	0.0	6.84e-03	0.0	-6.64e-04	0.0	0.03
						31.3	0.0	-1.10	0.0	-6.64e-04	0.0	-0.06
115	1	-0.10	0.0	0.0	-4.94	0.0	0.0	-0.15	0.0	-1.15e-03	0.0	-0.10
		-0.92	0.0	0.0	0.0	15.7	0.0	-2.63	0.0	-1.15e-03	0.0	-0.32
						31.3	0.0	-5.09	0.0	-1.15e-03	0.0	-0.92
115	2	-0.07	0.0	0.0	-3.42	0.0	0.0	-0.10	0.0	-8.31e-04	0.0	-0.07
		-0.64	0.0	0.0	0.0	15.7	0.0	-1.82	0.0	-8.31e-04	0.0	-0.22
						31.3	0.0	-3.53	0.0	-8.31e-04	0.0	-0.64
115	3	-0.05	0.0	0.0	-2.53	0.0	0.0	-0.08	0.0	-7.12e-04	0.0	-0.05
		-0.47	0.0	0.0	0.0	15.7	0.0	-1.35	0.0	-7.12e-04	0.0	-0.16
						31.3	0.0	-2.60	0.0	-7.12e-04	0.0	-0.47
115	4	-0.04	0.0	0.0	-2.17	0.0	0.0	-0.07	0.0	-6.64e-04	0.0	-0.04
		-0.40	0.0	0.0	0.0	15.7	0.0	-1.16	0.0	-6.64e-04	0.0	-0.14
						31.3	0.0	-2.24	0.0	-6.64e-04	0.0	-0.40
116	1	0.04	0.0	0.0	-3.73	0.0	0.0	1.88	0.0	-1.15e-03	0.0	-0.08
		-0.08	0.0	0.0	0.0	13.0	0.0	9.17e-03	0.0	-1.15e-03	0.0	0.04
						25.9	0.0	-1.85	0.0	-1.15e-03	0.0	-0.08
116	2	0.03	0.0	0.0	-2.59	0.0	0.0	1.30	0.0	-8.32e-04	0.0	-0.06

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.06	0.0	0.0	0.0	13.0	0.0	6.33e-03	0.0	-8.32e-04	0.0	0.03
						25.9	0.0	-1.28	0.0	-8.32e-04	0.0	-0.05
116	3	0.02	0.0	0.0	-1.91	0.0	0.0	0.96	0.0	-7.12e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	13.0	0.0	4.61e-03	0.0	-7.12e-04	0.0	0.02
						25.9	0.0	-0.95	0.0	-7.12e-04	0.0	-0.04
116	4	0.02	0.0	0.0	-1.65	0.0	0.0	0.83	0.0	-6.64e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	13.0	0.0	3.92e-03	0.0	-6.64e-04	0.0	0.02
						25.9	0.0	-0.82	0.0	-6.64e-04	0.0	-0.03
117	1	0.03	0.0	0.0	-3.78	0.0	0.0	1.96	0.0	-1.15e-03	0.0	-0.10
		-0.10	0.0	0.0	0.0	13.0	0.0	0.07	0.0	-1.15e-03	0.0	0.03
						25.9	0.0	-1.81	0.0	-1.15e-03	0.0	-0.08
117	2	0.02	0.0	0.0	-2.62	0.0	0.0	1.36	0.0	-8.32e-04	0.0	-0.07
		-0.07	0.0	0.0	0.0	13.0	0.0	0.05	0.0	-8.32e-04	0.0	0.02
						25.9	0.0	-1.26	0.0	-8.32e-04	0.0	-0.06
117	3	0.02	0.0	0.0	-1.94	0.0	0.0	1.01	0.0	-7.12e-04	0.0	-0.05
		-0.05	0.0	0.0	0.0	13.0	0.0	0.04	0.0	-7.12e-04	0.0	0.02
						25.9	0.0	-0.93	0.0	-7.12e-04	0.0	-0.04
117	4	0.01	0.0	0.0	-1.67	0.0	0.0	0.87	0.0	-6.64e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	13.0	0.0	0.03	0.0	-6.64e-04	0.0	0.01
						25.9	0.0	-0.80	0.0	-6.64e-04	0.0	-0.04
118	1	0.08	0.0	0.0	-3.68	0.0	0.0	2.12	0.0	-1.15e-03	0.0	-0.08
		-0.08	0.0	0.0	0.0	13.0	0.0	0.27	0.0	-1.15e-03	0.0	0.08
						25.9	0.0	-1.56	0.0	-1.15e-03	0.0	-6.84e-03
118	2	0.05	0.0	0.0	-2.55	0.0	0.0	1.47	0.0	-8.32e-04	0.0	-0.05
		-0.05	0.0	0.0	0.0	13.0	0.0	0.19	0.0	-8.32e-04	0.0	0.05
						25.9	0.0	-1.08	0.0	-8.32e-04	0.0	-4.97e-03
118	3	0.04	0.0	0.0	-1.89	0.0	0.0	1.09	0.0	-7.12e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	13.0	0.0	0.14	0.0	-7.12e-04	0.0	0.04

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						25.9	0.0	-0.80	0.0	-7.12e-04	0.0	-4.25e-03
118	4	0.03	0.0	0.0	-1.63	0.0	0.0	0.93	0.0	-6.64e-04	0.0	-0.03
		-0.03	0.0	0.0	0.0	13.0	0.0	0.12	0.0	-6.64e-04	0.0	0.03
						25.9	0.0	-0.69	0.0	-6.64e-04	0.0	-3.96e-03
119	1	-1.82e-03	0.0	0.0	-0.24	0.0	0.0	0.15	0.0	2.22e-04	0.0	-0.01
		-0.01	0.0	0.0	0.0	9.4	0.0	0.03	0.0	2.22e-04	0.0	-2.17e-03
						18.8	0.0	-0.09	0.0	2.22e-04	0.0	-4.96e-03
119	2	-1.31e-03	0.0	0.0	-0.18	0.0	0.0	0.12	0.0	1.31e-04	0.0	-7.95e-03
		-7.95e-03	0.0	0.0	0.0	9.4	0.0	0.02	0.0	1.31e-04	0.0	-1.56e-03
						18.8	0.0	-0.07	0.0	1.31e-04	0.0	-3.75e-03
119	3	-1.10e-03	0.0	0.0	-0.18	0.0	0.0	0.11	0.0	1.32e-05	0.0	-7.26e-03
		-7.26e-03	0.0	0.0	0.0	9.4	0.0	0.02	0.0	1.32e-05	0.0	-1.29e-03
						18.8	0.0	-0.07	0.0	1.32e-05	0.0	-3.61e-03
119	4	-1.01e-03	0.0	0.0	-0.17	0.0	0.0	0.11	0.0	-3.38e-05	0.0	-6.99e-03
		-6.99e-03	0.0	0.0	0.0	9.4	0.0	0.02	0.0	-3.38e-05	0.0	-1.18e-03
						18.8	0.0	-0.07	0.0	-3.38e-05	0.0	-3.56e-03
120	1	-0.01	0.0	0.0	-0.26	0.0	0.0	0.35	0.0	2.22e-04	0.0	-0.05
		-0.05	0.0	0.0	0.0	9.4	0.0	0.22	0.0	2.22e-04	0.0	-0.02
						18.8	0.0	0.09	0.0	2.22e-04	0.0	-0.01
120	2	-7.95e-03	0.0	0.0	-0.20	0.0	0.0	0.26	0.0	1.31e-04	0.0	-0.04
		-0.04	0.0	0.0	0.0	9.4	0.0	0.16	0.0	1.31e-04	0.0	-0.02
						18.8	0.0	0.06	0.0	1.31e-04	0.0	-7.95e-03
120	3	-7.26e-03	0.0	0.0	-0.19	0.0	0.0	0.23	0.0	1.32e-05	0.0	-0.03
		-0.03	0.0	0.0	0.0	9.4	0.0	0.14	0.0	1.32e-05	0.0	-0.02
						18.8	0.0	0.05	0.0	1.32e-05	0.0	-7.26e-03
120	4	-6.99e-03	0.0	0.0	-0.18	0.0	0.0	0.23	0.0	-3.38e-05	0.0	-0.03
		-0.03	0.0	0.0	0.0	9.4	0.0	0.13	0.0	-3.38e-05	0.0	-0.02
						18.8	0.0	0.04	0.0	-3.38e-05	0.0	-6.99e-03

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
121	1	-3.63e-04	0.0	0.0	-0.22	0.0	0.0	0.11	0.0	2.22e-04	0.0	-4.96e-03
		-6.12e-03	0.0	0.0	0.0	9.4	0.0	-7.08e-03	0.0	2.22e-04	0.0	-3.65e-04
						18.8	0.0	-0.11	0.0	2.22e-04	0.0	-6.12e-03
121	2	-2.73e-04	0.0	0.0	-0.17	0.0	0.0	0.08	0.0	1.31e-04	0.0	-3.75e-03
		-4.72e-03	0.0	0.0	0.0	9.4	0.0	-5.75e-03	0.0	1.31e-04	0.0	-2.78e-04
						18.8	0.0	-0.09	0.0	1.31e-04	0.0	-4.72e-03
121	3	-2.49e-04	0.0	0.0	-0.17	0.0	0.0	0.08	0.0	1.32e-05	0.0	-3.61e-03
		-4.73e-03	0.0	0.0	0.0	9.4	0.0	-6.40e-03	0.0	1.32e-05	0.0	-2.63e-04
						18.8	0.0	-0.09	0.0	1.32e-05	0.0	-4.73e-03
121	4	-2.39e-04	0.0	0.0	-0.17	0.0	0.0	0.08	0.0	-3.38e-05	0.0	-3.56e-03
		-4.74e-03	0.0	0.0	0.0	9.4	0.0	-6.66e-03	0.0	-3.38e-05	0.0	-2.57e-04
						18.8	0.0	-0.09	0.0	-3.38e-05	0.0	-4.74e-03
122	1	4.01e-03	0.0	0.0	-0.33	0.0	0.0	0.17	0.0	-0.01	0.0	-8.07e-03
		-8.07e-03	0.0	0.0	0.0	14.3	0.0	1.42e-03	0.0	-0.01	0.0	4.01e-03
						28.5	0.0	-0.17	0.0	-0.01	0.0	-7.67e-03
122	2	3.12e-03	0.0	0.0	-0.26	0.0	0.0	0.13	0.0	-7.81e-03	0.0	-6.22e-03
		-6.22e-03	0.0	0.0	0.0	14.3	0.0	1.40e-03	0.0	-7.81e-03	0.0	3.12e-03
						28.5	0.0	-0.13	0.0	-7.81e-03	0.0	-5.82e-03
122	3	3.17e-03	0.0	0.0	-0.26	0.0	0.0	0.13	0.0	-6.77e-03	0.0	-6.23e-03
		-6.23e-03	0.0	0.0	0.0	14.3	0.0	1.86e-03	0.0	-6.77e-03	0.0	3.17e-03
						28.5	0.0	-0.13	0.0	-6.77e-03	0.0	-5.70e-03
122	4	3.19e-03	0.0	0.0	-0.26	0.0	0.0	0.13	0.0	-6.35e-03	0.0	-6.24e-03
		-6.24e-03	0.0	0.0	0.0	14.3	0.0	2.05e-03	0.0	-6.35e-03	0.0	3.19e-03
						28.5	0.0	-0.13	0.0	-6.35e-03	0.0	-5.65e-03
123	1	0.03	0.0	0.0	-2.88	0.0	0.0	1.42	0.0	1.02	0.0	-0.02
		-0.03	0.0	0.0	0.0	8.5	0.0	-0.03	0.0	1.02	0.0	0.03
						17.1	0.0	-1.47	0.0	1.02	0.0	-0.03
123	2	0.02	0.0	0.0	-2.00	0.0	0.0	0.97	0.0	0.77	0.0	-0.02

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		-0.02	0.0	0.0	0.0	8.5	0.0	-0.03	0.0	0.77	0.0	0.02
						17.1	0.0	-1.03	0.0	0.77	0.0	-0.02
123	3	0.01	0.0	0.0	-1.49	0.0	0.0	0.70	0.0	0.74	0.0	-0.01
		-0.02	0.0	0.0	0.0	8.5	0.0	-0.05	0.0	0.74	0.0	0.01
						17.1	0.0	-0.79	0.0	0.74	0.0	-0.02
123	4	0.01	0.0	0.0	-1.28	0.0	0.0	0.58	0.0	0.72	0.0	-0.01
		-0.02	0.0	0.0	0.0	8.5	0.0	-0.06	0.0	0.72	0.0	0.01
						17.1	0.0	-0.70	0.0	0.72	0.0	-0.02
124	1	-0.18	0.0	0.0	-0.31	0.0	0.0	-4.72	0.0	3.74e-04	0.0	-0.18
		-1.02	0.0	0.0	0.0	8.6	0.0	-4.87	0.0	3.74e-04	0.0	-0.59
						17.1	0.0	-5.03	0.0	3.74e-04	0.0	-1.02
124	2	-0.14	0.0	0.0	-0.23	0.0	0.0	-3.57	0.0	2.69e-04	0.0	-0.14
		-0.77	0.0	0.0	0.0	8.6	0.0	-3.69	0.0	2.69e-04	0.0	-0.45
						17.1	0.0	-3.80	0.0	2.69e-04	0.0	-0.77
124	3	-0.13	0.0	0.0	-0.22	0.0	0.0	-3.42	0.0	2.24e-04	0.0	-0.13
		-0.74	0.0	0.0	0.0	8.6	0.0	-3.54	0.0	2.24e-04	0.0	-0.43
						17.1	0.0	-3.65	0.0	2.24e-04	0.0	-0.74
124	4	-0.13	0.0	0.0	-0.22	0.0	0.0	-3.37	0.0	2.06e-04	0.0	-0.13
		-0.72	0.0	0.0	0.0	8.6	0.0	-3.48	0.0	2.06e-04	0.0	-0.42
						17.1	0.0	-3.59	0.0	2.06e-04	0.0	-0.72
127	1	0.26	0.0	4.02e-06	-0.73	0.0	0.0	-0.05	0.0	2.22e-04	0.0	0.26
		0.09	0.0	0.0	0.0	19.3	0.0	-0.42	0.0	2.22e-04	0.0	0.21
						38.5	0.0	-0.78	0.0	2.22e-04	0.0	0.09
127	2	0.19	0.0	2.93e-06	-0.54	0.0	0.0	-0.03	0.0	1.31e-04	0.0	0.19
		0.07	0.0	0.0	0.0	19.3	0.0	-0.31	0.0	1.31e-04	0.0	0.15
						38.5	0.0	-0.57	0.0	1.31e-04	0.0	0.07
127	3	0.17	0.0	2.57e-06	-0.48	0.0	0.0	-0.02	0.0	1.34e-05	0.0	0.17
		0.06	0.0	0.0	0.0	19.3	0.0	-0.27	0.0	1.34e-05	0.0	0.14

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						38.5	0.0	-0.50	0.0	1.34e-05	0.0	0.06
127	4	0.16	0.0	2.42e-06	-0.46	0.0	0.0	-0.02	0.0	-3.36e-05	0.0	0.16
		0.06	0.0	0.0	0.0	19.3	0.0	-0.25	0.0	-3.36e-05	0.0	0.13
						38.5	0.0	-0.48	0.0	-3.36e-05	0.0	0.06
128	1	0.82	0.0	5.41e-06	-3.61	0.0	0.0	-0.03	0.0	-1.15e-03	0.0	0.82
		0.38	0.0	0.0	0.0	11.9	0.0	-1.84	0.0	-1.15e-03	0.0	0.70
						23.8	0.0	-3.64	0.0	-1.15e-03	0.0	0.38
128	2	0.57	0.0	3.75e-06	-2.50	0.0	0.0	-0.02	0.0	-8.31e-04	0.0	0.57
		0.26	0.0	0.0	0.0	11.9	0.0	-1.28	0.0	-8.31e-04	0.0	0.49
						23.8	0.0	-2.52	0.0	-8.31e-04	0.0	0.26
128	3	0.42	0.0	2.77e-06	-1.85	0.0	0.0	-0.01	0.0	-7.12e-04	0.0	0.42
		0.19	0.0	0.0	0.0	11.9	0.0	-0.94	0.0	-7.12e-04	0.0	0.36
						23.8	0.0	-1.87	0.0	-7.12e-04	0.0	0.19
128	4	0.36	0.0	2.38e-06	-1.59	0.0	0.0	-0.01	0.0	-6.64e-04	0.0	0.36
		0.17	0.0	0.0	0.0	11.9	0.0	-0.81	0.0	-6.64e-04	0.0	0.31
						23.8	0.0	-1.60	0.0	-6.64e-04	0.0	0.17
129	1	1.24	0.0	4.43e-05	-1.42	0.0	0.0	0.20	0.0	2.84e-03	0.0	1.23
		0.85	0.0	0.0	0.0	37.7	0.0	-0.49	0.0	2.84e-03	0.0	1.17
						75.4	0.0	-1.22	0.0	2.84e-03	0.0	0.85
129	2	0.93	0.0	3.36e-05	-1.07	0.0	0.0	0.15	0.0	1.66e-03	0.0	0.93
		0.64	0.0	0.0	0.0	37.7	0.0	-0.38	0.0	1.66e-03	0.0	0.89
						75.4	0.0	-0.92	0.0	1.66e-03	0.0	0.64
129	3	0.89	0.0	3.23e-05	-1.01	0.0	0.0	0.13	0.0	4.05e-04	0.0	0.88
		0.61	0.0	0.0	0.0	37.7	0.0	-0.37	0.0	4.05e-04	0.0	0.84
						75.4	0.0	-0.88	0.0	4.05e-04	0.0	0.61
129	4	0.87	0.0	3.17e-05	-0.99	0.0	0.0	0.12	0.0	-9.70e-05	0.0	0.87
		0.59	0.0	0.0	0.0	37.7	0.0	-0.36	0.0	-9.70e-05	0.0	0.82
						75.4	0.0	-0.86	0.0	-9.70e-05	0.0	0.59

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
130	1	25.73	0.0	3.53e-04	-18.08	0.0	0.0	0.22	0.0	4.99e-03	0.0	25.73
		20.31	0.0	0.0	0.0	30.7	0.0	-8.81	0.0	4.99e-03	0.0	24.41
						61.4	0.0	-17.86	0.0	4.99e-03	0.0	20.31
130	2	17.88	0.0	2.45e-04	-12.57	0.0	0.0	0.15	0.0	3.82e-03	0.0	17.88
		14.12	0.0	0.0	0.0	30.7	0.0	-6.12	0.0	3.82e-03	0.0	16.97
						61.4	0.0	-12.41	0.0	3.82e-03	0.0	14.12
130	3	13.38	0.0	1.83e-04	-9.40	0.0	0.0	0.11	0.0	3.78e-03	0.0	13.38
		10.56	0.0	0.0	0.0	30.7	0.0	-4.58	0.0	3.78e-03	0.0	12.69
						61.4	0.0	-9.29	0.0	3.78e-03	0.0	10.56
130	4	11.57	0.0	1.59e-04	-8.13	0.0	0.0	0.10	0.0	3.77e-03	0.0	11.57
		9.14	0.0	0.0	0.0	30.7	0.0	-3.96	0.0	3.77e-03	0.0	10.98
						61.4	0.0	-8.03	0.0	3.77e-03	0.0	9.14
131	1	0.26	0.0	-3.66e-06	-0.82	0.0	0.0	0.78	0.0	2.22e-04	0.0	0.12
		0.12	0.0	0.0	0.0	19.3	0.0	0.35	0.0	2.22e-04	0.0	0.23
						38.5	0.0	-0.05	0.0	2.22e-04	0.0	0.26
131	2	0.19	0.0	-2.69e-06	-0.60	0.0	0.0	0.57	0.0	1.31e-04	0.0	0.09
		0.09	0.0	0.0	0.0	19.3	0.0	0.26	0.0	1.31e-04	0.0	0.17
						38.5	0.0	-0.03	0.0	1.31e-04	0.0	0.19
131	3	0.17	0.0	-2.39e-06	-0.52	0.0	0.0	0.50	0.0	1.34e-05	0.0	0.07
		0.07	0.0	0.0	0.0	19.3	0.0	0.23	0.0	1.34e-05	0.0	0.15
						38.5	0.0	-0.02	0.0	1.34e-05	0.0	0.17
131	4	0.16	0.0	-2.27e-06	-0.49	0.0	0.0	0.47	0.0	-3.36e-05	0.0	0.07
		0.07	0.0	0.0	0.0	19.3	0.0	0.22	0.0	-3.36e-05	0.0	0.14
						38.5	0.0	-0.02	0.0	-3.36e-05	0.0	0.16
132	1	0.82	0.0	-5.34e-06	-3.66	0.0	0.0	3.63	0.0	-1.15e-03	0.0	0.39
		0.39	0.0	0.0	0.0	11.9	0.0	1.80	0.0	-1.15e-03	0.0	0.71
						23.8	0.0	-0.03	0.0	-1.15e-03	0.0	0.82
132	2	0.57	0.0	-3.70e-06	-2.54	0.0	0.0	2.52	0.0	-8.31e-04	0.0	0.27

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
		0.27	0.0	0.0	0.0	11.9	0.0	1.24	0.0	-8.31e-04	0.0	0.49
						23.8	0.0	-0.02	0.0	-8.31e-04	0.0	0.57
132	3	0.42	0.0	-2.73e-06	-1.87	0.0	0.0	1.86	0.0	-7.12e-04	0.0	0.20
		0.20	0.0	0.0	0.0	11.9	0.0	0.92	0.0	-7.12e-04	0.0	0.36
						23.8	0.0	-0.01	0.0	-7.12e-04	0.0	0.42
132	4	0.36	0.0	-2.35e-06	-1.61	0.0	0.0	1.60	0.0	-6.64e-04	0.0	0.17
		0.17	0.0	0.0	0.0	11.9	0.0	0.79	0.0	-6.64e-04	0.0	0.31
						23.8	0.0	-0.01	0.0	-6.64e-04	0.0	0.36
133	1	1.23	0.0	-5.58e-05	-1.30	0.0	0.0	1.51	0.0	2.84e-03	0.0	0.58
		0.58	0.0	0.0	0.0	37.7	0.0	0.87	0.0	2.84e-03	0.0	1.02
						75.4	0.0	0.20	0.0	2.84e-03	0.0	1.23
133	2	0.93	0.0	-4.21e-05	-0.99	0.0	0.0	1.14	0.0	1.66e-03	0.0	0.44
		0.44	0.0	0.0	0.0	37.7	0.0	0.65	0.0	1.66e-03	0.0	0.77
						75.4	0.0	0.15	0.0	1.66e-03	0.0	0.93
133	3	0.88	0.0	-3.99e-05	-0.95	0.0	0.0	1.09	0.0	4.05e-04	0.0	0.42
		0.42	0.0	0.0	0.0	37.7	0.0	0.62	0.0	4.05e-04	0.0	0.74
						75.4	0.0	0.13	0.0	4.05e-04	0.0	0.88
133	4	0.87	0.0	-3.91e-05	-0.94	0.0	0.0	1.06	0.0	-9.70e-05	0.0	0.42
		0.42	0.0	0.0	0.0	37.7	0.0	0.60	0.0	-9.70e-05	0.0	0.73
						75.4	0.0	0.12	0.0	-9.70e-05	0.0	0.87
134	1	25.73	0.0	-3.66e-04	-18.00	0.0	0.0	18.22	0.0	4.98e-03	0.0	20.06
		20.06	0.0	0.0	0.0	30.7	0.0	9.23	0.0	4.98e-03	0.0	24.27
						61.4	0.0	0.22	0.0	4.98e-03	0.0	25.73
134	2	17.88	0.0	-2.55e-04	-12.51	0.0	0.0	12.66	0.0	3.82e-03	0.0	13.94
		13.94	0.0	0.0	0.0	30.7	0.0	6.42	0.0	3.82e-03	0.0	16.87
						61.4	0.0	0.15	0.0	3.82e-03	0.0	17.88
134	3	13.38	0.0	-1.90e-04	-9.36	0.0	0.0	9.47	0.0	3.78e-03	0.0	10.43
		10.43	0.0	0.0	0.0	30.7	0.0	4.80	0.0	3.78e-03	0.0	12.62

RELAZIONE DI CALCOLO

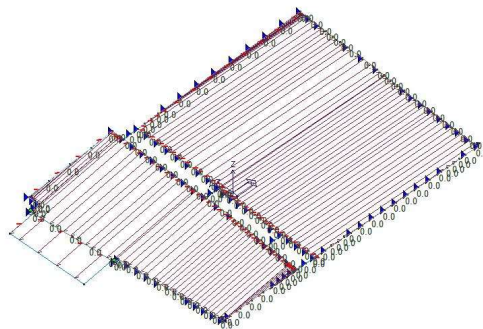
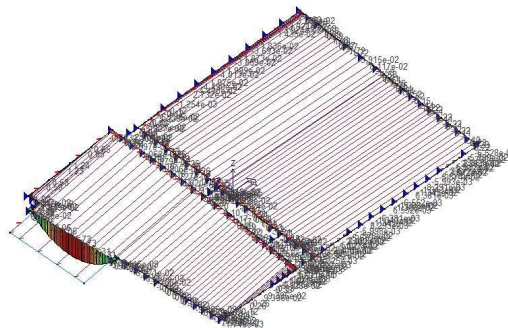
INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
						61.4	0.0	0.11	0.0	3.78e-03	0.0	13.38
134	4	11.57	0.0	-1.65e-04	-8.10	0.0	0.0	8.19	0.0	3.76e-03	0.0	9.02
		9.02	0.0	0.0	0.0	30.7	0.0	4.15	0.0	3.76e-03	0.0	10.92
						61.4	0.0	0.10	0.0	3.76e-03	0.0	11.57
135	1	0.09	0.0	6.85e-06	-0.65	0.0	0.0	-0.78	0.0	2.22e-04	0.0	0.09
		-0.33	0.0	0.0	0.0	19.3	0.0	-1.11	0.0	2.22e-04	0.0	-0.09
						38.5	0.0	-1.43	0.0	2.22e-04	0.0	-0.33
135	2	0.07	0.0	5.02e-06	-0.48	0.0	0.0	-0.57	0.0	1.31e-04	0.0	0.07
		-0.25	0.0	0.0	0.0	19.3	0.0	-0.82	0.0	1.31e-04	0.0	-0.06
						38.5	0.0	-1.05	0.0	1.31e-04	0.0	-0.25
135	3	0.06	0.0	4.44e-06	-0.44	0.0	0.0	-0.50	0.0	1.34e-05	0.0	0.06
		-0.22	0.0	0.0	0.0	19.3	0.0	-0.73	0.0	1.34e-05	0.0	-0.06
						38.5	0.0	-0.94	0.0	1.34e-05	0.0	-0.22
135	4	0.06	0.0	4.21e-06	-0.42	0.0	0.0	-0.48	0.0	-3.36e-05	0.0	0.06
		-0.21	0.0	0.0	0.0	19.3	0.0	-0.69	0.0	-3.36e-05	0.0	-0.05
						38.5	0.0	-0.90	0.0	-3.36e-05	0.0	-0.21
136	1	0.38	0.0	1.11e-05	-3.56	0.0	0.0	-3.64	0.0	-1.15e-03	0.0	0.38
		-0.92	0.0	0.0	0.0	11.9	0.0	-5.43	0.0	-1.15e-03	0.0	-0.16
						23.8	0.0	-7.21	0.0	-1.15e-03	0.0	-0.92
136	2	0.26	0.0	7.70e-06	-2.47	0.0	0.0	-2.52	0.0	-8.31e-04	0.0	0.26
		-0.64	0.0	0.0	0.0	11.9	0.0	-3.76	0.0	-8.31e-04	0.0	-0.11
						23.8	0.0	-4.99	0.0	-8.31e-04	0.0	-0.64
136	3	0.19	0.0	5.69e-06	-1.83	0.0	0.0	-1.87	0.0	-7.12e-04	0.0	0.19
		-0.47	0.0	0.0	0.0	11.9	0.0	-2.78	0.0	-7.12e-04	0.0	-0.08
						23.8	0.0	-3.69	0.0	-7.12e-04	0.0	-0.47
136	4	0.17	0.0	4.89e-06	-1.57	0.0	0.0	-1.60	0.0	-6.64e-04	0.0	0.17
		-0.40	0.0	0.0	0.0	11.9	0.0	-2.39	0.0	-6.64e-04	0.0	-0.07
						23.8	0.0	-3.17	0.0	-6.64e-04	0.0	-0.40

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Cmb	M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3	Pos.	N	V 2	V 3	T	M 2	M 3
137	1	0.85	0.0	1.12e-04	-1.54	0.0	0.0	-1.22	0.0	2.84e-03	0.0	0.85
		-0.64	0.0	0.0	0.0	37.7	0.0	-1.97	0.0	2.84e-03	0.0	0.25
						75.4	0.0	-2.76	0.0	2.84e-03	0.0	-0.64
137	2	0.64	0.0	8.43e-05	-1.15	0.0	0.0	-0.92	0.0	1.66e-03	0.0	0.64
		-0.48	0.0	0.0	0.0	37.7	0.0	-1.49	0.0	1.66e-03	0.0	0.19
						75.4	0.0	-2.07	0.0	1.66e-03	0.0	-0.48
137	3	0.61	0.0	8.03e-05	-1.07	0.0	0.0	-0.88	0.0	4.05e-04	0.0	0.61
		-0.46	0.0	0.0	0.0	37.7	0.0	-1.41	0.0	4.05e-04	0.0	0.18
						75.4	0.0	-1.95	0.0	4.05e-04	0.0	-0.46
137	4	0.59	0.0	7.86e-05	-1.04	0.0	0.0	-0.86	0.0	-9.70e-05	0.0	0.59
		-0.45	0.0	0.0	0.0	37.7	0.0	-1.38	0.0	-9.70e-05	0.0	0.17
						75.4	0.0	-1.90	0.0	-9.70e-05	0.0	-0.45
138	1	20.31	0.0	9.17e-04	-18.16	0.0	0.0	-17.86	0.0	5.02e-03	0.0	20.31
		3.76	0.0	0.0	0.0	30.7	0.0	-26.93	0.0	5.02e-03	0.0	13.43
						61.4	0.0	-36.02	0.0	5.02e-03	0.0	3.76
138	2	14.12	0.0	6.38e-04	-12.62	0.0	0.0	-12.41	0.0	3.84e-03	0.0	14.12
		2.62	0.0	0.0	0.0	30.7	0.0	-18.72	0.0	3.84e-03	0.0	9.34
						61.4	0.0	-25.04	0.0	3.84e-03	0.0	2.62
138	3	10.56	0.0	4.77e-04	-9.44	0.0	0.0	-9.29	0.0	3.80e-03	0.0	10.56
		1.96	0.0	0.0	0.0	30.7	0.0	-14.00	0.0	3.80e-03	0.0	6.98
						61.4	0.0	-18.72	0.0	3.80e-03	0.0	1.96
138	4	9.14	0.0	4.13e-04	-8.17	0.0	0.0	-8.03	0.0	3.78e-03	0.0	9.14
		1.69	0.0	0.0	0.0	30.7	0.0	-12.11	0.0	3.78e-03	0.0	6.04
						61.4	0.0	-16.20	0.0	3.78e-03	0.0	1.69
Trave		M3 mx/mn	M2 mx/mn	D 2 / D 3	Q 2 / Q 3		N	V 2	V 3	T		
		-1.72	0.0	-9.23e-04	-18.16		0.0	-37.94	0.0	-0.64		
		25.73	0.0	9.17e-04	0.0		0.0	37.98	0.0	1.02		

[illegible]

RISULTATI 001 SLU	
Momento 3-3 [kN m]	
25.73	
23.90	
22.07	
20.24	
18.41	
16.58	
14.75	
12.92	
11.09	
9.26	
7.43	
5.60	
3.77	
1.94	
0.11	
-1.72	

Modello_Cop.P1*

VERIFICHE ELEMENTI TRAVE IN C.A.

LEGENDA TABELLA VERIFICHE ELEMENTI TRAVE IN C.A.

In tabella vengono riportati per ogni elemento il numero identificativo ed il codice di verifica con le sigle **Ok** o **NV**.

Nel caso in cui si sia proceduto alla progettazione con il metodo degli stati limite (**S.L.**) vengono riportati: il rapporto x/d , le verifiche per sollecitazioni proporzionali e la verifica per compressione media con l'indicazione delle combinazioni in cui si sono attinti i rispettivi valori.

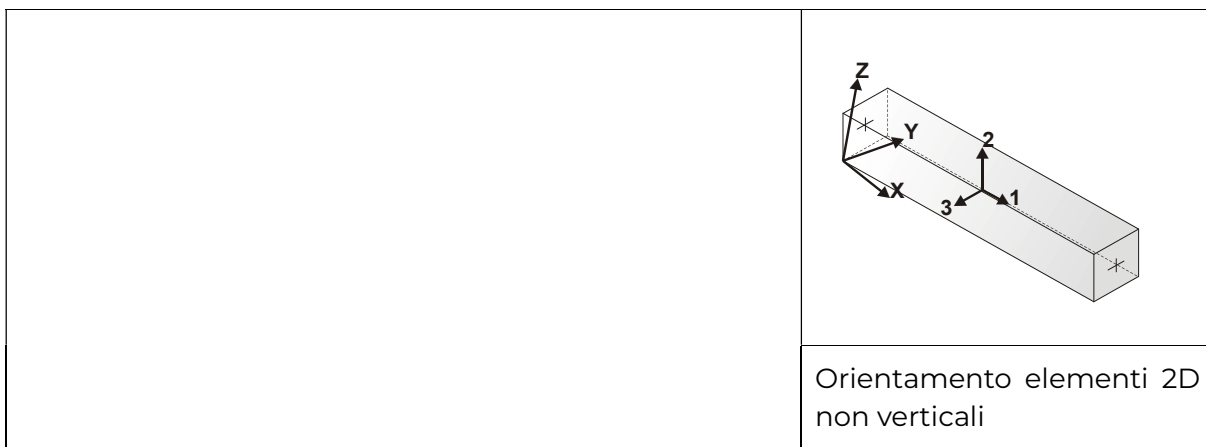
Nel caso in cui si sia proceduto alla progettazione con le tensioni ammissibili (**T.A.**) vengono riportate le massime tensioni nell'elemento (massima compressione nel calcestruzzo, massima compressione media nel calcestruzzo, massima tensione nell'acciaio, massima tensione tangenziale) con l'indicazione delle combinazioni in cui si sono attinti i rispettivi valori.

Nel caso in cui la struttura abbia comportamento dissipativo e sia prevista la progettazione con il criterio della gerarchia delle resistenze (**G.R.**) vengono riportate le verifiche di sovreresistenza e del nodo.

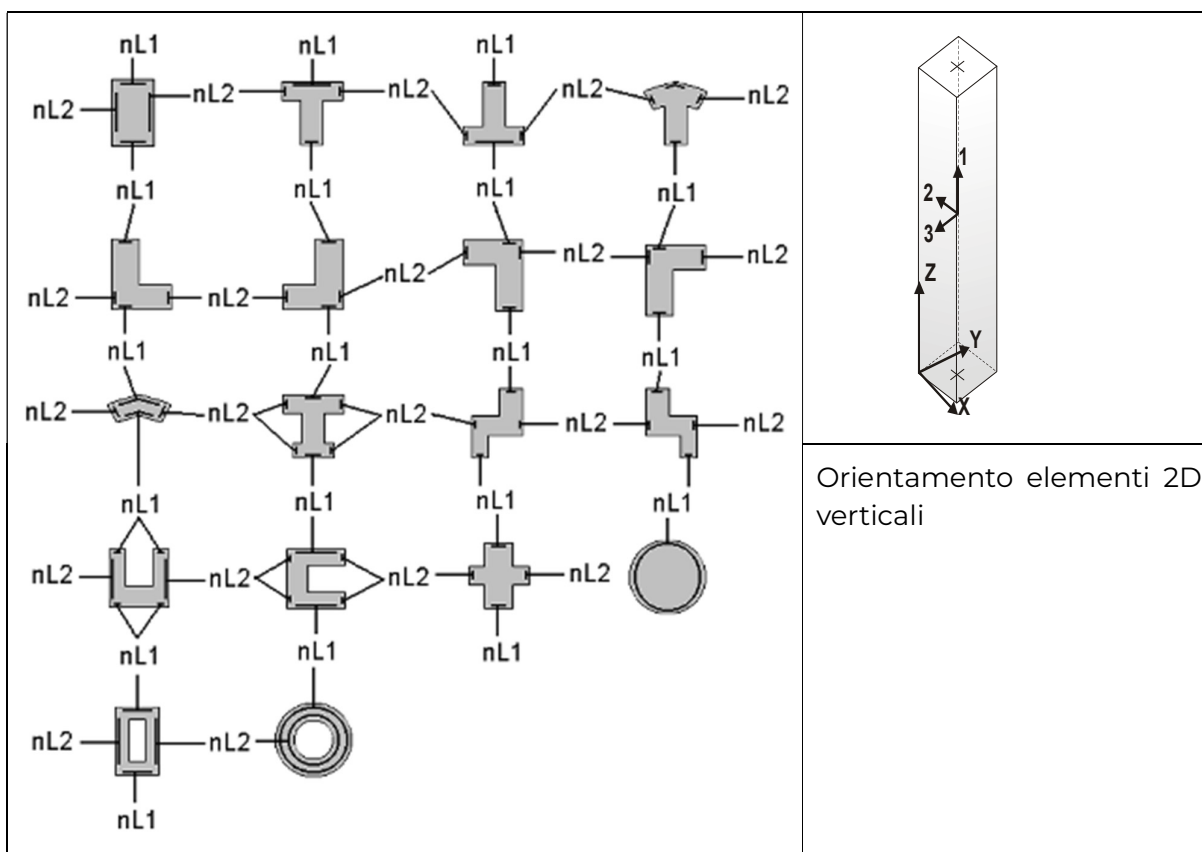
Per gli elementi tipo pilastro sono riportati numero e diametro dei ferri di vertice, numero e diametro di ferri disposti lungo i lati L1 (paralleli alla base della sezione) e lungo i lati L2 (paralleli all'altezza della sezione).

Per gli elementi tipo trave sono riportati infine le quantità di armatura inferiore e superiore.

Schema della distribuzione delle armature longitudinali



INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



Simbologia adottata nelle tabelle di verifica

Per le verifiche agli S.L. dei pilastri è presente una tabella con i simboli di seguito descritti:

M_P X Y	Numero della pilastrata (P) e posizione in pianta (X,Y)
Pilas.	numero identificativo dell'elemento D2
Note	Codici identificativi delle sezione (s) e materiale (m) pilastro
Stato	Codici relativi all'esito delle verifiche effettuate appresso descritte
Quota	Quota sezione di verifica
%Af	Percentuale di area di armatura rispetto a quella di calcestruzzo
r. snell.	Rapporto di snellezza λ su λ^* : valore superiore a 1 per elementi snelli nel caso in cui viene effettuata la verifica con il metodo diretto dello stato di equilibrio

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Armat. long.	Numero e diametro (d) dei ferri di armatura longitudinale distinti in ferri di vertice + ferri di lato nelle posizioni nL1 e nL2, come da schemi in figura precedente
V N/M	Verifica a pressoflessione con rapporto E_d/R_d : valore minore o uguale a 1 per verifica positiva
V N sis	Verifica a compressione solo calcestruzzo con rapporto N_{sd}/N_{rd} ed N_{rd} calcolato come al punto 7.4.4.2.1: valore minore o uguale a 1 per verifica positiva
Staffe	Dati tratto di staffatura oggetto di verifica, nello specifico: numero delle braccia, diametro, passo, lunghezza L tratto
V V/T cls	Verifica a taglio/torsione con rapporto V_{ed}/V_{rd} : valore minore o uguale a 1 per verifica positiva
Rif. cmb.	Riferimento combinazioni da cui si generano le verifiche più gravose per il pilastro

Per le verifiche di gerarchia delle resistenze dei pilastri è presente una tabella con i simboli di seguito descritti:

Pilas.	numero identificativo dell'elemento D2 pilastro
sovr. Xi (Xf)	Verifica sovrarresistenza come da formula 7.4.4 in direzione X, alla base (i) ed alla sommità (f): rapporto tra i momenti resistenti dei pilastri e delle travi. La verifica è positiva se maggiore del γ_{Rd} adottato
sovr. Yi (Yf)	Verifica sovrarresistenza come da formula 7.4.4 in direzione Y, alla base (i) ed alla sommità (f): rapporto tra i momenti resistenti dei pilastri e delle travi. La verifica è positiva se maggiore del γ_{Rd} adottato
M 2-2 i (f)	Valore del momento resistente 2-2 alla base (i) ed alla sommità (f) con massimo momento in presenza dello sforzo normale di calcolo
M 3-3 i (f)	Valore del momento resistente 3-3 alla base (i) ed alla sommità (f) con massimo momento in presenza dello sforzo normale di calcolo
Luce per V	Luce di calcolo per la definizione del taglio (generato dai momenti resistenti)

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

V M2-2 Valore del taglio generato dai momenti resistenti 2-2 (3-3)
(M3-3)

Per le verifiche dei dettagli costruttivi relativi alla duttilità è presente una tabella con i simboli di seguito descritti:

(Non presente nel caso di comportamento strutturale non dissipativo)

Pilas	Numero identificativo D2 pilastro
ni	Sforzo assiale adimensionalizzato di progetto relativo alla combinazione sismica SLV
alfaomega	Prodotto tra il coefficiente di efficacia del confinamento e il rapporto meccanico dell'armatura trasversale di confinamento all'interno del nodo
V.7.4.29 (3-3)	2-2 Rapporto tra la domanda di staffe minima nel nodo e il rapporto meccanico dell'armatura trasversale di confinamento inserito all'interno del nodo in direzione 2 (3)
V. 7.4.29 Stato	Codici relativi all'esito della verifica 7.4.29
dmu_fi (3-3)	2-2 Domanda in duttilità di curvatura in direzione 2 (3)
cmu_fi (3-3)	2-2 Capacità in duttilità di curvatura in direzione 2 (3)
V. dutt. (3-3)	2-2 Rapporto tra la domanda in duttilità di curvatura e la capacità in duttilità di curvatura in direzione 2 (3)

Per le verifiche dei nodi trave-pilastro di elementi nuovi è presente una tabella con i simboli di seguito descritti:

Nodo	Numero identificativo del nodo trave-pilastro
Stato	Esito delle verifiche
Pilastro	Numero identificativo D2 pilastro

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Diam st	Diametro staffe nodo
Passo	Passo staffe nodo
n. br. 2 (3)	Numero braccia staffe per il taglio in direzione 2 (3)
Bj2 (3)	Larghezza effettiva del nodo per il taglio in direzione 2 (3)
Hjc2 (3)	Distanza tra le giaciture più esterne delle armature del pilastro per il taglio in direzione 2 (3)
V. 7.4.8	Rapporto tra il taglio V_{jbd} e il taglio resistente come da formula 7.4.8
V. Ash	Rapporto tra il passo staffe calcolato secondo il capitolo 7.4.4.3.1. e il passo staffe effettivamente inserita nel nodo. Nel caso di valore indica passo staffe utilizzato deriva dalle formule presenti nel paragrafo 7.4.4.3.1. Nel caso di valore minore di 1 il passo staffe utilizzato deriva del pilastro superiore o inferiore al nodo
7.4.10	Check passo staffe valutato in funzione della formula 7.4.10: SI il passo staffe è calcolato utilizzando la formula 7.4.10; NO il passo staffe è calcolato utilizzando le formule 7.4.11 e/o 7.4.12; NR calcolo passo staffe non richiesto;
Rif. comb.	Riferimento combinazioni da cui si generano le verifiche più gravose per il nodo

Per le verifiche dei nodi trave-pilastro di elementi esistenti è presente una tabella con i simboli di seguito descritti:

Pilastro I	Numero identificativo D2 del pilastro inferiore.
Pilastro S	Numero identificativo D2 del pilastro superiore.
Nodo	Numero identificativo del nodo trave-pilastro.
SL cod	Stato limite di riferimento e relativo esito delle verifiche.
ver. (+)	Coefficiente di sicurezza, calcolato come rapporto D/C, nei riguardi della verifica di resistenza a trazione

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

V +	Azione di Taglio presente al di sopra del nodo nella verifica di resistenza a trazione
V + af s	Sollecitazione di trazione presente nell' armatura longitudinale superiore della trave nella verifica di resistenza a trazione
N +	Azione Assiale presente al di sopra del nodo nella verifica di resistenza a trazione
ver. (-)	Coefficiente di sicurezza, calcolato come rapporto D/C, nei riguardi della verifica di resistenza a compressione
V -	Azione di Taglio presente al di sopra del nodo nella verifica di resistenza a compressione
V - af s	Sollecitazione di trazione presente nell' armatura longitudinale superiore della trave nella verifica di resistenza a compressione
N -	Azione Assiale presente al di sopra del nodo nella verifica di resistenza a compressione
AreaV2	Area resistente del nodo in direzione 2 ($A_{j2}=b_{j2}*h_{jc2}$).
AreaV3	Area resistente del nodo in direzione 3 ($A_{j3}=b_{j3}*h_{jc3}$).
Rif. comb.	Combinazione (direzione) di riferimento nella verifica di trazione.

Per le verifiche agli S.L. delle travi è presente una tabella con i simboli di seguito descritti:

M_T Z P	Numero della travata (T), quota media (Z), n° pilastrata iniziale (P) e finale (P) (nodo in assenza di pilastrata)
Trave	numero identificativo dell'elemento D2
Note	Codici identificativi sezione (s) e materiale (m) trave; sono inoltre presenti le sigle relative all'esito delle verifiche effettuate appresso descritte
%Af	Percentuale di area di armatura rispetto a quella di calcestruzzo
Af inf.	Area di armatura longitudinale posta all'intradosso
Af sup	Area di armatura longitudinale posta all'estradosso
Af long.	Area complessiva armatura longitudinale

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

x/d	rapporto tra posizione dell'asse neutro e altezza utile
V N/M	Verifica a pressoflessione rapporto E_d/R_d : valore minore o uguale a 1 per verifica positiva
Staffe	Dati tratto di staffatura oggetto di verifica, nello specifico: numero delle braccia, diametro, passo, lunghezza L tratto
V V/T cls	Verifica a taglio/torsione con rapporto V_{ed}/V_{rd} : valore minore o uguale a 1 per verifica positiva
Rif. cmb.	Riferimento combinazioni da cui si generano le verifiche più gravose per la trave

Per le verifiche di gerarchia delle resistenze delle travi è presente una tabella con i simboli di seguito descritti:

Trave	numero identificativo dell'elemento D2 trave
M negativo i (f)	Valore del momento resistente negativo all' estremità iniziale i (finale f) della trave
M positivo i (f)	Valore del momento resistente positivo all' estremità iniziale i (finale f) della trave
Luce per V	Luce di calcolo per la definizione del taglio (generato dai momenti resistenti)
V M-i M+f	Taglio generato dai momenti resistenti negativo i e positivo f
V M+i M-f	Taglio generato dai momenti resistenti positivo i e negativo f
$V_{Ed, min}$	Valore di taglio minimo per verifica condizioni p.to 7.4.4.1.1 armatura diagonale (solo per CD "A")
$V_{Ed, max}$	Valore di taglio massimo per verifica condizioni p.to 7.4.4.1.1 armatura diagonale (solo per CD "A")
V_{r1}	Valore di taglio come da formula 7.4.1 per armatura diagonale (solo per CD "A")
A_s	Area singolo ordine armature diagonali come da formula 7.4.2 (solo per CD "A")

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Per le verifiche a taglio ciclico di travi e pilastri esistenti è presente una tabella con i simboli di seguito descritti:

Trave/Pilastro	Numero identificativo dell'elemento D2 trave/pilastro
V. SLV	Codice relativo all'esito delle verifiche
Nodo	Numero identificativo del nodo di verifica
Ver. VC	Fattore di sicurezza nei confronti della verifica a taglio ciclico (verificato se < 1.00)
Direz.	Direzione di verifica
N fr	Valore di sforzo normale calcolato con fattore di comportamento fragile
V fr	Valore di taglio calcolato con fattore di comportamento fragile
M fr	Valore di momento calcolato con fattore di comportamento fragile
N dutt	Valore di sforzo normale calcolato con fattore di comportamento duttile
LV	Lunghezza di taglio
Mud,pl	Parte plastica della domanda di duttilità
V cic	Resistenza a taglio in condizioni cicliche (C8.7.2.8)
Cmb	Riferimento combinazioni da cui si generano le verifiche più gravose

							M_T= 1	Z=0.0	N=24	N=29			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
		cm									L=cm		
23	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.87e-05	0.05	0.03	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	8.35e-03	0.01	6.29e-03	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	9.03e-03	0.07	0.04	2d8/15 L=32	1,1,1	
96	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	9.03e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	4.10e-03	1.57e-03	3.08e-04	2d8/15 L=32	1,1,1	

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

		32.4	1.17	4.0	4.0	0.0	0.27	8.60e-03	0.06	0.04	2d8/15 L=32	1,1,1	
94	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.60e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	4.33e-03	1.13e-03	2.23e-05	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	8.64e-03	0.06	0.04	2d8/15 L=32	1,1,1	
98	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.64e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	4.33e-03	1.11e-03	5.07e-06	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	8.66e-03	0.06	0.04	2d8/15 L=32	1,1,1	
93	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.66e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	4.34e-03	1.10e-03	0.0	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	8.66e-03	0.06	0.04	2d8/15 L=32	1,1,1	
97	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.66e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	4.27e-03	1.25e-03	9.60e-05	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	8.80e-03	0.06	0.04	2d8/15 L=32	1,1,1	
95	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.80e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	5.56e-03	4.02e-03	1.90e-03	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	6.09e-03	0.05	0.03	2d8/15 L=32	1,1,1	
2	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.09e-03	3.26e-03	1.40e-03	2d8/15 L=32	1,1,1	
	s=1,m=3	16.2	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=32	1,1,1	
		32.4	1.17	4.0	4.0	0.0	0.27	0.06	0.12	0.07	2d8/15 L=32	1,1,1	
22	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.06	0.17	0.11	2d8/15 L=24	1,1,1	
	s=1,m=3	12.0	1.17	4.0	4.0	0.0	0.27	0.01	0.13	0.08	2d8/15 L=24	1,1,1	
		24.0	1.17	4.0	4.0	0.0	0.27	0.03	0.09	0.05	2d8/15 L=24	1,1,1	
56	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.03	0.09	0.05	2d8/15 L=24	1,1,1	
	s=1,m=3	12.0	1.17	4.0	4.0	0.0	0.27	0.05	0.05	0.03	2d8/15 L=24	1,1,1	
		24.0	1.17	4.0	4.0	0.0	0.27	0.05	7.14e-03	5.75e-04	2d8/15 L=24	1,1,1	
45	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.05	7.14e-03	5.75e-04	2d8/15 L=24	1,1,1	
	s=1,m=3	12.0	1.17	4.0	4.0	0.0	0.27	0.05	0.05	0.03	2d8/15 L=24	1,1,1	
		24.0	1.17	4.0	4.0	0.0	0.27	0.02	0.09	0.05	2d8/15 L=24	1,1,1	
40	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.02	0.09	0.05	2d8/15 L=24	1,1,1	

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	s=1,m=3	12.0	1.17	4.0	4.0	0.0	0.27	0.01	0.13	0.08	2d8/15 L=24	1,1,1	
		24.0	1.17	4.0	4.0	0.0	0.27	0.06	0.17	0.11	2d8/15 L=24	1,1,1	
21	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.06	0.13	0.08	2d8/15 L=52	1,1,1	
	s=1,m=3	26.0	1.17	4.0	4.0	0.0	0.27	2.10e-03	0.04	0.02	2d8/15 L=52	1,1,1	
		52.0	1.17	4.0	4.0	0.0	0.27	8.95e-03	0.06	0.04	2d8/15 L=52	1,1,1	
1	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.95e-03	0.05	0.03	2d8/15 L=52	1,1,1	
	s=1,m=3	26.0	1.17	4.0	4.0	0.0	0.27	4.78e-03	0.04	0.03	2d8/15 L=52	1,1,1	
		52.0	1.17	4.0	4.0	0.0	0.27	0.07	0.13	0.08	2d8/15 L=52	1,1,1	
20	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.07	0.18	0.11	2d8/15 L=25	1,1,1	
	s=1,m=3	12.5	1.17	4.0	4.0	0.0	0.27	0.01	0.13	0.09	2d8/15 L=25	1,1,1	
		25.1	1.17	4.0	4.0	0.0	0.27	0.03	0.09	0.06	2d8/15 L=25	1,1,1	
55	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.03	0.09	0.06	2d8/15 L=25	1,1,1	
	s=1,m=3	12.5	1.17	4.0	4.0	0.0	0.27	0.05	0.05	0.03	2d8/15 L=25	1,1,1	
		25.1	1.17	4.0	4.0	0.0	0.27	0.06	1.51e-03	3.70e-04	2d8/15 L=25	1,1,1	
44	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.06	1.51e-03	3.70e-04	2d8/15 L=25	1,1,1	
	s=1,m=3	12.5	1.17	4.0	4.0	0.0	0.27	0.05	0.04	0.03	2d8/15 L=25	1,1,1	
		25.1	1.17	4.0	4.0	0.0	0.27	0.03	0.09	0.06	2d8/15 L=25	1,1,1	
53	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.03	0.09	0.06	2d8/15 L=25	1,1,1	
	s=1,m=3	12.5	1.17	4.0	4.0	0.0	0.27	0.01	0.13	0.08	2d8/15 L=25	1,1,1	
		25.1	1.17	4.0	4.0	0.0	0.27	0.07	0.18	0.11	2d8/15 L=25	1,1,1	
19	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.07	0.12	0.08	2d8/15 L=32	1,1,1	
	s=1,m=3	16.0	1.17	4.0	4.0	0.0	0.27	0.02	0.07	0.04	2d8/15 L=32	1,1,1	
		32.0	1.17	4.0	4.0	0.0	0.27	5.81e-03	0.01	6.61e-03	2d8/15 L=32	1,1,1	
8	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.81e-03	0.06	0.04	2d8/15 L=32	1,1,1	
	s=1,m=3	16.0	1.17	4.0	4.0	0.0	0.27	8.03e-03	3.14e-03	1.43e-03	2d8/15 L=32	1,1,1	
		32.0	1.17	4.0	4.0	0.0	0.27	3.76e-05	0.03	0.02	2d8/15 L=32	1,1,1	
							M_T= 2	Z=0.0	N=8	N=96			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
7	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.55e-04	5.64e-03	3.65e-03	2d8/15 L=27	1,1,1	

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	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	1.49e-03	2.22e-03	1.42e-03	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	2.74e-03	0.02	0.01	2d8/15 L=27	1,1,1	
59	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.74e-03	0.03	0.02	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	2.07e-03	9.24e-04	5.85e-04	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	3.47e-03	0.03	0.02	2d8/15 L=27	1,1,1	
58	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.47e-03	0.03	0.02	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	1.74e-03	6.70e-05	2.83e-05	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	3.52e-03	0.03	0.02	2d8/15 L=27	1,1,1	
60	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.52e-03	0.02	0.02	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	6.81e-04	2.82e-03	1.82e-03	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	5.73e-03	0.03	0.02	2d8/15 L=27	1,1,1	
6	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.07e-03	0.04	0.03	2d8/15 L=23	1,1,1	
	s=1,m=3	11.5	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=23	1,1,1	
		23.0	1.17	4.0	4.0	0.0	0.27	0.05	0.09	0.06	2d8/15 L=23	1,1,1	
5	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.05	0.12	0.07	2d8/15 L=27	1,1,1	
	s=1,m=3	13.5	1.17	4.0	4.0	0.0	0.27	9.57e-03	0.09	0.05	2d8/15 L=27	1,1,1	
		27.0	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=27	1,1,1	
50	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=27	1,1,1	
	s=1,m=3	13.5	1.17	4.0	4.0	0.0	0.27	0.03	0.04	0.02	2d8/15 L=27	1,1,1	
		27.0	1.17	4.0	4.0	0.0	0.27	0.04	7.42e-03	6.93e-04	2d8/15 L=27	1,1,1	
39	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.04	7.42e-03	6.93e-04	2d8/15 L=27	1,1,1	
	s=1,m=3	13.5	1.17	4.0	4.0	0.0	0.27	0.04	0.03	0.02	2d8/15 L=27	1,1,1	
		27.0	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=27	1,1,1	
41	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=27	1,1,1	
	s=1,m=3	13.5	1.17	4.0	4.0	0.0	0.27	7.82e-03	0.09	0.05	2d8/15 L=27	1,1,1	
		27.0	1.17	4.0	4.0	0.0	0.27	0.05	0.12	0.07	2d8/15 L=27	1,1,1	
4	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.05	0.09	0.05	2d8/15 L=44	1,1,1	
	s=1,m=3	22.1	1.17	4.0	4.0	0.0	0.27	0.01	0.04	0.02	2d8/15 L=44	1,1,1	
		44.1	1.17	4.0	4.0	0.0	0.27	5.74e-03	0.02	0.01	2d8/15 L=44	1,1,1	

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61	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.74e-03	0.05	0.03	2d8/15 L=44	1,1,1	
	s=1,m=3	22.1	1.17	4.0	4.0	0.0	0.27	7.89e-03	0.01	1.78e-03	2d8/15 L=44	1,1,1	
		44.1	1.17	4.0	4.0	0.0	0.27	9.27e-03	0.06	0.03	2d8/15 L=44	1,1,1	
3	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	8.91e-03	0.04	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	2.01e-03	5.71e-03	1.63e-03	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	6.44e-03	0.04	0.02	2d8/15 L=35	1,1,1	
65	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.44e-03	0.04	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.52e-03	3.56e-03	2.30e-04	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	6.82e-03	0.04	0.03	2d8/15 L=35	1,1,1	
63	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.82e-03	0.04	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.41e-03	3.21e-03	6.04e-06	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	6.86e-03	0.04	0.03	2d8/15 L=35	1,1,1	
67	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.86e-03	0.04	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.45e-03	3.24e-03	2.19e-05	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	6.93e-03	0.04	0.03	2d8/15 L=35	1,1,1	
62	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.93e-03	0.05	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.48e-03	3.23e-03	2.04e-05	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	6.99e-03	0.05	0.03	2d8/15 L=35	1,1,1	
66	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.99e-03	0.05	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.51e-03	3.24e-03	2.48e-05	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	7.06e-03	0.05	0.03	2d8/15 L=35	1,1,1	
64	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	7.06e-03	0.05	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.58e-03	3.26e-03	3.58e-05	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	7.04e-03	0.05	0.03	2d8/15 L=35	1,1,1	
68	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	7.04e-03	0.04	0.03	2d8/15 L=35	1,1,1	
	s=1,m=3	17.4	1.17	4.0	4.0	0.0	0.27	3.04e-03	4.47e-03	8.23e-04	2d8/15 L=35	1,1,1	
		34.8	1.17	4.0	4.0	0.0	0.27	8.33e-03	0.05	0.03	2d8/15 L=35	1,1,1	
99	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	9.47e-03	0.19	0.04	2d8/15 L=39	1,1,1	
	s=1,m=3	19.5	1.17	4.0	4.0	0.0	0.27	8.73e-03	0.14	5.74e-03	2d8/15 L=39	1,1,1	

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		39.0	1.17	4.0	4.0	0.0	0.27	1.31e-03	0.17	0.02	2d8/15 L=39	1,1,1	
							M_T= 3	Z=0.0	N=3	N=24			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
35	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	0.03	0.05	0.03	2d8/15 L=25	1,1,1	
	s=2,m=3	12.3	1.12	4.0	4.0	0.0	0.33	0.02	0.04	0.03	2d8/15 L=25	1,1,1	
		24.5	1.12	4.0	4.0	0.0	0.33	1.27e-03	0.04	0.03	2d8/15 L=25	1,1,1	
82	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	1.27e-03	0.01	4.90e-03	2d8/15 L=24	1,1,1	
	s=2,m=3	12.3	1.12	4.0	4.0	0.0	0.33	1.93e-03	0.02	6.65e-03	2d8/15 L=24	1,1,1	
		24.5	1.12	4.0	4.0	0.0	0.33	6.11e-03	0.02	8.40e-03	2d8/15 L=24	1,1,1	
17	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.51e-03	0.01	8.91e-03	2d8/15 L=24	1,1,1	
	s=1,m=3	11.8	1.17	4.0	4.0	0.0	0.27	4.92e-04	0.01	6.61e-03	2d8/15 L=24	1,1,1	
		23.5	1.17	4.0	4.0	0.0	0.27	2.34e-03	6.88e-03	4.34e-03	2d8/15 L=24	1,1,1	
54	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.34e-03	6.88e-03	4.34e-03	2d8/15 L=23	1,1,1	
	s=1,m=3	11.8	1.17	4.0	4.0	0.0	0.27	4.00e-03	3.41e-03	2.08e-03	2d8/15 L=23	1,1,1	
		23.5	1.17	4.0	4.0	0.0	0.27	4.50e-03	4.35e-04	1.47e-04	2d8/15 L=23	1,1,1	
43	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.50e-03	4.35e-04	1.47e-04	2d8/15 L=24	1,1,1	
	s=1,m=3	11.8	1.17	4.0	4.0	0.0	0.27	3.85e-03	3.83e-03	2.35e-03	2d8/15 L=24	1,1,1	
		23.5	1.17	4.0	4.0	0.0	0.27	2.07e-03	7.19e-03	4.53e-03	2d8/15 L=24	1,1,1	
52	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.07e-03	7.19e-03	4.53e-03	2d8/15 L=23	1,1,1	
	s=1,m=3	11.8	1.17	4.0	4.0	0.0	0.27	8.38e-04	0.01	6.69e-03	2d8/15 L=23	1,1,1	
		23.5	1.17	4.0	4.0	0.0	0.27	4.86e-03	0.01	8.83e-03	2d8/15 L=23	1,1,1	
18	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.86e-03	9.01e-03	5.70e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	3.49e-04	2.42e-03	1.42e-03	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.21e-03	4.50e-03	2.77e-03	2d8/15 L=58	1,1,1	
12	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.21e-03	6.18e-03	3.86e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	1.08e-03	6.00e-04	2.36e-04	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.77e-03	6.77e-03	4.24e-03	2d8/15 L=58	1,1,1	
10	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.77e-03	6.36e-03	3.98e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	7.80e-04	3.28e-04	5.94e-05	2d8/15 L=58	1,1,1	

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		57.6	1.17	4.0	4.0	0.0	0.27	1.59e-03	6.04e-03	3.77e-03	2d8/15 L=58	1,1,1	
14	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.59e-03	6.01e-03	3.75e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	7.90e-04	2.48e-04	7.51e-06	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.53e-03	5.85e-03	3.65e-03	2d8/15 L=58	1,1,1	
9	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.53e-03	5.74e-03	3.58e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	7.40e-04	2.56e-04	1.26e-05	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.46e-03	5.57e-03	3.46e-03	2d8/15 L=58	1,1,1	
13	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.46e-03	5.51e-03	3.42e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	7.26e-04	2.91e-04	3.52e-05	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.33e-03	5.26e-03	3.26e-03	2d8/15 L=58	1,1,1	
11	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.33e-03	5.02e-03	3.11e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	5.67e-04	3.92e-04	1.01e-04	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.55e-03	5.20e-03	3.22e-03	2d8/15 L=58	1,1,1	
15	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.55e-03	5.92e-03	3.69e-03	2d8/15 L=58	1,1,1	
	s=1,m=3	28.8	1.17	4.0	4.0	0.0	0.27	1.21e-03	1.26e-03	6.63e-04	2d8/15 L=58	1,1,1	
		57.6	1.17	4.0	4.0	0.0	0.27	1.72e-04	3.74e-03	2.28e-03	2d8/15 L=58	1,1,1	
							M_T= 4	Z=0.0	N=34	N=38			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
71	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.07e-03	0.05	0.04	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	5.88e-03	7.39e-03	4.68e-03	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	3.94e-04	0.04	0.02	2d8/15 L=27	1,1,1	
69	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.11e-03	0.05	0.03	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	3.06e-03	2.21e-04	2.54e-05	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	6.07e-03	0.05	0.03	2d8/15 L=27	1,1,1	
70	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.12e-03	0.05	0.03	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	3.06e-03	1.87e-04	3.41e-06	2d8/15 L=27	1,1,1	
		27.3	1.17	4.0	4.0	0.0	0.27	6.11e-03	0.05	0.03	2d8/15 L=27	1,1,1	
29	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.72e-03	0.05	0.03	2d8/15 L=27	1,1,1	
	s=1,m=3	13.6	1.17	4.0	4.0	0.0	0.27	3.27e-03	6.97e-04	3.34e-04	2d8/15 L=27	1,1,1	

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		27.3	1.17	4.0	4.0	0.0	0.27	6.12e-03	0.05	0.03	2d8/15 L=27	1,1,1	
30	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.36e-03	0.01	3.76e-03	2d8/15 L=31	1,1,1	
	s=1,m=3	15.5	1.17	4.0	4.0	0.0	0.27	0.01	0.05	0.03	2d8/15 L=31	1,1,1	
		31.0	1.17	4.0	4.0	0.0	0.27	0.05	0.11	0.07	2d8/15 L=31	1,1,1	
31	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.05	0.15	0.10	2d8/15 L=22	1,1,1	
	s=1,m=3	10.8	1.17	4.0	4.0	0.0	0.27	7.67e-03	0.11	0.07	2d8/15 L=22	1,1,1	
		21.5	1.17	4.0	4.0	0.0	0.27	0.02	0.08	0.05	2d8/15 L=22	1,1,1	
16	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.02	0.08	0.05	2d8/15 L=22	1,1,1	
	s=1,m=3	10.8	1.17	4.0	4.0	0.0	0.27	0.04	0.04	0.02	2d8/15 L=22	1,1,1	
		21.5	1.17	4.0	4.0	0.0	0.27	0.04	2.72e-03	1.64e-05	2d8/15 L=22	1,1,1	
48	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.04	2.72e-03	1.64e-05	2d8/15 L=21	1,1,1	
	s=1,m=3	10.8	1.17	4.0	4.0	0.0	0.27	0.04	0.04	0.02	2d8/15 L=21	1,1,1	
		21.5	1.17	4.0	4.0	0.0	0.27	0.02	0.08	0.05	2d8/15 L=21	1,1,1	
51	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.02	0.08	0.05	2d8/15 L=22	1,1,1	
	s=1,m=3	10.8	1.17	4.0	4.0	0.0	0.27	7.73e-03	0.11	0.07	2d8/15 L=22	1,1,1	
		21.5	1.17	4.0	4.0	0.0	0.27	0.05	0.15	0.10	2d8/15 L=22	1,1,1	
32	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.05	0.10	0.07	2d8/15 L=25	1,1,1	
	s=1,m=3	12.8	1.17	4.0	4.0	0.0	0.27	0.02	0.06	0.04	2d8/15 L=25	1,1,1	
		25.5	1.17	4.0	4.0	0.0	0.27	6.54e-03	0.01	8.08e-03	2d8/15 L=25	1,1,1	
73	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.54e-03	0.05	0.03	2d8/15 L=25	1,1,1	
	s=1,m=3	12.8	1.17	4.0	4.0	0.0	0.27	2.10e-03	2.66e-03	9.94e-04	2d8/15 L=25	1,1,1	
		25.5	1.17	4.0	4.0	0.0	0.27	5.43e-03	0.04	0.03	2d8/15 L=25	1,1,1	
72	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.43e-03	0.05	0.03	2d8/15 L=25	1,1,1	
	s=1,m=3	12.8	1.17	4.0	4.0	0.0	0.27	2.69e-03	1.18e-03	3.19e-05	2d8/15 L=25	1,1,1	
		25.5	1.17	4.0	4.0	0.0	0.27	5.40e-03	0.05	0.03	2d8/15 L=25	1,1,1	
74	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.40e-03	0.05	0.03	2d8/15 L=26	1,1,1	
	s=1,m=3	12.8	1.17	4.0	4.0	0.0	0.27	2.90e-03	1.64e-03	3.31e-04	2d8/15 L=26	1,1,1	
		25.5	1.17	4.0	4.0	0.0	0.27	5.00e-03	0.04	0.03	2d8/15 L=26	1,1,1	
							M_T= 5	Z=0.0	N=29	N=108			

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Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
102	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	6.91e-05	0.02	0.01	2d8/15 L=22	1,1,1	
	s=1,m=3	11.0	1.17	4.0	4.0	0.0	0.27	6.46e-03	0.02	0.01	2d8/15 L=22	1,1,1	
		22.0	1.17	4.0	4.0	0.0	0.27	0.01	0.03	0.02	2d8/15 L=22	1,1,1	
101	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.02	0.02	2d8/15 L=39	1,1,1	
	s=1,m=3	19.3	1.17	4.0	4.0	0.0	0.27	2.92e-03	0.02	0.01	2d8/15 L=39	1,1,1	
		38.5	1.17	4.0	4.0	0.0	0.27	4.66e-03	0.01	6.96e-03	2d8/15 L=39	1,1,1	
131	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.66e-03	0.01	6.96e-03	2d8/15 L=39	1,1,1	
	s=1,m=3	19.3	1.17	4.0	4.0	0.0	0.27	8.95e-03	4.94e-03	3.18e-03	2d8/15 L=39	1,1,1	
		38.5	1.17	4.0	4.0	0.0	0.27	0.01	6.80e-04	4.10e-04	2d8/15 L=39	1,1,1	
127	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	6.80e-04	4.10e-04	2d8/15 L=39	1,1,1	
	s=1,m=3	19.3	1.17	4.0	4.0	0.0	0.27	8.31e-03	5.90e-03	3.80e-03	2d8/15 L=39	1,1,1	
		38.5	1.17	4.0	4.0	0.0	0.27	3.71e-03	0.01	7.00e-03	2d8/15 L=39	1,1,1	
135	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.71e-03	0.01	7.00e-03	2d8/15 L=38	1,1,1	
	s=1,m=3	19.3	1.17	4.0	4.0	0.0	0.27	3.51e-03	0.02	9.99e-03	2d8/15 L=38	1,1,1	
		38.5	1.17	4.0	4.0	0.0	0.27	0.01	0.02	0.01	2d8/15 L=38	1,1,1	
100	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.02	0.01	2d8/15 L=19	1,1,1	
	s=1,m=3	9.4	1.17	4.0	4.0	0.0	0.27	7.34e-03	0.02	0.01	2d8/15 L=19	1,1,1	
		18.8	1.17	4.0	4.0	0.0	0.27	2.03e-03	0.02	0.01	2d8/15 L=19	1,1,1	
120	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.03e-03	4.87e-03	3.13e-03	2d8/15 L=19	1,1,1	
	s=1,m=3	9.4	1.17	4.0	4.0	0.0	0.27	9.82e-04	3.02e-03	1.93e-03	2d8/15 L=19	1,1,1	
		18.8	1.17	4.0	4.0	0.0	0.27	4.23e-04	1.25e-03	7.80e-04	2d8/15 L=19	1,1,1	
119	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	4.23e-04	2.16e-03	1.37e-03	2d8/15 L=19	1,1,1	
	s=1,m=3	9.4	1.17	4.0	4.0	0.0	0.27	8.57e-05	4.59e-04	2.67e-04	2d8/15 L=19	1,1,1	
		18.8	1.17	4.0	4.0	0.0	0.27	1.96e-04	1.27e-03	7.92e-04	2d8/15 L=19	1,1,1	
121	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.96e-04	1.51e-03	9.49e-04	2d8/15 L=19	1,1,1	
	s=1,m=3	9.4	1.17	4.0	4.0	0.0	0.27	1.44e-05	1.45e-04	6.35e-05	2d8/15 L=19	1,1,1	
		18.8	1.17	4.0	4.0	0.0	0.27	2.42e-04	1.63e-03	1.03e-03	2d8/15 L=19	1,1,1	
38	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	3.50e-04	4.36e-03	1.96e-03	2d8/15 L=29	1,1,1	

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	s=2,m=3	14.3	1.12	4.0	4.0	0.0	0.33	2.58e-04	2.18e-03	7.43e-05	2d8/15 L=29	1,1,1	
		28.5	1.12	4.0	4.0	0.0	0.33	4.46e-04	4.53e-03	2.11e-03	2d8/15 L=29	1,1,1	
122	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	4.46e-04	4.47e-03	2.05e-03	2d8/15 L=28	1,1,1	
	s=2,m=3	14.3	1.12	4.0	4.0	0.0	0.33	2.22e-04	2.12e-03	1.74e-05	2d8/15 L=28	1,1,1	
		28.5	1.12	4.0	4.0	0.0	0.33	4.24e-04	4.43e-03	2.02e-03	2d8/15 L=28	1,1,1	
28	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.79e-04	1.17e-03	5.68e-04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.5	1.17	4.0	4.0	0.0	0.27	5.20e-04	2.13e-03	1.19e-03	2d8/15 L=35	1,1,1	
		35.0	1.17	4.0	4.0	0.0	0.27	2.12e-03	4.84e-03	2.95e-03	2d8/15 L=35	1,1,1	
27	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.12e-03	6.78e-03	4.33e-03	2d8/15 L=21	1,1,1	
	s=1,m=3	10.7	1.17	4.0	4.0	0.0	0.27	3.29e-04	5.12e-03	3.25e-03	2d8/15 L=21	1,1,1	
		21.4	1.17	4.0	4.0	0.0	0.27	9.52e-04	3.46e-03	2.17e-03	2d8/15 L=21	1,1,1	
85	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	9.52e-04	3.46e-03	2.17e-03	2d8/15 L=21	1,1,1	
	s=1,m=3	10.7	1.17	4.0	4.0	0.0	0.27	1.72e-03	1.80e-03	1.09e-03	2d8/15 L=21	1,1,1	
		21.4	1.17	4.0	4.0	0.0	0.27	1.99e-03	1.41e-04	1.61e-05	2d8/15 L=21	1,1,1	
47	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.99e-03	1.41e-04	1.61e-05	2d8/15 L=21	1,1,1	
	s=1,m=3	10.7	1.17	4.0	4.0	0.0	0.27	1.73e-03	1.81e-03	1.10e-03	2d8/15 L=21	1,1,1	
		21.4	1.17	4.0	4.0	0.0	0.27	9.49e-04	3.53e-03	2.21e-03	2d8/15 L=21	1,1,1	
49	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	9.49e-04	3.53e-03	2.21e-03	2d8/15 L=21	1,1,1	
	s=1,m=3	10.7	1.17	4.0	4.0	0.0	0.27	3.62e-04	5.25e-03	3.34e-03	2d8/15 L=21	1,1,1	
		21.4	1.17	4.0	4.0	0.0	0.27	2.20e-03	6.99e-03	4.46e-03	2d8/15 L=21	1,1,1	
26	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.20e-03	4.85e-03	3.00e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	5.09e-04	1.95e-03	1.12e-03	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	3.64e-04	1.41e-03	7.65e-04	2d8/15 L=37	1,1,1	
89	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.64e-04	2.98e-03	1.78e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	3.28e-04	3.90e-04	1.01e-04	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	5.31e-04	3.29e-03	1.98e-03	2d8/15 L=37	1,1,1	
87	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.31e-04	3.15e-03	1.89e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	2.52e-04	2.49e-04	9.12e-06	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	5.16e-04	3.12e-03	1.87e-03	2d8/15 L=37	1,1,1	

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91	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.16e-04	3.13e-03	1.88e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	2.59e-04	2.36e-04	0.0	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	5.17e-04	3.13e-03	1.88e-03	2d8/15 L=37	1,1,1	
86	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.17e-04	3.13e-03	1.88e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	2.59e-04	2.36e-04	0.0	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	5.16e-04	3.13e-03	1.88e-03	2d8/15 L=37	1,1,1	
90	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.16e-04	3.12e-03	1.87e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	2.52e-04	2.49e-04	9.08e-06	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	5.31e-04	3.14e-03	1.89e-03	2d8/15 L=37	1,1,1	
88	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.31e-04	3.29e-03	1.98e-03	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	3.28e-04	3.90e-04	1.01e-04	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	3.65e-04	2.98e-03	1.78e-03	2d8/15 L=37	1,1,1	
92	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.65e-04	1.41e-03	7.66e-04	2d8/15 L=37	1,1,1	
	s=1,m=3	18.7	1.17	4.0	4.0	0.0	0.27	5.08e-04	1.95e-03	1.11e-03	2d8/15 L=37	1,1,1	
		37.4	1.17	4.0	4.0	0.0	0.27	2.20e-03	4.85e-03	3.00e-03	2d8/15 L=37	1,1,1	
25	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.20e-03	7.53e-03	4.82e-03	2d8/15 L=21	1,1,1	
	s=1,m=3	10.3	1.17	4.0	4.0	0.0	0.27	2.99e-04	5.66e-03	3.61e-03	2d8/15 L=21	1,1,1	
		20.5	1.17	4.0	4.0	0.0	0.27	1.06e-03	3.78e-03	2.39e-03	2d8/15 L=21	1,1,1	
57	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.06e-03	3.78e-03	2.39e-03	2d8/15 L=20	1,1,1	
	s=1,m=3	10.3	1.17	4.0	4.0	0.0	0.27	1.86e-03	1.90e-03	1.16e-03	2d8/15 L=20	1,1,1	
		20.5	1.17	4.0	4.0	0.0	0.27	2.11e-03	2.06e-04	6.47e-05	2d8/15 L=20	1,1,1	
46	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.11e-03	2.06e-04	6.47e-05	2d8/15 L=20	1,1,1	
	s=1,m=3	10.3	1.17	4.0	4.0	0.0	0.27	1.80e-03	2.10e-03	1.30e-03	2d8/15 L=20	1,1,1	
		20.5	1.17	4.0	4.0	0.0	0.27	9.34e-04	4.01e-03	2.54e-03	2d8/15 L=20	1,1,1	
42	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	9.34e-04	4.01e-03	2.54e-03	2d8/15 L=20	1,1,1	
	s=1,m=3	10.3	1.17	4.0	4.0	0.0	0.27	4.44e-04	5.60e-03	3.57e-03	2d8/15 L=20	1,1,1	
		20.5	1.17	4.0	4.0	0.0	0.27	2.29e-03	7.18e-03	4.60e-03	2d8/15 L=20	1,1,1	
24	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	2.29e-03	5.21e-03	3.21e-03	2d8/15 L=41	1,1,1	
	s=1,m=3	20.5	1.17	4.0	4.0	0.0	0.27	3.14e-04	2.03e-03	1.15e-03	2d8/15 L=41	1,1,1	

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		41.0	1.17	4.0	4.0	0.0	0.27	2.03e-04	1.66e-03	9.09e-04	2d8/15 L=41	1,1,1	
							M_T= 6	Z=0.0	N=4	N=39			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
36	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	3.16e-03	7.00e-03	4.52e-03	2d8/15 L=22	1,1,1	
	s=2,m=3	10.8	1.12	4.0	4.0	0.0	0.33	1.33e-03	5.23e-03	2.98e-03	2d8/15 L=22	1,1,1	
		21.5	1.12	4.0	4.0	0.0	0.33	2.53e-04	3.46e-03	1.45e-03	2d8/15 L=22	1,1,1	
83	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	2.53e-04	3.81e-03	1.75e-03	2d8/15 L=21	1,1,1	
	s=2,m=3	10.8	1.12	4.0	4.0	0.0	0.33	2.25e-04	2.03e-03	2.14e-04	2d8/15 L=21	1,1,1	
		21.5	1.12	4.0	4.0	0.0	0.33	4.43e-05	3.31e-03	1.32e-03	2d8/15 L=21	1,1,1	
33	ok,ok	0.0	1.16	4.0	4.0	0.0	0.27	5.90e-04	0.03	9.75e-04	2d8/15 L=6	1,1,1	
	s=1,m=3	3.0	1.16	4.0	4.0	0.0	0.27	7.00e-04	0.03	6.67e-04	2d8/15 L=6	1,1,1	
		6.1	1.16	4.0	4.0	0.0	0.27	7.68e-04	0.03	3.53e-04	2d8/15 L=6	1,1,1	
							M_T= 7	Z=0.0	N=22	N=39			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
34	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.40e-03	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	7.30e-03	5.66e-03	3.31e-03	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.07	0.04	2d8/15 L=35	1,1,1	
78	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	4.94e-03	9.20e-04	2.37e-04	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
76	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	5.12e-03	5.91e-04	2.29e-05	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
80	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	5.12e-03	5.61e-04	3.81e-06	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
75	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	5.13e-03	5.68e-04	7.93e-06	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	

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79	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	5.16e-03	5.97e-04	2.70e-05	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
77	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	4.85e-03	1.25e-03	4.49e-04	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	0.01	0.06	0.04	2d8/15 L=35	1,1,1	
81	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.07	0.05	2d8/15 L=35	1,1,1	
	s=1,m=3	17.6	1.17	4.0	4.0	0.0	0.27	9.19e-03	9.85e-03	6.03e-03	2d8/15 L=35	1,1,1	
		35.1	1.17	4.0	4.0	0.0	0.27	1.61e-03	0.05	0.03	2d8/15 L=35	1,1,1	
							M_T= 8	Z=0.0	N=7	N=35			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
37	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	1.72e-04	3.57e-03	1.70e-03	2d8/15 L=25	1,1,1	
	s=2,m=3	12.8	1.12	4.0	4.0	0.0	0.33	2.82e-04	1.76e-03	1.24e-04	2d8/15 L=25	1,1,1	
		25.5	1.12	4.0	4.0	0.0	0.33	3.16e-04	3.86e-03	1.94e-03	2d8/15 L=25	1,1,1	
84	ok,ok	0.0	1.12	4.0	4.0	0.0	0.33	3.16e-04	2.84e-03	1.06e-03	2d8/15 L=26	1,1,1	
	s=2,m=3	12.8	1.12	4.0	4.0	0.0	0.33	2.29e-04	2.49e-03	7.60e-04	2d8/15 L=26	1,1,1	
		25.5	1.12	4.0	4.0	0.0	0.33	1.19e-03	4.59e-03	2.58e-03	2d8/15 L=26	1,1,1	
							M_T= 9	Z=0.0	N=105	N=108			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
105	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	1.04e-06	0.03	0.02	2d8/15 L=31	1,1,1	
	s=1,m=3	15.7	1.17	4.0	4.0	0.0	0.27	5.17e-03	6.35e-03	3.96e-03	2d8/15 L=31	1,1,1	
		31.3	1.17	4.0	4.0	0.0	0.27	5.44e-03	0.04	0.03	2d8/15 L=31	1,1,1	
114	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.44e-03	0.04	0.02	2d8/15 L=31	1,1,1	
	s=1,m=3	15.7	1.17	4.0	4.0	0.0	0.27	2.48e-03	4.62e-04	1.41e-04	2d8/15 L=31	1,1,1	
		31.3	1.17	4.0	4.0	0.0	0.27	5.22e-03	0.03	0.02	2d8/15 L=31	1,1,1	
113	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	5.22e-03	0.04	0.02	2d8/15 L=31	1,1,1	
	s=1,m=3	15.7	1.17	4.0	4.0	0.0	0.27	3.18e-03	1.72e-03	9.61e-04	2d8/15 L=31	1,1,1	
		31.3	1.17	4.0	4.0	0.0	0.27	3.87e-03	0.03	0.02	2d8/15 L=31	1,1,1	
115	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.87e-03	2.30e-03	1.33e-03	2d8/15 L=31	1,1,1	

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	s=1,m=3	15.7	1.17	4.0	4.0	0.0	0.27	0.01	0.04	0.02	2d8/15 L=31	1,1,1	
		31.3	1.17	4.0	4.0	0.0	0.27	0.04	0.07	0.05	2d8/15 L=31	1,1,1	
104	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.04	0.10	0.07	2d8/15 L=24	1,1,1	
	s=1,m=3	11.9	1.17	4.0	4.0	0.0	0.27	6.14e-03	0.08	0.05	2d8/15 L=24	1,1,1	
		23.8	1.17	4.0	4.0	0.0	0.27	0.02	0.05	0.03	2d8/15 L=24	1,1,1	
132	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.02	0.05	0.03	2d8/15 L=24	1,1,1	
	s=1,m=3	11.9	1.17	4.0	4.0	0.0	0.27	0.03	0.03	0.02	2d8/15 L=24	1,1,1	
		23.8	1.17	4.0	4.0	0.0	0.27	0.03	6.50e-04	2.63e-04	2d8/15 L=24	1,1,1	
128	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.03	6.50e-04	2.63e-04	2d8/15 L=24	1,1,1	
	s=1,m=3	11.9	1.17	4.0	4.0	0.0	0.27	0.03	0.03	0.02	2d8/15 L=24	1,1,1	
		23.8	1.17	4.0	4.0	0.0	0.27	0.01	0.05	0.03	2d8/15 L=24	1,1,1	
136	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.01	0.05	0.03	2d8/15 L=24	1,1,1	
	s=1,m=3	11.9	1.17	4.0	4.0	0.0	0.27	6.45e-03	0.08	0.05	2d8/15 L=24	1,1,1	
		23.8	1.17	4.0	4.0	0.0	0.27	0.04	0.10	0.06	2d8/15 L=24	1,1,1	
103	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	0.04	0.07	0.05	2d8/15 L=26	1,1,1	
	s=1,m=3	13.0	1.17	4.0	4.0	0.0	0.27	0.02	0.04	0.03	2d8/15 L=26	1,1,1	
		25.9	1.17	4.0	4.0	0.0	0.27	3.95e-03	0.02	0.01	2d8/15 L=26	1,1,1	
117	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.95e-03	0.03	0.02	2d8/15 L=26	1,1,1	
	s=1,m=3	13.0	1.17	4.0	4.0	0.0	0.27	1.26e-03	1.20e-03	6.18e-04	2d8/15 L=26	1,1,1	
		25.9	1.17	4.0	4.0	0.0	0.27	3.22e-03	0.03	0.02	2d8/15 L=26	1,1,1	
116	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.22e-03	0.03	0.02	2d8/15 L=26	1,1,1	
	s=1,m=3	13.0	1.17	4.0	4.0	0.0	0.27	1.62e-03	3.71e-04	8.22e-05	2d8/15 L=26	1,1,1	
		25.9	1.17	4.0	4.0	0.0	0.27	3.11e-03	0.03	0.02	2d8/15 L=26	1,1,1	
118	ok,ok	0.0	1.17	4.0	4.0	0.0	0.27	3.11e-03	0.03	0.02	2d8/15 L=26	1,1,1	
	s=1,m=3	13.0	1.17	4.0	4.0	0.0	0.27	3.03e-03	4.04e-03	2.46e-03	2d8/15 L=26	1,1,1	
		25.9	1.17	4.0	4.0	0.0	0.27	2.70e-04	0.02	0.01	2d8/15 L=26	1,1,1	
							M_T= 10	Z=0.0	N=96	N=97			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
106	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	0.04	0.03	0.02	2d8/15 L=75	1,1,1	

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	s=5,m=3	37.7	0.87	4.0	4.0	0.0	0.25	4.19e-03	0.02	0.02	2d8/15 L=75	1,1,1	
		75.4	0.87	4.0	4.0	0.0	0.25	0.02	0.02	0.01	2d8/15 L=75	1,1,1	
133	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	0.02	0.02	0.01	2d8/15 L=75	1,1,1	
	s=5,m=3	37.7	0.87	4.0	4.0	0.0	0.25	0.04	9.37e-03	7.81e-03	2d8/15 L=75	1,1,1	
		75.4	0.87	4.0	4.0	0.0	0.25	0.05	2.47e-03	1.83e-03	2d8/15 L=75	1,1,1	
129	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	0.05	2.47e-03	1.83e-03	2d8/15 L=75	1,1,1	
	s=5,m=3	37.7	0.87	4.0	4.0	0.0	0.25	0.05	5.44e-03	4.41e-03	2d8/15 L=75	1,1,1	
		75.4	0.87	4.0	4.0	0.0	0.25	0.03	0.01	0.01	2d8/15 L=75	1,1,1	
137	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	0.03	0.01	0.01	2d8/15 L=75	1,1,1	
	s=5,m=3	37.7	0.87	4.0	4.0	0.0	0.25	9.75e-03	0.02	0.02	2d8/15 L=75	1,1,1	
		75.4	0.87	4.0	4.0	0.0	0.25	0.02	0.03	0.02	2d8/15 L=75	1,1,1	
							M_T= 11	Z=0.0	N=97	N=98			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
107	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	1.35e-03	0.13	3.38e-03	2d8/15 L=17	1,1,1	
	s=5,m=3	8.5	0.87	4.0	4.0	0.0	0.25	1.87e-03	0.13	1.88e-03	2d8/15 L=17	1,1,1	
		17.1	0.87	4.0	4.0	0.0	0.25	9.51e-04	0.14	0.01	2d8/15 L=17	1,1,1	
123	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	9.51e-04	0.14	0.01	2d8/15 L=17	1,1,1	
	s=5,m=3	8.5	0.87	4.0	4.0	0.0	0.25	1.33e-03	0.13	2.42e-04	2d8/15 L=17	1,1,1	
		17.1	0.87	4.0	4.0	0.0	0.25	1.12e-03	0.14	0.01	2d8/15 L=17	1,1,1	
							M_T= 12	Z=0.0	N=98	N=99			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
108	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	4.77e-04	8.67e-03	7.47e-03	2d8/15 L=17	1,1,1	
	s=5,m=3	8.6	0.87	4.0	4.0	0.0	0.25	3.48e-03	0.01	8.87e-03	2d8/15 L=17	1,1,1	
		17.1	0.87	4.0	4.0	0.0	0.25	7.00e-03	0.01	0.01	2d8/15 L=17	1,1,1	
124	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	7.00e-03	0.05	0.04	2d8/15 L=17	1,1,1	
	s=5,m=3	8.6	0.87	4.0	4.0	0.0	0.25	0.02	0.05	0.04	2d8/15 L=17	1,1,1	
		17.1	0.87	4.0	4.0	0.0	0.25	0.04	0.05	0.05	2d8/15 L=17	1,1,1	
							M_T= 13	Z=0.0	N=99	N=104			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	

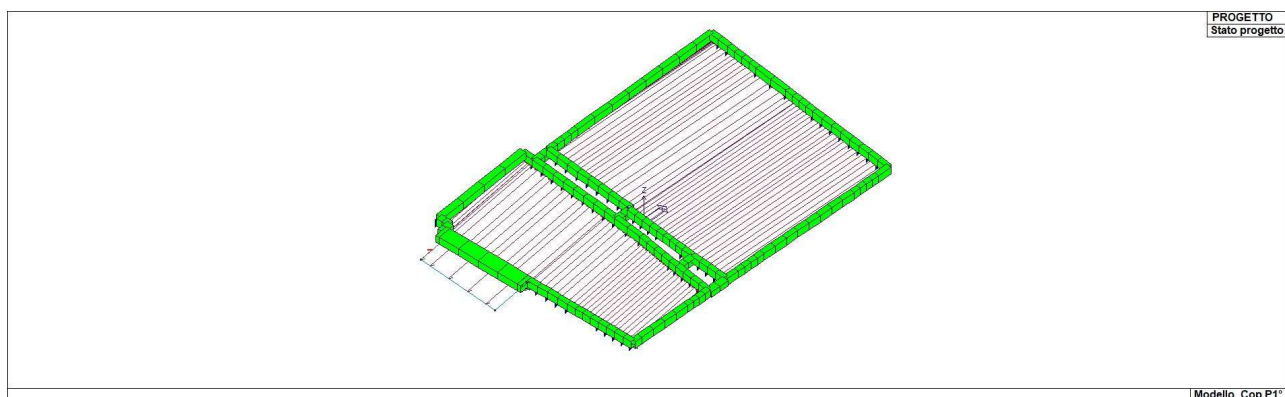
RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

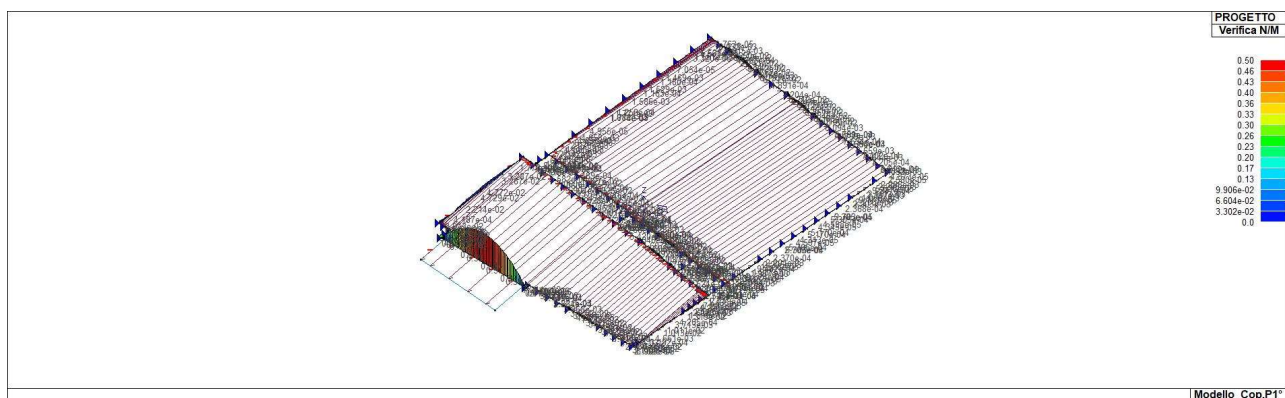
109	ok,ok	0.0	0.87	8.0	8.0	0.0	0.25	0.0	0.20	0.17	4d8/15 L=9	0,1,1	
	s=3,m=3	4.5	0.87	8.0	8.0	0.0	0.25	0.03	0.19	0.17	4d8/15 L=9	1,1,1	
		9.1	0.87	8.0	8.0	0.0	0.25	0.06	0.19	0.16	4d8/15 L=9	1,1,1	
110	ok,ok	0.0	0.87	8.0	8.0	0.0	0.25	0.06	0.19	0.16	4d8/15 L=61	1,1,1	
	s=3,m=3	30.7	0.87	8.0	8.0	0.0	0.25	0.25	0.14	0.12	4d8/15 L=61	1,1,1	
		61.4	0.87	8.0	8.0	0.0	0.25	0.39	0.09	0.08	4d8/15 L=61	1,1,1	
134	ok,ok	0.0	0.87	8.0	8.0	0.0	0.25	0.39	0.09	0.08	4d8/15 L=61	1,1,1	
	s=3,m=3	30.7	0.87	8.0	8.0	0.0	0.25	0.47	0.05	0.04	4d8/15 L=61	1,1,1	
		61.4	0.87	8.0	8.0	0.0	0.25	0.50	1.37e-03	9.90e-04	4d8/15 L=61	1,1,1	
130	ok,ok	0.0	0.87	8.0	8.0	0.0	0.25	0.50	1.37e-03	9.90e-04	4d8/15 L=61	1,1,1	
	s=3,m=3	30.7	0.87	8.0	8.0	0.0	0.25	0.47	0.05	0.04	4d8/15 L=61	1,1,1	
		61.4	0.87	8.0	8.0	0.0	0.25	0.39	0.09	0.08	4d8/15 L=61	1,1,1	
138	ok,ok	0.0	0.87	8.0	8.0	0.0	0.25	0.39	0.09	0.08	4d8/15 L=61	1,1,1	
	s=3,m=3	30.7	0.87	8.0	8.0	0.0	0.25	0.26	0.14	0.12	4d8/15 L=61	1,1,1	
		61.4	0.87	8.0	8.0	0.0	0.25	0.07	0.19	0.16	4d8/15 L=61	1,1,1	
111	ok,ok	0.0	0.87	8.0	8.0	0.0	0.25	0.07	0.19	0.16	4d8/15 L=10	1,1,1	
	s=3,m=3	5.1	0.87	8.0	8.0	0.0	0.25	0.04	0.19	0.17	4d8/15 L=10	1,1,1	
		10.2	0.87	8.0	8.0	0.0	0.25	0.0	0.20	0.17	4d8/15 L=10	0,1,1	
							M_T= 14	Z=0.0	N=104	N=105			
Trave	Note	Pos.	%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc	Staffe	Rif. cmb	
112	ok,ok	0.0	0.87	4.0	4.0	0.0	0.25	1.94e-04	4.03e-04	3.47e-04	2d8/15 L=12	1,1,1	
	s=5,m=3	6.0	0.87	4.0	4.0	0.0	0.25	1.79e-04	5.34e-04	4.60e-04	2d8/15 L=12	1,1,1	
		12.0	0.87	4.0	4.0	0.0	0.25	4.41e-05	1.47e-03	1.27e-03	2d8/15 L=12	1,1,1	
Trave			%Af	Af inf.	Af. sup	Af long.	x/d	V N/M	V V/T cls	V V/T acc			
			1.17	8.04	8.04	0.0	0.33	0.50	0.20	0.17			

RELAZIONE DI CALCOLO

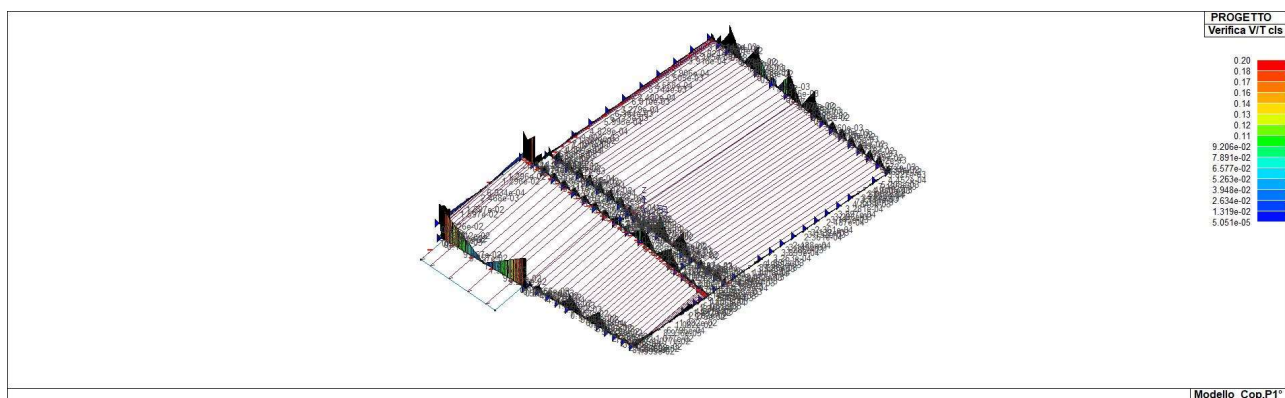
INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



71_CA_TRV_01_Stato progetto



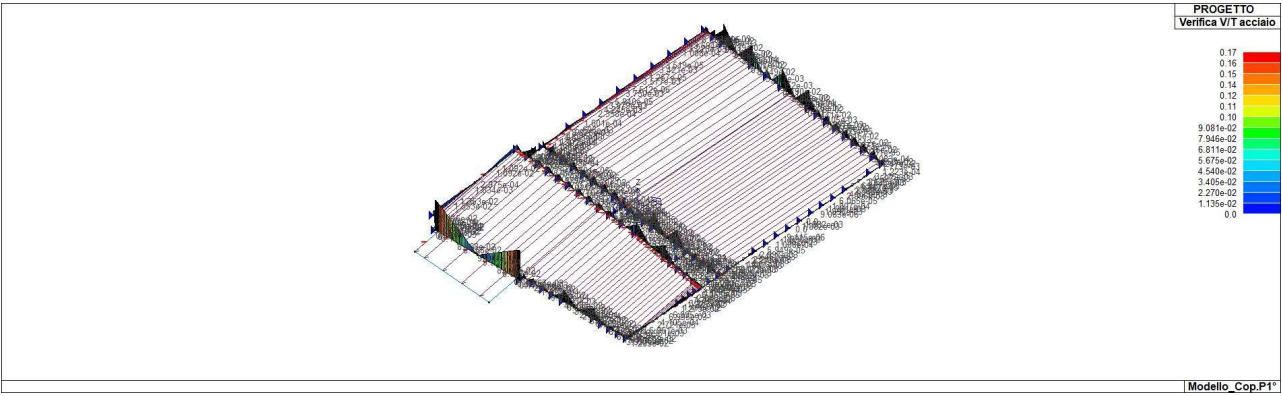
71_CA_TRV_09_Verifica NM



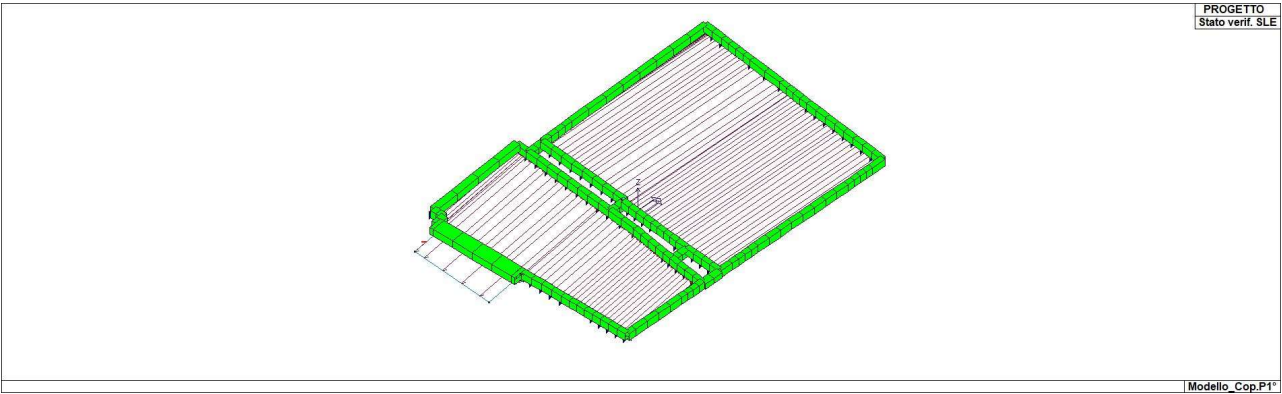
71_CA_TRV_11_Verifica VT cls

RELAZIONE DI CALCOLO

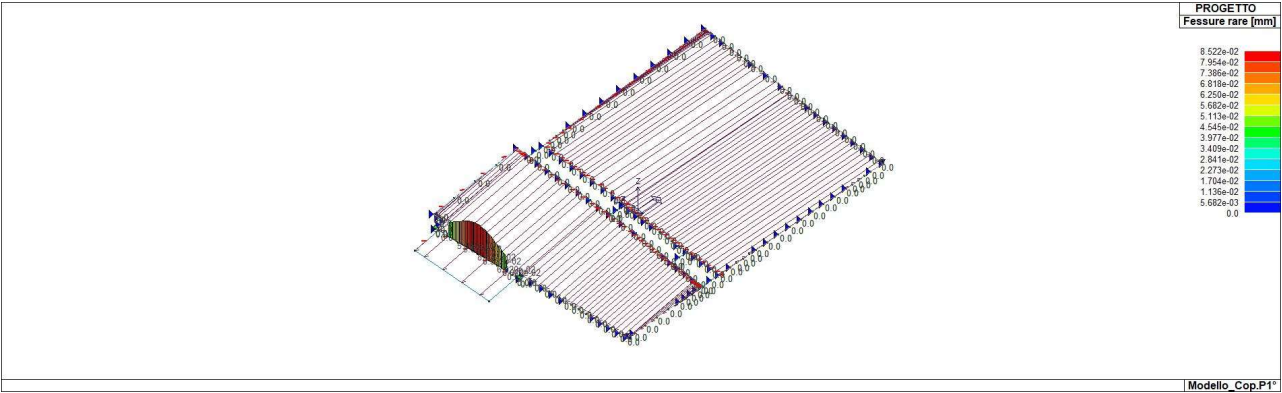
INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



71_CA_TRV_12_Verifica VT acciaio

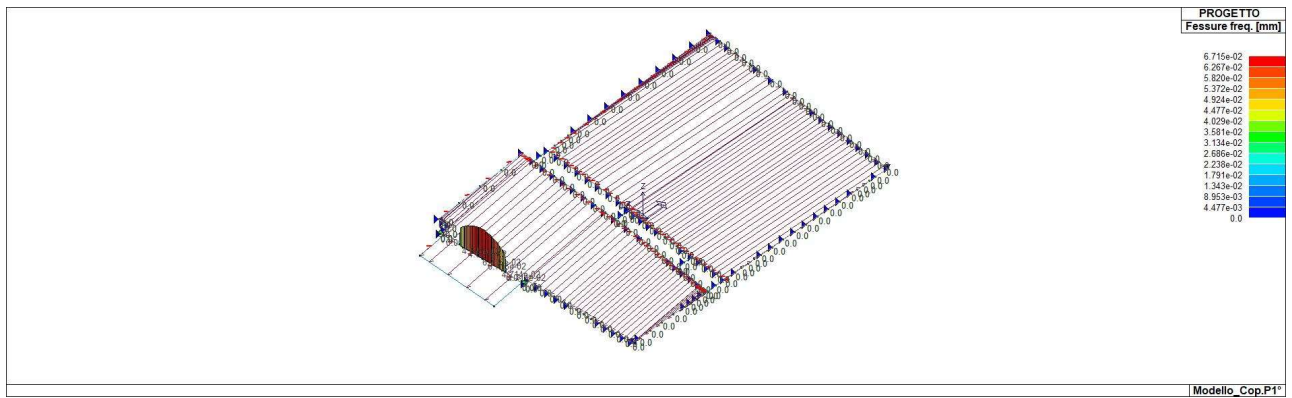


71_CA_TRV_19_Stato verif SLE

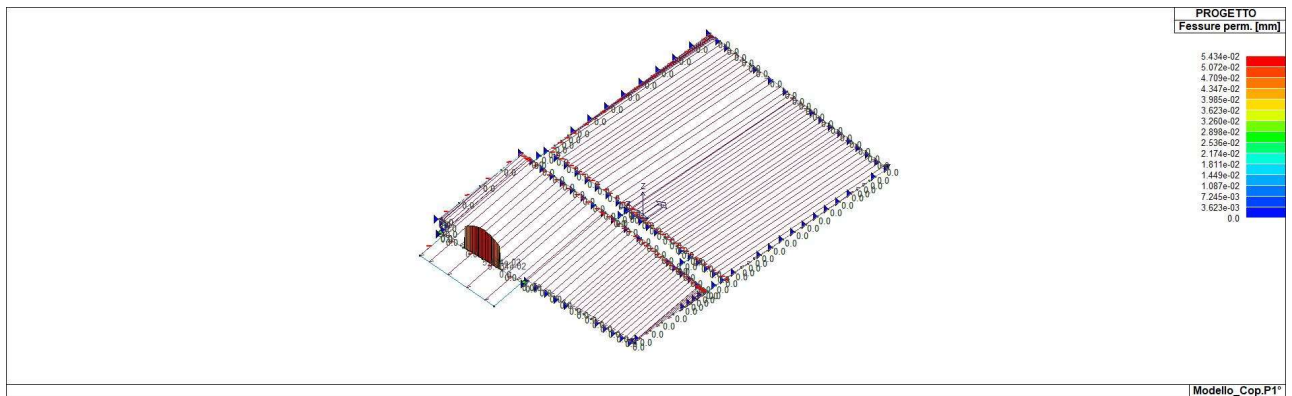


71_CA_TRV_20_Fessure rare

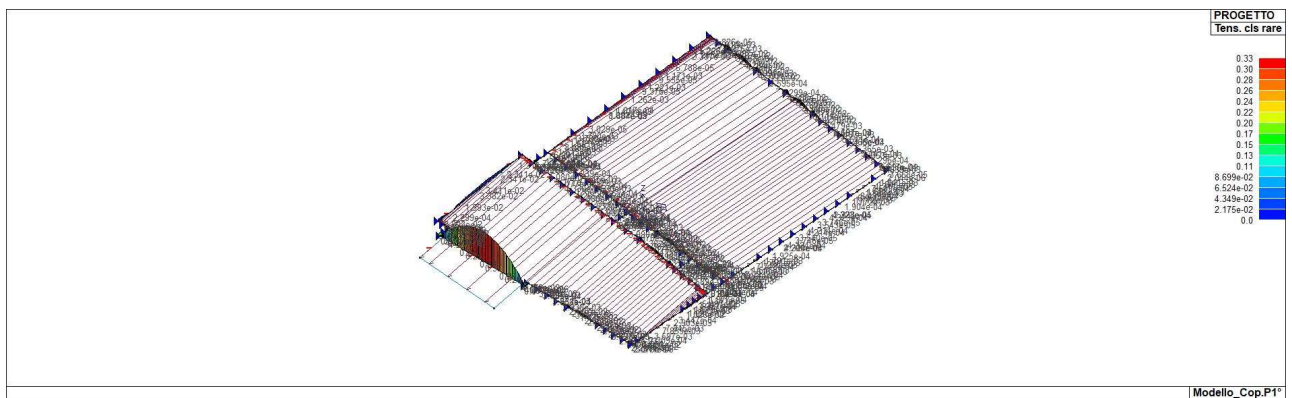
INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



71_CA_TRV_21_Fessure freq



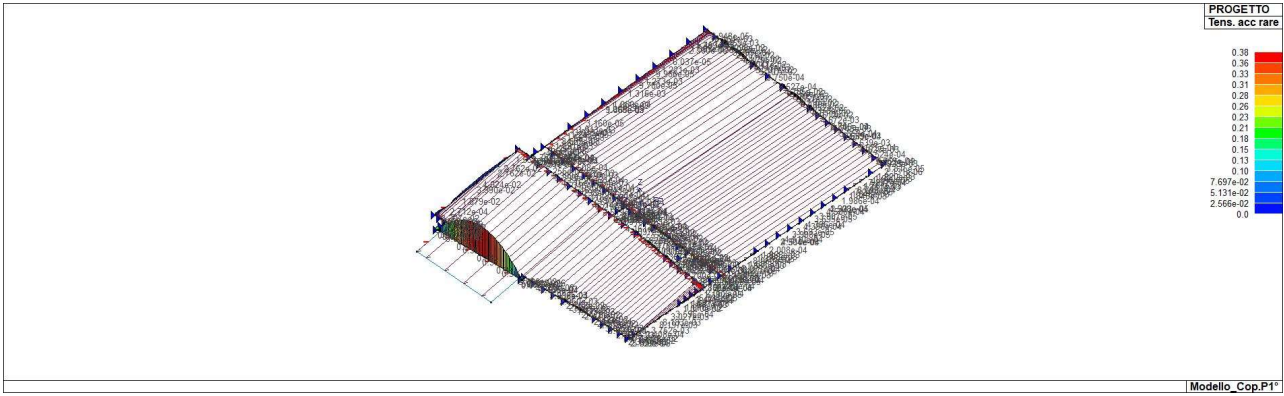
71_CA_TRV_22_Fessure perm



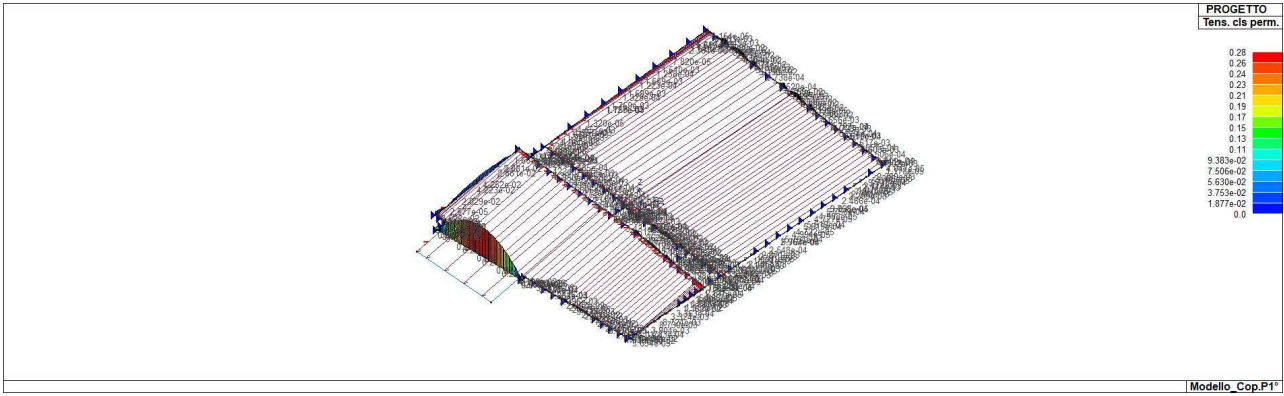
71_CA_TRV_23_Tens cls rare

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



71_CA_TRV_24_Tens acc rare



71_CA_TRV_25_Tens cls perm

STATI LIMITE D' ESERCIZIO

LEGENDA TABELLA STATI LIMITE D' ESERCIZIO

In tabella vengono riportati i valori di interesse per il controllo degli stati limite d'esercizio.

In particolare vengono riportati, in relazione al tipo di elemento strutturale, i risultati relativi alle tre categorie di combinazione considerate:

Combinazioni rare

Combinazioni frequenti

Combinazioni quasi permanenti.

I valori di interesse sono i seguenti:

rRfck	rapporto tra la massima compressione nel calcestruzzo e la tensione fck in combinazioni rare [normalizzato a 1]
rRfyk	rapporto tra la massima tensione nell'acciaio e la tensione fyk in combinazioni rare [normalizzato a 1]
rPfck	rapporto tra la massima compressione nel calcestruzzo e la tensione fck in combinazioni quasi permanenti [normalizzato a 1]
wR	apertura caratteristica delle fessure in combinazioni rare [mm]
wF	apertura caratteristica delle fessure in combinazioni frequenti [mm]
wP	apertura caratteristica delle fessure in combinazioni quasi permanenti [mm]
dR	massima deformazione in combinazioni rare
dF	massima deformazione in combinazioni frequenti
dP	massima deformazione in combinazioni quasi permanenti

Per ognuno dei nove valori soprariportati viene indicata (Rif.cmb) la combinazione in cui si è verificato.

In relazione al tipo di elemento strutturale i valori sono selezionati nel modo seguente:

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

pilastri	rRfck	rRfyk	rPfck	per sezioni significative
travi	rRfck	rRfyk	rPfck	per sezioni significative
	wR	wF	wP	per sezioni significative
	dR	dF	dP	massimi in campata
setti e gusci	rRfck	rRfyk	rPfck	massimi nei nodi dell'elemento
	wR	wF	wP	massimi nei nodi dell'elemento

Si precisa che i valori di massima deformazione per travi sono riferiti al piano verticale (piano locale 1-2 con momenti flettenti 3-3).

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	cm					mm	mm	mm		cm	cm	cm	
1	0.0	6.58e-03	6.86e-03	6.84e-03	2,2,4	0.0	0.0	0.0	0,0,0	-8.71e-05	-7.34e-05	-6.79e-05	2,3,4
	26.0	3.51e-03	3.66e-03	3.65e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	52.0	0.05	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0				
2	0.0	4.48e-03	4.67e-03	4.66e-03	2,2,4	0.0	0.0	0.0	0,0,0	9.10e-05	7.67e-05	7.10e-05	2,3,4
	16.2	0.01	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	0.04	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0				
3	0.0	6.54e-03	6.82e-03	5.52e-03	2,2,4	0.0	0.0	0.0	0,0,0	-7.72e-05	-5.70e-05	-4.89e-05	2,3,4
	17.4	1.47e-03	1.54e-03	1.24e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	4.72e-03	4.92e-03	3.98e-03	2,2,4	0.0	0.0	0.0	0,0,0				
4	0.0	0.03	0.04	0.03	2,2,4	0.0	0.0	0.0	0,0,0	1.05e-04	7.76e-05	6.67e-05	2,3,4
	22.1	7.89e-03	8.23e-03	6.69e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	44.1	4.21e-03	4.39e-03	3.55e-03	2,2,4	0.0	0.0	0.0	0,0,0				
5	0.0	0.03	0.04	0.03	2,2,4	0.0	0.0	0.0	0,0,0	-2.95e-03	-2.18e-03	-1.87e-03	2,3,4
	13.5	7.03e-03	7.33e-03	5.95e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
6	0.0	4.46e-03	4.65e-03	3.78e-03	2,2,4	0.0	0.0	0.0	0,0,0	6.77e-05	5.00e-05	4.30e-05	2,3,4

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	11.5	0.02	0.02	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
	23.0	0.03	0.04	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
7	0.0	3.33e-04	3.47e-04	3.53e-04	2,2,4	0.0	0.0	0.0	0,0,0	-2.63e-05	-2.04e-05	-1.80e-05	2,3,4
	13.6	1.11e-03	1.16e-03	1.05e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	2.02e-03	2.10e-03	1.74e-03	2,2,4	0.0	0.0	0.0	0,0,0				
8	0.0	4.28e-03	4.46e-03	4.45e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.31e-04	-1.10e-04	-1.02e-04	2,3,4
	16.0	5.91e-03	6.16e-03	6.16e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.0	2.83e-05	2.95e-05	3.15e-05	2,2,4	0.0	0.0	0.0	0,0,0				
9	0.0	1.22e-03	1.27e-03	1.56e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.84e-05	-2.76e-05	-2.73e-05	2,3,4
	28.8	5.92e-04	6.18e-04	7.60e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	1.17e-03	1.22e-03	1.51e-03	2,2,4	0.0	0.0	0.0	0,0,0				
10	0.0	1.40e-03	1.46e-03	1.76e-03	2,2,4	0.0	0.0	0.0	0,0,0	-3.01e-05	-2.90e-05	-2.85e-05	2,3,4
	28.8	6.20e-04	6.47e-04	7.85e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	1.26e-03	1.32e-03	1.60e-03	2,2,4	0.0	0.0	0.0	0,0,0				
11	0.0	1.07e-03	1.12e-03	1.40e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.30e-05	-2.28e-05	-2.27e-05	2,3,4
	28.8	4.59e-04	4.78e-04	6.02e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	1.25e-03	1.31e-03	1.65e-03	2,2,4	0.0	0.0	0.0	0,0,0				
12	0.0	9.68e-04	1.01e-03	1.23e-03	2,2,4	0.0	0.0	0.0	0,0,0	-3.83e-05	-3.64e-05	-3.56e-05	2,3,4
	28.8	8.50e-04	8.87e-04	1.05e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	1.40e-03	1.46e-03	1.76e-03	2,2,4	0.0	0.0	0.0	0,0,0				
13	0.0	1.17e-03	1.22e-03	1.51e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.77e-05	-2.71e-05	-2.69e-05	2,3,4
	28.8	5.83e-04	6.08e-04	7.56e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	1.07e-03	1.12e-03	1.40e-03	2,2,4	0.0	0.0	0.0	0,0,0				
14	0.0	1.26e-03	1.32e-03	1.60e-03	2,2,4	0.0	0.0	0.0	0,0,0	-3.00e-05	-2.90e-05	-2.86e-05	2,3,4
	28.8	6.30e-04	6.57e-04	8.00e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	1.22e-03	1.27e-03	1.56e-03	2,2,4	0.0	0.0	0.0	0,0,0				
15	0.0	1.25e-03	1.31e-03	1.65e-03	2,2,4	0.0	0.0	0.0	0,0,0	-4.04e-05	-4.00e-05	-3.99e-05	2,3,4
	28.8	9.75e-04	1.02e-03	1.28e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	57.6	1.25e-04	1.30e-04	1.27e-04	2,2,4	0.0	0.0	0.0	0,0,0				
16	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-2.30e-03	-1.94e-03	-1.80e-03	2,3,4
	10.8	0.03	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
	21.5	0.03	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
17	0.0	3.50e-03	3.65e-03	4.19e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.83e-04	-2.62e-04	-2.53e-04	2,3,4
	11.8	3.85e-04	4.01e-04	4.67e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	23.5	1.81e-03	1.89e-03	2.16e-03	2,2,4	0.0	0.0	0.0	0,0,0				
18	0.0	3.78e-03	3.94e-03	4.56e-03	2,2,4	0.0	0.0	0.0	0,0,0	8.27e-06	7.06e-06	6.58e-06	2,3,4
	28.8	2.55e-04	2.66e-04	2.62e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	57.6	9.68e-04	1.01e-03	1.23e-03	2,2,4	0.0	0.0	0.0	0,0,0				
19	0.0	0.05	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0	1.12e-04	9.46e-05	8.76e-05	2,3,4
	16.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	32.0	4.28e-03	4.46e-03	4.45e-03	2,2,4	0.0	0.0	0.0	0,0,0				
20	0.0	0.05	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0	-3.81e-03	-3.21e-03	-2.97e-03	2,3,4
	12.5	9.42e-03	9.83e-03	9.79e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.1	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
21	0.0	0.05	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0	-1.28e-04	-1.08e-04	-9.98e-05	2,3,4
	26.0	1.54e-03	1.61e-03	1.60e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	52.0	6.58e-03	6.86e-03	6.84e-03	2,2,4	0.0	0.0	0.0	0,0,0				
22	0.0	0.04	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0	-3.28e-03	-2.76e-03	-2.56e-03	2,3,4
	12.0	7.47e-03	7.80e-03	7.77e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	24.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
23	0.0	3.66e-05	3.81e-05	4.05e-05	2,2,4	0.0	0.0	0.0	0,0,0	-1.47e-04	-1.24e-04	-1.15e-04	2,3,4
	16.2	6.14e-03	6.40e-03	6.38e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	6.64e-03	6.92e-03	6.90e-03	2,2,4	0.0	0.0	0.0	0,0,0				
24	0.0	1.84e-03	1.92e-03	2.39e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.68e-06	-3.12e-06	-3.29e-06	2,3,4
	20.5	2.35e-04	2.45e-04	2.58e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	41.0	1.49e-04	1.55e-04	1.51e-04	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
25	0.0	1.77e-03	1.85e-03	2.30e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.62e-05	-9.39e-05	-9.30e-05	2,3,4
	10.3	2.44e-04	2.54e-04	3.26e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	20.5	8.44e-04	8.80e-04	1.08e-03	2,2,4	0.0	0.0	0.0	0,0,0				
26	0.0	1.79e-03	1.87e-03	2.37e-03	2,2,4	0.0	0.0	0.0	0,0,0	2.30e-06	2.24e-06	2.22e-06	2,3,4
	18.7	4.12e-04	4.30e-04	5.42e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	2.98e-04	3.10e-04	3.98e-04	2,2,4	0.0	0.0	0.0	0,0,0				
27	0.0	1.73e-03	1.80e-03	2.31e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.15e-04	-1.14e-04	-1.14e-04	2,3,4
	10.7	2.69e-04	2.81e-04	3.62e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	21.4	7.74e-04	8.08e-04	1.03e-03	2,2,4	0.0	0.0	0.0	0,0,0				
28	0.0	2.27e-04	2.37e-04	3.02e-04	2,2,4	0.0	0.0	0.0	0,0,0	1.97e-06	1.97e-06	1.97e-06	2,3,4
	17.5	4.24e-04	4.42e-04	5.66e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	35.0	1.73e-03	1.80e-03	2.31e-03	2,2,4	0.0	0.0	0.0	0,0,0				
29	0.0	4.20e-03	4.39e-03	4.35e-03	2,2,4	0.0	0.0	0.0	0,0,0	-7.98e-05	-6.73e-05	-6.23e-05	2,3,4
	13.6	2.40e-03	2.50e-03	2.51e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	4.50e-03	4.69e-03	4.67e-03	2,2,4	0.0	0.0	0.0	0,0,0				
30	0.0	3.94e-03	4.11e-03	4.11e-03	2,2,4	0.0	0.0	0.0	0,0,0	4.45e-05	3.75e-05	3.47e-05	2,3,4
	15.5	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
	31.0	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
31	0.0	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0	-2.30e-03	-1.94e-03	-1.80e-03	2,3,4
	10.8	5.64e-03	5.88e-03	5.86e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	21.5	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
32	0.0	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0	4.52e-05	3.80e-05	3.52e-05	2,3,4
	12.8	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
	25.5	4.81e-03	5.01e-03	5.00e-03	2,2,4	0.0	0.0	0.0	0,0,0				
33	0.0	4.25e-04	4.43e-04	4.13e-04	2,2,4	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	2,3,4
	3.0	5.09e-04	5.31e-04	5.12e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	6.1	5.60e-04	5.84e-04	5.67e-04	2,2,4	0.0	0.0	0.0	0,0,0				
34	0.0	3.97e-03	4.14e-03	4.11e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.61e-04	-1.35e-04	-1.25e-04	2,3,4

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	17.6	5.37e-03	5.60e-03	5.59e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	7.75e-03	8.09e-03	8.06e-03	2,2,4	0.0	0.0	0.0	0,0,0				
35	0.0	0.03	0.03	0.04	2,2,4	0.0	0.0	0.0	0,0,0	9.80e-05	9.29e-05	9.09e-05	2,3,4
	12.3	0.01	0.01	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	24.5	1.09e-03	1.04e-03	1.32e-03	2,2,4	0.0	0.0	0.0	0,0,0				
36	0.0	2.58e-03	2.46e-03	2.48e-03	2,2,4	0.0	0.0	0.0	0,0,0	3.97e-06	2.55e-06	2.00e-06	2,3,4
	10.8	1.07e-03	1.02e-03	9.45e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	21.5	2.24e-04	2.13e-04	2.97e-04	2,2,4	0.0	0.0	0.0	0,0,0				
37	0.0	1.41e-04	1.34e-04	1.35e-04	2,2,4	0.0	0.0	0.0	0,0,0	-2.49e-06	-2.53e-06	-2.55e-06	2,3,4
	12.8	2.52e-04	2.41e-04	3.54e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	25.5	2.85e-04	2.72e-04	3.98e-04	2,2,4	0.0	0.0	0.0	0,0,0				
38	0.0	3.08e-04	2.94e-04	4.05e-04	2,2,4	0.0	0.0	0.0	0,0,0	-2.90e-06	-2.90e-06	-2.90e-06	2,3,4
	14.3	2.29e-04	2.19e-04	3.08e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	28.5	3.95e-04	3.77e-04	5.29e-04	2,2,4	0.0	0.0	0.0	0,0,0				
39	0.0	0.03	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0	-2.95e-03	-2.18e-03	-1.87e-03	2,3,4
	13.5	0.03	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	27.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
40	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-3.28e-03	-2.76e-03	-2.56e-03	2,3,4
	12.0	8.79e-03	9.16e-03	9.13e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	24.0	0.05	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0				
41	0.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0	-2.95e-03	-2.18e-03	-1.87e-03	2,3,4
	13.5	5.74e-03	5.99e-03	4.86e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.0	0.03	0.04	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
42	0.0	7.51e-04	7.83e-04	9.74e-04	2,2,4	0.0	0.0	0.0	0,0,0	-9.62e-05	-9.39e-05	-9.30e-05	2,3,4
	10.3	3.55e-04	3.70e-04	4.55e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	20.5	1.84e-03	1.92e-03	2.39e-03	2,2,4	0.0	0.0	0.0	0,0,0				
43	0.0	3.49e-03	3.64e-03	4.16e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.83e-04	-2.62e-04	-2.53e-04	2,3,4
	11.8	2.99e-03	3.11e-03	3.56e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	23.5	1.60e-03	1.67e-03	1.90e-03	2,2,4	0.0	0.0	0.0	0,0,0				
44	0.0	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0	-3.81e-03	-3.21e-03	-2.97e-03	2,3,4
	12.5	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
	25.1	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
45	0.0	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0	-3.28e-03	-2.76e-03	-2.56e-03	2,3,4
	12.0	0.03	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
	24.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
46	0.0	1.69e-03	1.76e-03	2.17e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.62e-05	-9.39e-05	-9.30e-05	2,3,4
	10.3	1.44e-03	1.51e-03	1.86e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	20.5	7.51e-04	7.83e-04	9.74e-04	2,2,4	0.0	0.0	0.0	0,0,0				
47	0.0	1.62e-03	1.69e-03	2.15e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.15e-04	-1.14e-04	-1.14e-04	2,3,4
	10.7	1.41e-03	1.47e-03	1.87e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	21.4	7.72e-04	8.05e-04	1.02e-03	2,2,4	0.0	0.0	0.0	0,0,0				
48	0.0	0.03	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0	-2.30e-03	-1.94e-03	-1.80e-03	2,3,4
	10.8	0.03	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
	21.5	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
49	0.0	7.72e-04	8.05e-04	1.02e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.15e-04	-1.14e-04	-1.14e-04	2,3,4
	10.7	2.94e-04	3.07e-04	3.89e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	21.4	1.79e-03	1.87e-03	2.37e-03	2,2,4	0.0	0.0	0.0	0,0,0				
50	0.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0	-2.95e-03	-2.18e-03	-1.87e-03	2,3,4
	13.5	0.03	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	27.0	0.03	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
51	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-2.30e-03	-1.94e-03	-1.80e-03	2,3,4
	10.8	5.68e-03	5.93e-03	5.91e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	21.5	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
52	0.0	1.60e-03	1.67e-03	1.90e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.83e-04	-2.62e-04	-2.53e-04	2,3,4
	11.8	6.58e-04	6.86e-04	8.09e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	23.5	3.78e-03	3.94e-03	4.56e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
53	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-3.81e-03	-3.21e-03	-2.97e-03	2,3,4
	12.5	8.58e-03	8.95e-03	8.92e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.1	0.05	0.05	0.05	2,2,4	0.0	0.0	0.0	0,0,0				
54	0.0	1.81e-03	1.89e-03	2.16e-03	2,2,4	0.0	0.0	0.0	0,0,0	-2.83e-04	-2.62e-04	-2.53e-04	2,3,4
	11.8	3.10e-03	3.23e-03	3.70e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	23.5	3.49e-03	3.64e-03	4.16e-03	2,2,4	0.0	0.0	0.0	0,0,0				
55	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-3.81e-03	-3.21e-03	-2.97e-03	2,3,4
	12.5	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
	25.1	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
56	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-3.28e-03	-2.76e-03	-2.56e-03	2,3,4
	12.0	0.03	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
	24.0	0.04	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
57	0.0	8.44e-04	8.80e-04	1.08e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.62e-05	-9.39e-05	-9.30e-05	2,3,4
	10.3	1.49e-03	1.55e-03	1.91e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	20.5	1.69e-03	1.76e-03	2.17e-03	2,2,4	0.0	0.0	0.0	0,0,0				
58	0.0	2.55e-03	2.66e-03	2.16e-03	2,2,4	0.0	0.0	0.0	0,0,0	-4.48e-05	-3.32e-05	-2.86e-05	2,3,4
	13.6	1.28e-03	1.34e-03	1.09e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	2.59e-03	2.70e-03	2.20e-03	2,2,4	0.0	0.0	0.0	0,0,0				
59	0.0	2.02e-03	2.10e-03	1.74e-03	2,2,4	0.0	0.0	0.0	0,0,0	-4.62e-05	-3.42e-05	-2.94e-05	2,3,4
	13.6	1.52e-03	1.58e-03	1.28e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	2.55e-03	2.66e-03	2.16e-03	2,2,4	0.0	0.0	0.0	0,0,0				
60	0.0	2.59e-03	2.70e-03	2.20e-03	2,2,4	0.0	0.0	0.0	0,0,0	-3.93e-05	-2.91e-05	-2.51e-05	2,3,4
	13.6	5.00e-04	5.22e-04	4.29e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	4.21e-03	4.39e-03	3.56e-03	2,2,4	0.0	0.0	0.0	0,0,0				
61	0.0	4.21e-03	4.39e-03	3.55e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.95e-04	-1.44e-04	-1.23e-04	2,3,4
	22.1	5.79e-03	6.04e-03	4.89e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	44.1	6.80e-03	7.09e-03	5.74e-03	2,2,4	0.0	0.0	0.0	0,0,0				
62	0.0	5.08e-03	5.30e-03	4.27e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.51e-05	-7.00e-05	-6.00e-05	2,3,4

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	17.4	2.55e-03	2.66e-03	2.15e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	5.13e-03	5.35e-03	4.31e-03	2,2,4	0.0	0.0	0.0	0,0,0				
63	0.0	5.00e-03	5.22e-03	4.21e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.33e-05	-6.87e-05	-5.89e-05	2,3,4
	17.4	2.50e-03	2.61e-03	2.10e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	5.03e-03	5.25e-03	4.23e-03	2,2,4	0.0	0.0	0.0	0,0,0				
64	0.0	5.18e-03	5.40e-03	4.35e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.72e-05	-7.15e-05	-6.12e-05	2,3,4
	17.4	2.63e-03	2.74e-03	2.21e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	5.16e-03	5.38e-03	4.33e-03	2,2,4	0.0	0.0	0.0	0,0,0				
65	0.0	4.72e-03	4.92e-03	3.98e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.38e-05	-6.91e-05	-5.92e-05	2,3,4
	17.4	2.58e-03	2.69e-03	2.17e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	5.00e-03	5.22e-03	4.21e-03	2,2,4	0.0	0.0	0.0	0,0,0				
66	0.0	5.13e-03	5.35e-03	4.31e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.59e-05	-7.06e-05	-6.05e-05	2,3,4
	17.4	2.57e-03	2.68e-03	2.16e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	5.18e-03	5.40e-03	4.35e-03	2,2,4	0.0	0.0	0.0	0,0,0				
67	0.0	5.03e-03	5.25e-03	4.23e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.42e-05	-6.94e-05	-5.95e-05	2,3,4
	17.4	2.53e-03	2.64e-03	2.13e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	5.08e-03	5.30e-03	4.27e-03	2,2,4	0.0	0.0	0.0	0,0,0				
68	0.0	5.16e-03	5.38e-03	4.33e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.27e-05	-6.82e-05	-5.84e-05	2,3,4
	17.4	2.22e-03	2.32e-03	1.86e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	34.8	6.13e-03	6.39e-03	5.15e-03	2,2,4	0.0	0.0	0.0	0,0,0				
69	0.0	4.49e-03	4.68e-03	4.67e-03	2,2,4	0.0	0.0	0.0	0,0,0	-7.85e-05	-6.61e-05	-6.12e-05	2,3,4
	13.6	2.25e-03	2.35e-03	2.34e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	4.46e-03	4.65e-03	4.64e-03	2,2,4	0.0	0.0	0.0	0,0,0				
70	0.0	4.50e-03	4.69e-03	4.67e-03	2,2,4	0.0	0.0	0.0	0,0,0	-7.85e-05	-6.62e-05	-6.12e-05	2,3,4
	13.6	2.25e-03	2.34e-03	2.34e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	27.3	4.49e-03	4.68e-03	4.67e-03	2,2,4	0.0	0.0	0.0	0,0,0				
71	0.0	4.46e-03	4.65e-03	4.64e-03	2,2,4	0.0	0.0	0.0	0,0,0	-9.40e-05	-7.92e-05	-7.32e-05	2,3,4
	13.6	4.32e-03	4.51e-03	4.49e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	27.3	2.88e-04	3.00e-04	3.10e-04	2,2,4	0.0	0.0	0.0	0,0,0				
72	0.0	3.99e-03	4.16e-03	4.15e-03	2,2,4	0.0	0.0	0.0	0,0,0	-6.85e-05	-5.77e-05	-5.34e-05	2,3,4
	12.8	1.98e-03	2.06e-03	2.06e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.5	3.97e-03	4.14e-03	4.12e-03	2,2,4	0.0	0.0	0.0	0,0,0				
73	0.0	4.81e-03	5.01e-03	5.00e-03	2,2,4	0.0	0.0	0.0	0,0,0	-6.55e-05	-5.52e-05	-5.11e-05	2,3,4
	12.8	1.55e-03	1.61e-03	1.61e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.5	3.99e-03	4.16e-03	4.15e-03	2,2,4	0.0	0.0	0.0	0,0,0				
74	0.0	3.97e-03	4.14e-03	4.12e-03	2,2,4	0.0	0.0	0.0	0,0,0	-6.95e-05	-5.85e-05	-5.41e-05	2,3,4
	12.8	2.13e-03	2.23e-03	2.20e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.5	3.67e-03	3.83e-03	3.85e-03	2,2,4	0.0	0.0	0.0	0,0,0				
75	0.0	7.53e-03	7.85e-03	7.83e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.41e-04	-1.19e-04	-1.10e-04	2,3,4
	17.6	3.77e-03	3.93e-03	3.92e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	7.55e-03	7.87e-03	7.84e-03	2,2,4	0.0	0.0	0.0	0,0,0				
76	0.0	7.49e-03	7.81e-03	7.78e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.41e-04	-1.18e-04	-1.10e-04	2,3,4
	17.6	3.77e-03	3.93e-03	3.91e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	7.52e-03	7.84e-03	7.82e-03	2,2,4	0.0	0.0	0.0	0,0,0				
77	0.0	7.52e-03	7.84e-03	7.82e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.39e-04	-1.17e-04	-1.08e-04	2,3,4
	17.6	3.56e-03	3.72e-03	3.70e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	8.04e-03	8.38e-03	8.36e-03	2,2,4	0.0	0.0	0.0	0,0,0				
78	0.0	7.75e-03	8.09e-03	8.06e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.39e-04	-1.17e-04	-1.08e-04	2,3,4
	17.6	3.63e-03	3.79e-03	3.77e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	7.49e-03	7.81e-03	7.78e-03	2,2,4	0.0	0.0	0.0	0,0,0				
79	0.0	7.55e-03	7.87e-03	7.84e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.41e-04	-1.19e-04	-1.10e-04	2,3,4
	17.6	3.79e-03	3.95e-03	3.94e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	7.52e-03	7.84e-03	7.82e-03	2,2,4	0.0	0.0	0.0	0,0,0				
80	0.0	7.52e-03	7.84e-03	7.82e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.41e-04	-1.19e-04	-1.10e-04	2,3,4
	17.6	3.76e-03	3.92e-03	3.91e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	7.53e-03	7.85e-03	7.83e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
81	0.0	8.04e-03	8.38e-03	8.36e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.79e-04	-1.51e-04	-1.40e-04	2,3,4
	17.6	6.75e-03	7.04e-03	7.02e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	35.1	1.19e-03	1.24e-03	1.22e-03	2,2,4	0.0	0.0	0.0	0,0,0				
82	0.0	1.09e-03	1.04e-03	1.32e-03	2,2,4	0.0	0.0	0.0	0,0,0	9.82e-06	8.75e-06	8.32e-06	2,3,4
	12.3	1.60e-03	1.53e-03	1.85e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	24.5	5.15e-03	4.91e-03	6.17e-03	2,2,4	0.0	0.0	0.0	0,0,0				
83	0.0	2.24e-04	2.13e-04	2.97e-04	2,2,4	0.0	0.0	0.0	0,0,0	-1.67e-06	-1.66e-06	-1.65e-06	2,3,4
	10.8	1.95e-04	1.86e-04	2.48e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	21.5	4.74e-05	4.52e-05	8.84e-05	2,2,4	0.0	0.0	0.0	0,0,0				
84	0.0	2.85e-04	2.72e-04	3.98e-04	2,2,4	0.0	0.0	0.0	0,0,0	-1.67e-06	-1.15e-06	-1.25e-06	2,3,4
	12.8	1.55e-04	1.48e-04	7.77e-05	2,2,4	0.0	0.0	0.0	0,0,0				
	25.5	9.55e-04	9.11e-04	9.98e-04	2,2,4	0.0	0.0	0.0	0,0,0				
85	0.0	7.74e-04	8.08e-04	1.03e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.15e-04	-1.14e-04	-1.14e-04	2,3,4
	10.7	1.40e-03	1.46e-03	1.86e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	21.4	1.62e-03	1.69e-03	2.15e-03	2,2,4	0.0	0.0	0.0	0,0,0				
86	0.0	4.21e-04	4.40e-04	5.62e-04	2,2,4	0.0	0.0	0.0	0,0,0	-4.73e-06	-4.73e-06	-4.73e-06	2,3,4
	18.7	2.11e-04	2.20e-04	2.81e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	4.20e-04	4.38e-04	5.60e-04	2,2,4	0.0	0.0	0.0	0,0,0				
87	0.0	4.32e-04	4.51e-04	5.76e-04	2,2,4	0.0	0.0	0.0	0,0,0	-4.68e-06	-4.68e-06	-4.68e-06	2,3,4
	18.7	2.06e-04	2.14e-04	2.74e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	4.20e-04	4.38e-04	5.60e-04	2,2,4	0.0	0.0	0.0	0,0,0				
88	0.0	4.32e-04	4.51e-04	5.76e-04	2,2,4	0.0	0.0	0.0	0,0,0	-8.85e-06	-8.83e-06	-8.82e-06	2,3,4
	18.7	2.66e-04	2.78e-04	3.52e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	2.99e-04	3.12e-04	4.05e-04	2,2,4	0.0	0.0	0.0	0,0,0				
89	0.0	2.98e-04	3.10e-04	3.98e-04	2,2,4	0.0	0.0	0.0	0,0,0	-8.86e-06	-8.86e-06	-8.85e-06	2,3,4
	18.7	2.67e-04	2.78e-04	3.55e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	4.32e-04	4.51e-04	5.76e-04	2,2,4	0.0	0.0	0.0	0,0,0				
90	0.0	4.20e-04	4.38e-04	5.60e-04	2,2,4	0.0	0.0	0.0	0,0,0	-4.68e-06	-4.68e-06	-4.69e-06	2,3,4

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	18.7	2.06e-04	2.14e-04	2.74e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	4.32e-04	4.51e-04	5.76e-04	2,2,4	0.0	0.0	0.0	0,0,0				
91	0.0	4.20e-04	4.38e-04	5.60e-04	2,2,4	0.0	0.0	0.0	0,0,0	-4.73e-06	-4.73e-06	-4.73e-06	2,3,4
	18.7	2.11e-04	2.20e-04	2.81e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	4.21e-04	4.40e-04	5.62e-04	2,2,4	0.0	0.0	0.0	0,0,0				
92	0.0	2.99e-04	3.12e-04	4.05e-04	2,2,4	0.0	0.0	0.0	0,0,0	2.19e-06	2.01e-06	1.94e-06	2,3,4
	18.7	4.04e-04	4.21e-04	5.10e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	37.4	1.77e-03	1.85e-03	2.30e-03	2,2,4	0.0	0.0	0.0	0,0,0				
93	0.0	6.36e-03	6.64e-03	6.62e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.16e-04	-9.79e-05	-9.06e-05	2,3,4
	16.2	3.19e-03	3.33e-03	3.31e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	6.37e-03	6.64e-03	6.62e-03	2,2,4	0.0	0.0	0.0	0,0,0				
94	0.0	6.32e-03	6.59e-03	6.57e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.16e-04	-9.76e-05	-9.03e-05	2,3,4
	16.2	3.18e-03	3.32e-03	3.31e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	6.35e-03	6.63e-03	6.60e-03	2,2,4	0.0	0.0	0.0	0,0,0				
95	0.0	6.47e-03	6.75e-03	6.72e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.26e-04	-1.06e-04	-9.82e-05	2,3,4
	16.2	4.09e-03	4.26e-03	4.25e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	4.48e-03	4.67e-03	4.66e-03	2,2,4	0.0	0.0	0.0	0,0,0				
96	0.0	6.64e-03	6.92e-03	6.90e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.14e-04	-9.60e-05	-8.88e-05	2,3,4
	16.2	3.02e-03	3.15e-03	3.14e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	6.32e-03	6.59e-03	6.57e-03	2,2,4	0.0	0.0	0.0	0,0,0				
97	0.0	6.37e-03	6.64e-03	6.62e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.16e-04	-9.75e-05	-9.02e-05	2,3,4
	16.2	3.14e-03	3.28e-03	3.26e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	6.47e-03	6.75e-03	6.72e-03	2,2,4	0.0	0.0	0.0	0,0,0				
98	0.0	6.35e-03	6.63e-03	6.60e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.16e-04	-9.77e-05	-9.04e-05	2,3,4
	16.2	3.18e-03	3.32e-03	3.31e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	32.4	6.36e-03	6.64e-03	6.62e-03	2,2,4	0.0	0.0	0.0	0,0,0				
99	0.0	6.94e-03	7.24e-03	5.91e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.74e-04	-1.30e-04	-1.12e-04	2,3,4
	19.5	6.45e-03	6.73e-03	5.66e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	39.0	1.06e-03	1.11e-03	1.41e-03	2,2,4	0.0	0.0	0.0	0,0,0				
100	0.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0	1.89e-05	1.67e-05	1.58e-05	2,3,4
	9.4	5.71e-03	5.96e-03	6.40e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	18.8	1.58e-03	1.65e-03	1.78e-03	2,2,4	0.0	0.0	0.0	0,0,0				
101	0.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0	-1.36e-03	-1.20e-03	-1.13e-03	2,3,4
	19.3	2.28e-03	2.38e-03	2.58e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	38.5	3.60e-03	3.75e-03	3.90e-03	2,2,4	0.0	0.0	0.0	0,0,0				
102	0.0	5.31e-05	5.54e-05	5.65e-05	2,2,4	0.0	0.0	0.0	0,0,0	2.16e-05	1.90e-05	1.80e-05	2,3,4
	11.0	5.01e-03	5.22e-03	5.53e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	22.0	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
103	0.0	0.03	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0	4.75e-05	3.51e-05	3.01e-05	2,3,4
	13.0	0.01	0.01	9.43e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.9	2.90e-03	3.02e-03	2.46e-03	2,2,4	0.0	0.0	0.0	0,0,0				
104	0.0	0.03	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-1.95e-03	-1.44e-03	-1.24e-03	2,3,4
	11.9	4.51e-03	4.70e-03	3.82e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	23.8	0.01	0.01	9.51e-03	2,2,4	0.0	0.0	0.0	0,0,0				
105	0.0	0.0	0.0	1.03e-06	2,2,4	0.0	0.0	0.0	0,0,0	-8.85e-05	-6.52e-05	-5.60e-05	2,3,4
	15.7	3.79e-03	3.95e-03	3.20e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	31.3	3.99e-03	4.17e-03	3.37e-03	2,2,4	0.0	0.0	0.0	0,0,0				
106	0.0	0.03	0.03	0.04	2,2,4	0.0	0.0	0.0	0,0,0	-0.02	-0.02	-0.02	2,3,4
	37.7	2.93e-03	3.46e-03	3.38e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	75.4	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
107	0.0	9.46e-04	1.12e-03	1.11e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.38e-05	-1.06e-05	-9.31e-06	2,3,4
	8.5	1.26e-03	1.49e-03	1.25e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	17.1	6.26e-04	7.39e-04	5.41e-04	2,2,4	0.0	0.0	0.0	0,0,0				
108	0.0	3.25e-04	3.83e-04	3.32e-04	2,2,4	0.0	0.0	0.0	0,0,0	5.44e-06	5.17e-06	5.07e-06	2,3,4
	8.6	2.48e-03	2.93e-03	3.08e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	17.1	5.01e-03	5.91e-03	6.28e-03	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
109	0.0	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	0,0,0	-0.02	-0.02	-0.01	2,3,4
	4.5	0.02	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	9.1	0.04	0.05	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
110	0.0	0.04	0.05	0.04	2,2,4	0.0	0.0	0.0	0,0,0	-0.25	-0.16	-0.13	2,3,4
	30.7	0.17	0.20	0.14	2,2,4	0.0	0.0	0.0	0,0,0				
	61.4	0.25	0.30	0.22	2,2,4	0.06	0.05	0.0	2,3,0				
111	0.0	0.05	0.06	0.04	2,2,4	0.0	0.0	0.0	0,0,0	0.02	0.02	0.01	2,3,4
	5.1	0.02	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	10.2	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	0,0,0				
112	0.0	1.41e-04	1.66e-04	1.84e-04	2,2,4	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	2,3,4
	6.0	1.31e-04	1.55e-04	1.77e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	12.0	3.03e-05	3.58e-05	3.23e-05	2,2,4	0.0	0.0	0.0	0,0,0				
113	0.0	3.83e-03	4.00e-03	3.23e-03	2,2,4	0.0	0.0	0.0	0,0,0	-7.28e-05	-5.37e-05	-4.61e-05	2,3,4
	15.7	2.34e-03	2.44e-03	1.97e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	31.3	2.84e-03	2.96e-03	2.40e-03	2,2,4	0.0	0.0	0.0	0,0,0				
114	0.0	3.99e-03	4.17e-03	3.37e-03	2,2,4	0.0	0.0	0.0	0,0,0	-6.81e-05	-5.03e-05	-4.31e-05	2,3,4
	15.7	1.82e-03	1.90e-03	1.53e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	31.3	3.83e-03	4.00e-03	3.23e-03	2,2,4	0.0	0.0	0.0	0,0,0				
115	0.0	2.84e-03	2.96e-03	2.40e-03	2,2,4	0.0	0.0	0.0	0,0,0	5.13e-05	3.79e-05	3.26e-05	2,3,4
	15.7	9.15e-03	9.54e-03	7.74e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	31.3	0.03	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
116	0.0	2.36e-03	2.47e-03	2.01e-03	2,2,4	0.0	0.0	0.0	0,0,0	-4.06e-05	-3.01e-05	-2.59e-05	2,3,4
	13.0	1.19e-03	1.24e-03	1.01e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.9	2.28e-03	2.38e-03	1.94e-03	2,2,4	0.0	0.0	0.0	0,0,0				
117	0.0	2.90e-03	3.02e-03	2.46e-03	2,2,4	0.0	0.0	0.0	0,0,0	-3.92e-05	-1.70e-05	-1.46e-05	2,3,4
	13.0	9.23e-04	9.62e-04	7.83e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	25.9	2.36e-03	2.47e-03	2.01e-03	2,2,4	0.0	0.0	0.0	0,0,0				
118	0.0	2.28e-03	2.38e-03	1.94e-03	2,2,4	0.0	0.0	0.0	0,0,0	-4.73e-05	-3.50e-05	-3.00e-05	2,3,4

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	13.0	2.22e-03	2.32e-03	1.86e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	25.9	2.08e-04	2.17e-04	2.21e-04	2,2,4	0.0	0.0	0.0	0,0,0				
119	0.0	3.33e-04	3.47e-04	3.90e-04	2,2,4	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	2,3,4
	9.4	6.51e-05	6.79e-05	6.57e-05	2,2,4	0.0	0.0	0.0	0,0,0				
	18.8	1.57e-04	1.64e-04	1.99e-04	2,2,4	0.0	0.0	0.0	0,0,0				
120	0.0	1.58e-03	1.65e-03	1.78e-03	2,2,4	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	2,3,4
	9.4	7.62e-04	7.95e-04	8.47e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	18.8	3.33e-04	3.47e-04	3.90e-04	2,2,4	0.0	0.0	0.0	0,0,0				
121	0.0	1.57e-04	1.64e-04	1.99e-04	2,2,4	0.0	0.0	0.0	0,0,0	0.0	0.0	0.0	2,3,4
	9.4	1.16e-05	1.21e-05	1.43e-05	2,2,4	0.0	0.0	0.0	0,0,0				
	18.8	1.98e-04	2.06e-04	2.65e-04	2,2,4	0.0	0.0	0.0	0,0,0				
122	0.0	3.95e-04	3.77e-04	5.29e-04	2,2,4	0.0	0.0	0.0	0,0,0	-2.74e-06	-2.76e-06	-2.76e-06	2,3,4
	14.3	1.98e-04	1.89e-04	2.71e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	28.5	3.70e-04	3.53e-04	4.79e-04	2,2,4	0.0	0.0	0.0	0,0,0				
123	0.0	6.26e-04	7.39e-04	5.41e-04	2,2,4	0.0	0.0	0.0	0,0,0	-9.06e-06	-6.60e-06	-5.62e-06	2,3,4
	8.5	8.40e-04	9.91e-04	5.52e-04	2,2,4	0.0	0.0	0.0	0,0,0				
	17.1	8.06e-04	9.51e-04	1.01e-03	2,2,4	0.0	0.0	0.0	0,0,0				
124	0.0	5.01e-03	5.91e-03	6.28e-03	2,2,4	0.0	0.0	0.0	0,0,0	4.47e-05	4.29e-05	4.21e-05	2,3,4
	8.6	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	17.1	0.03	0.03	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
127	0.0	7.85e-03	8.18e-03	8.72e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.36e-03	-1.20e-03	-1.13e-03	2,3,4
	19.3	6.46e-03	6.74e-03	7.25e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	38.5	2.90e-03	3.03e-03	3.32e-03	2,2,4	0.0	0.0	0.0	0,0,0				
128	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-1.95e-03	-1.44e-03	-1.24e-03	2,3,4
	11.9	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	23.8	0.01	0.01	9.27e-03	2,2,4	0.0	0.0	0.0	0,0,0				
129	0.0	0.03	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0	-0.02	-0.02	-0.02	2,3,4
	37.7	0.03	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Trave	Pos.	rRfck	rRfyk	rPfck	Rif. cmb	wR	wF	wP	Rif. cmb	dR	dF	dP	Rif. cmb
	75.4	0.02	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0				
130	0.0	0.33	0.38	0.28	2,2,4	0.09	0.07	0.05	2,3,4	-0.25	-0.16	-0.13	2,3,4
	30.7	0.31	0.37	0.27	2,2,4	0.08	0.06	0.05	2,3,4				
	61.4	0.26	0.30	0.22	2,2,4	0.06	0.05	0.0	2,3,0				
131	0.0	3.60e-03	3.75e-03	3.90e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.36e-03	-1.20e-03	-1.13e-03	2,3,4
	19.3	6.93e-03	7.23e-03	7.64e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	38.5	7.85e-03	8.18e-03	8.72e-03	2,2,4	0.0	0.0	0.0	0,0,0				
132	0.0	0.01	0.01	9.51e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.95e-03	-1.44e-03	-1.24e-03	2,3,4
	11.9	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
	23.8	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
133	0.0	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0	-0.02	-0.02	-0.02	2,3,4
	37.7	0.03	0.03	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
	75.4	0.03	0.04	0.04	2,2,4	0.0	0.0	0.0	0,0,0				
134	0.0	0.25	0.30	0.22	2,2,4	0.06	0.05	0.0	2,3,0	-0.25	-0.16	-0.13	2,3,4
	30.7	0.31	0.36	0.27	2,2,4	0.08	0.06	0.05	2,3,4				
	61.4	0.33	0.38	0.28	2,2,4	0.09	0.07	0.05	2,3,4				
135	0.0	2.90e-03	3.03e-03	3.32e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.36e-03	-1.20e-03	-1.13e-03	2,3,4
	19.3	2.71e-03	2.83e-03	2.96e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	38.5	0.01	0.01	0.01	2,2,4	0.0	0.0	0.0	0,0,0				
136	0.0	0.01	0.01	9.27e-03	2,2,4	0.0	0.0	0.0	0,0,0	-1.95e-03	-1.44e-03	-1.24e-03	2,3,4
	11.9	4.74e-03	4.94e-03	4.01e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	23.8	0.03	0.03	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
137	0.0	0.02	0.03	0.03	2,2,4	0.0	0.0	0.0	0,0,0	-0.02	-0.02	-0.02	2,3,4
	37.7	6.90e-03	8.14e-03	8.28e-03	2,2,4	0.0	0.0	0.0	0,0,0				
	75.4	0.02	0.02	0.02	2,2,4	0.0	0.0	0.0	0,0,0				
138	0.0	0.26	0.30	0.22	2,2,4	0.06	0.05	0.0	2,3,0	-0.25	-0.16	-0.13	2,3,4
	30.7	0.17	0.20	0.15	2,2,4	0.0	0.0	0.0	0,0,0				
	61.4	0.05	0.06	0.04	2,2,4	0.0	0.0	0.0	0,0,0				

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Programma TECNARIA per travi miste legno e calcestruzzo - Versione 4.32

Solai misti legno-calcestruzzo con connettori TECNARIA

Verifiche agli Stati Limite in accordo a:

- Norme Tecniche per le Costruzioni DM 17/01/2018
- Eurocodice 5 - Progettazione delle strutture di legno
- Approvazione Tecnica CSTB connettori Tecnaria
- Valutazione Tecnica Europea ETA-18/0649

Progetto : CAMPATA_540cm

Progettista : ing. Stefano Vantaggiato

I risultati di seguito descritti si ottengono esclusivamente con l'utilizzo dei connettori Tecnaria; ogni altro utilizzo del presente calcolo è da ritenersi del tutto inattendibile.

DATI

Solaio a semplice orditura con assito carotato
Trave puntellata o tirantata

GEOMETRIA

- Spessore soletta: 5 cm
- Spessore assito/pianelle/tavelle: 2 cm
- Spessore isolante: 0 cm
- Interasse travi: 60 cm
- Base travi: 14 cm
- Altezza travi: 16 cm
- Luce travi: 540 cm
- Limite freccia istantanea carichi variabili: $L/300 = 18.00$ mm
- Limite freccia attiva: 2: Finiture non fragili = 12.71 mm
- Limite freccia finale comb. quasi perm.: $L/200 = 27.00$ mm

CARICHI

- Permanenti strutturali: 1.54 kN/m²
- Permanenti non strutturali
- Carico iniziale 1: 0.00 kN/m²
- Carico iniziale 2: 1.00 kN/m²
- 1° carico fragile: 0.80 kN/m²
- Carichi successivi: 0.70 kN/m²
- Totale perm. non strutturali: 2.50 kN/m²
- Variabili: 2.00 kN/m²
- Carico SLU a metro lineare: 4.96 kN/m

MATERIALI

- Legno - Tipo : GL24 secondo EN 14080:2013
- Resistenza a flessione caratteristica $f_{m,k}$: 24.0 N/mm²
- k_h a flessione: 1.10
- Resistenza a trazione caratteristica $f_{t,0,k}$: 19.2 N/mm²
- k_h a trazione: 1.10
- Resistenza a taglio caratteristica $f_{v,k}$: 3.50 N/mm²
- Modulo di elasticità medio $E_{0,m}$: 11500 N/mm²
- Peso specifico medio ρ_m : 4.2 kN/m³
- Coeff. modificazione azioni variabili K_{mod} : 0.80
- Fattore di deformazione K_{def} : 0.60
- Riduzione larghezza per verifica a taglio k_{cr} : 0.67
- Coefficiente di sicurezza: 1.45
- Classe calcestruzzo: C25/30 - Rck30
- Resistenza caratteristica cilindrica $f_{c,k}$: 25.0 N/mm²
- Resistenza caratteristica a trazione $5\% f_{ctk}$: 1.8 N/mm²
- Modulo elasticità E: 30500 N/mm²
- Peso specifico ρ : 25.0 kN/m³
- Coefficiente di viscosità Φ : 2.50
- Coefficiente di sicurezza γ_m : 1.50
- Connettore: Tecnaria CTL BASE 12/ 60 posato su trave
- Resistenza caratteristica connettore F_k : 17200 N
- Rigidezza connettore in esercizio K_{ser} : 17900 N/mm
- Rigidezza connettore ultima K_u : 9990 N/mm
- Coefficiente di sicurezza γ_{mk} : 1.50
- Altri parametri
- Peso specifico assito/pianelle/tavelle: 4.20 kN/m³
- Peso specifico isolante: 0.25 kN/m³
- Coefficiente parziale carichi strutturali g_{G1} : 1.30
- Coefficiente parziale perm. non strut. g_{G2} : 1.30

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Coefficiente parziale carichi variabili gQ: 1.50
Coefficiente psi2 carichi quasi permanenti: 0.30
Appoggio del tavolato su trave: 2.0 cm
Resistenza di progetto armatura fyd: 391.3 N/mm²

RISULTATI

Connettori Tecnaria BASE 12/ 60 posati su trave con tavolato carotato
Connettori a spaziatura costante: 12.1 cm
Numero di connettori per trave: 45
Numero di connettori a m²: 13.89
Armatura longitudinale minima nel raccordo: 0.00 cm²/trave nella parte inferiore.
Armatura longitudinale minima nella soletta: 0.17 cm²/trave nella parte inferiore.
Armatura trasversale nella soletta: 0.72 cm²/m

VERIFICHE

Larghezza soletta collaborante: 60.0 cm
- Larghezza interruzione/carotaggio tavolato: 6.5 cm
STATO LIMITE ULTIMO
- Momento massimo: 18.06 kNm
Taglio massimo: 13.38 kN
- Verifiche a tempo zero
CLS - tensione max: 8.96 N/mm² <= 14.17 N/mm²
CLS - tensione min: -2.23 N/mm²
LEGNO - tensoflessione: 0.85 <= 1.00
LEGNO - taglio: 0.83 N/mm² <= 1.93 N/mm²
CONN. - taglio: 9057 N <= 9173 N
- Verifiche a tempo infinito
CLS - tensione max: 7.30 N/mm² <= 14.17 N/mm²
CLS - tensione min: -0.50 N/mm²
LEGNO - tensoflessione: 0.89 <= 1.00
LEGNO - taglio: 0.85 N/mm² <= 1.93 N/mm²
CONN. - taglio: 9143 N <= 9173 N
STATO LIMITE DI ESERCIZIO
EJ a tempo zero: 2891.5 kNm²
EJ a tempo infinito: 1838.7 kNm²
Freccia istantanea car. var.: 4.59 mm <= 18.00 mm
Freccia attiva: 12.37 mm <= 12.71 mm
Freccia a tempo infinito: 20.05 mm <= 27.00 mm

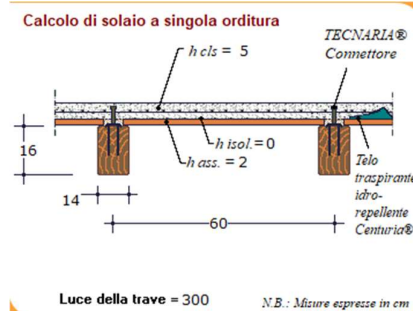
AVVERTENZE

- Oltre all'armatura trasversale indicata inserire armatura per il calcolo a flessione della soletta in direzione trasversale alle travi (almeno rete d6 20x20)
- Puntellare o tirantare il solaio prima del getto fino a completa maturazione del cls
- Si consiglia di collegare la soletta alle murature perimetrali mediante perforazioni armate utilizzando l'apposita resina epossidica bicomponente RTEC400.
- Dettagli di rinforzo sezioni: www.tecnaria.com/legno/dwg.htm

TECNARIA S.p.A. V.le Pecori Giraldi, 55 36061 Bassano del Grappa (VI)
tel 0424 502029 fax 0424 502386 info@tecnaria.com www.tecnaria.com

Come si può vedere dalle immagini sopra, ipotizzando un solaio in tecnica mista legno-calcestruzzo, composto da travetti in legno 14x16cm, posti ad un interasse di 60cm, distribuiti su una luce di 550cm massimo e unito da appositi connettori tipo TECNARIA BASE 12/ 60, posti a passo 12,1cm, alla cappa sovrastante di c.a. sp.5cm, le verifiche risultano soddisfatte.

SOLAIO LUCE MAX 300CM



Schematizzazione come trave in semplice appoggio:

DATI

Solaio a semplice orditura con assito carotato
Trave puntellata o tirantata

GEOMETRIA

- Spessore soletta: 5 cm
- Spessore assito/pianelle/tavelle: 2 cm
- Spessore isolante: 0 cm
- Interasse travi: 60 cm
- Base travi: 14 cm
- Altezza travi: 16 cm
- Luce travi: 300 cm
- Limite freccia istantanea carichi variabili: $L/300 = 10.00$ mm
- Limite freccia attiva: 2: Finiture non fragili = 8.97 mm
- Limite freccia finale comb. quasi perm.: $L/200 = 15.00$ mm

CARICHI

- Permanenti strutturali: 1.54 kN/m²
- Permanenti non strutturali
- Carico iniziale 1: 0.00 kN/m²
- Carico iniziale 2: 0.00 kN/m²
- 1° carico fragile: 0.80 kN/m²
- Carichi successivi: 0.70 kN/m²
- Totale perm. non strutturali: 1.50 kN/m²
- Variabili: 4 kN/m²
- Carico SLU a metro lineare: 5.98 kN/m

MATERIALI

- Legno - Tipo: GL24 secondo EN 14080:2013
- Resistenza a flessione caratteristica f_m, k : 24.0 N/mm²
- k_h a flessione: 1.10
- Resistenza a trazione caratteristica $f_{t,0}, k$: 19.2 N/mm²
- k_h a trazione: 1.10
- Resistenza a taglio caratteristica f_v, k : 3.50 N/mm²
- Modulo di elasticità medio E_0, m : 11500 N/mm²
- Peso specifico medio ρ_m : 4.2 kN/m³
- Coeff. modificazione azioni variabili K_{mod} : 0.80
- Fattore di deformazione K_{def} : 0.60
- Riduzione larghezza per verifica a taglio k_{cr} : 0.67
- Coefficiente di sicurezza: 1.45
- Classe calcestruzzo: C25/30 - Rck30
- Resistenza caratteristica cilindrica $f_{c,k}$: 25.0 N/mm²
- Resistenza caratteristica a trazione 5% f_{ctk} : 1.8 N/mm²
- Modulo elasticità E : 30500 N/mm²
- Peso specifico ρ : 25.0 kN/m³
- Coefficiente di viscosità ϕ : 2.50
- Coefficiente di sicurezza γ_m : 1.50
- Connettore: Tecnaria CTL BASE 12/ 60 posato su trave
- Resistenza caratteristica connettore F_k : 17200 N
- Rigidezza connettore in esercizio K_{ser} : 17900 N/mm
- Rigidezza connettore ultima K_u : 9990 N/mm
- Coefficiente di sicurezza γ_{mk} : 1.50
- Altri parametri
- Peso specifico assito/pianelle/tavelle: 4.20 kN/m³
- Peso specifico isolante: 0.25 kN/m³
- Coefficiente parziale carichi strutturali g_{G1} : 1.30

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Coefficiente parziale perm. non strut. gG2: 1.30
 Coefficiente parziale carichi variabili gQ: 1.50
 Coefficiente psi2 carichi quasi permanenti: 0.60
 Appoggio del tavolato su trave: 2.0 cm
 Resistenza di progetto armatura fyd: 391.3 N/mm²

RISULTATI

Connettori Tecnaria BASE 12/ 60 posati su trave con tavolato carotato
 Connettori a spaziatura costante: 40.0 cm
 Numero di connettori per trave: 8
 Numero di connettori a m²: 4.44
 Armatura longitudinale minima nel raccordo: 0.00 cm²/trave nella parte inferiore.
 Armatura longitudinale minima nella soletta: 0.73 cm²/trave nella parte inferiore.
 Armatura trasversale nella soletta: 0.44 cm²/m

VERIFICHE

Larghezza soletta collaborante: 60.0 cm
 - Larghezza interruzione/carotaggio tavolato: 6.5 cm
 STATO LIMITE ULTIMO
 - Momento massimo: 6.72 kNm
 Taglio massimo: 8.96 kN
 - Verifiche a tempo zero
 CLS - tensione max: 5.38 N/mm² ≤ 14.17 N/mm²
 CLS - tensione min: -4.30 N/mm²
 LEGNO - tensoflessione: 0.46 ≤ 1.00
 LEGNO - taglio: 0.59 N/mm² ≤ 1.93 N/mm²
 CONN. - taglio: 8650 N ≤ 9173 N
 - Verifiche a tempo infinito
 CLS - tensione max: 3.90 N/mm² ≤ 14.17 N/mm²
 CLS - tensione min: -2.75 N/mm²
 LEGNO - tensoflessione: 0.50 ≤ 1.00
 LEGNO - taglio: 0.63 N/mm² ≤ 1.93 N/mm²
 CONN. - taglio: 9171 N ≤ 9173 N
 STATO LIMITE DI ESERCIZIO
 EJ a tempo zero: 1270.2 kNm²
 EJ a tempo infinito: 715.2 kNm²
 Freccia istantanea car. var.: 1.99 mm ≤ 10.00 mm
 Freccia attiva: 3.65 mm ≤ 8.57 mm
 Freccia a tempo infinito: 4.82 mm ≤ 15.00 mm

AVVERTENZE

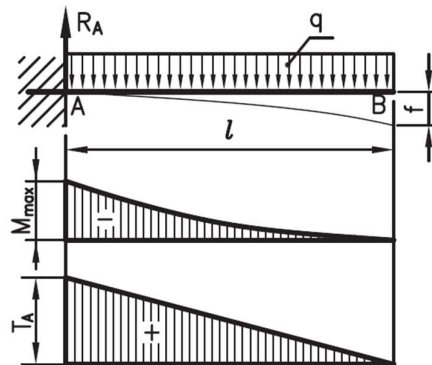
- Oltre all'armatura trasversale indicata inserire armatura per il calcolo a flessione della soletta in direzione trasversale alle travi (almeno rete d6 20x20)
- Puntellare o tirantare il solaio prima del getto fino a completa maturazione del cls
- Si consiglia di collegare la soletta alle murature perimetrali mediante perforazioni armate utilizzando l'apposita resina epossidica bicomponente RTEC400.
- Dettagli di rinforzo sezioni: www.tecnaria.com/legno/dwg.htm

TECNARIA S.p.A. V.le Pecori Giraldi, 55 36061 Bassano del Grappa (VI)
 tel 0424 502029 fax 0424 502386 info@tecnaria.com www.tecnaria.com

Come si può vedere dalle immagini sopra, ipotizzando un solaio in tecnica mista legno-calcestruzzo, composto da travetti in legno 14x16cm, posti ad un interasse di 60cm, distribuiti su una luce di 360cm massimo e unito da appositi connettori tipo TECNARIA BASE 12/ 60, posti a passo 40,0cm, alla cappa sovrastante di c.a. sp.5cm, le verifiche risultano soddisfatte.

SOLAIO IN AGGETTO

Per il caso specifico si considera la sola resistenza dei travetti in aggetto:



$$G_1 = 154,00 \text{ daN/mq}$$

$$G_2 = 150,00 \text{ daN/mq}$$

$$Q_k = 400,00 \text{ daN/mq}$$

$$\text{Int.} = 0,60 \text{ m}$$

$$\text{Luce} = 1,05 \text{ m}$$

$$q_{\text{SLE,QP}} = 2,54 \text{ kN/m}$$

$$q_{\text{SLU}} = 4,22 \text{ kN/m}$$

$$q_{\text{SLU}} = 5,97 \text{ kN/m}$$

$$V_{\text{Ed,SLE,QP}} = 2,67 \text{ kN}$$

$$M_{\text{Ed,SLE,QP}} = 1,40 \text{ kN.m}$$

$$V_{\text{Ed,SLE,R}} = 4,43 \text{ kN}$$

$$M_{\text{Ed,SLE,R}} = 2,33 \text{ kN.m}$$

$$V_{\text{Ed,SLU}} = 6,27 \text{ kN}$$

$$M_{\text{Ed,SLU}} = 3,29 \text{ kN.m}$$

Verifica a flessione semplice

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

$$\sigma_{mx,Ed} = W_x / M_{Ed} = 5,51 \text{ N/mm}^2$$

$$f_{mx,d} = f_{fless,k} \times K_{mod} \times k_{hx} / \gamma_M = 14,57 \text{ N/mm}^2$$

$$K_{mod} = 0,8 \text{ (media durata)}$$

$$f_{fless,x} = 24,0 \text{ N/mm}^2$$

$$\sigma_{mx,Ed} / f_{mx,d} = 0,38 \rightarrow \text{OK}$$

Verifica a taglio

$$T_{d,x} = (3 \times V_{Ed}) / (2 \times b_{ef} \times H) = 0,59 \text{ N/mm}^2$$

$$f_{v,d} = (f_{taglio,k} \times K_{mod}) / \gamma_M = 1,93 \text{ N/mm}^2$$

$$b_{ef} = b \times K_{cr} = 100 \text{ mm}$$

$$K_{cr} = 2,50 \times f_{taglio,k} = 0,71$$

$$T_{d,x} / f_{v,d} = 0,30 \rightarrow \text{OK}$$

Verifica delle deformate

Freccia istantanea – carichi permanenti

$$Q_{(G1,2)} = 1,82 \text{ kN/m}$$

$$f_{(G1,2)V} = 0,08 \text{ mm}$$

$$f_{(G1,2)M} = 0,84 \text{ mm}$$

$$f_{ist,1} = 0,92 \text{ mm}$$

Freccia istantanea – carichi accidentali

$$Q_{(Qk)} = 2,40 \text{ kN/m}$$

$$f_{(Qk)V} = 0,11 \text{ mm}$$

$$f_{(Qk)M} = 1,11 \text{ mm}$$

$$f_{ist,2,1} = 1,21 \text{ mm}$$

Freccia differita – carichi permanenti

$$Q_{(G1,2)} = 1,82 \text{ kN/m}$$

$$f_{(G1,2)V} = 0,08 \text{ mm}$$

$$f_{(G1,2)M} = 0,84 \text{ mm}$$

$$f_{diff,1} = 0,55 \text{ mm}$$

Freccia differita – carichi accidentali

$$Q_{(Q_k)} = 0,72 \text{ kN/m}$$

$$f_{(Q_k)V} = 0,03 \text{ mm}$$

$$f_{(Q_k)M} = 0,33 \text{ mm}$$

$$f_{\text{diff},2,1} = 0,22 \text{ mm}$$

Freccia finale per soli carichi variabili

$$f_{2,\text{fin}} = f_{\text{ist},2,1} + f_{\text{diff},2,1} = 1,43 \text{ mm} < L/200$$

Freccia massima finale complessiva

$$f_{\text{net},\text{fin}} = f_{\text{ist},1} + f_{\text{diff},1} = 2,91 \text{ mm} < L/250$$

VERIFICA DEL SISTEMA DI CONNESSIONE

Le travi in legno vengono collegate ai cordoli perimetrali per mezzo di connessioni a piastra in acciaio bullonate e ancoranti chimici.

In particolare, nel solaio di copertura di piano terra si possono individuare due diverse combinazioni di carico:

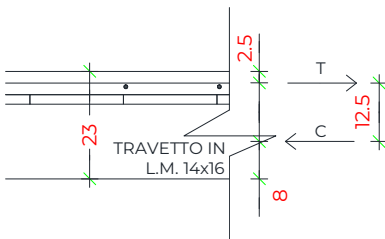
La prima deriva da un solaio in solo appoggio, per cui si avrà una sola azione di taglio:

$$V_{\text{Ed,SLU},1} = 13,38 \text{ kN}$$

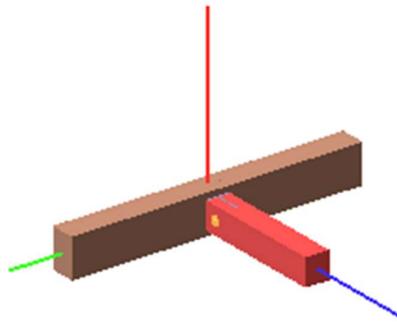
La seconda, deriva da una porzione di solaio in aggetto, per cui si avrà azione di taglio e momento flettente:

$$V_{\text{Ed,SLU},2} = 6,27 \text{ kN}$$

$M_{\text{Ed,SLU},2} = 3,29 \text{ kN.m}$ → la flessione può essere scomposta in una coppia di forze uguali e contrarie distanti un braccio “b”, di cui la trazione verrà gestita posizionando apposite barre longitudinali all’interno della cappa in c.a., mentre la compressione andrà a sfogare sul travetto stesso.



$$N_{Ed,SLU,2} = M/b = 26,32 \text{ kN}$$



Classe di servizio 2

L'opera è caratterizzata da un'umidità del materiale in equilibrio con l'ambiente a una temperatura di 20°C e un'umidità relativa dell'aria circostante che supera l'85% solo per poche settimane all'anno.

Coefficiente di sicurezza utilizzato

$$Y_M = 1,50$$

Trave lato 3+

Dimensioni sezione ($B_{el} \times H_{el}$):.....140x160mm

Legno: GL24h – UNI EN 14080:2013

Essenza: conifere

Massa volumica caratteristica:..... $\rho_k = 385,00 \text{ daN/mc}$

Massa volumica media:..... $\rho_m = 420,00 \text{ daN/mc}$

Resistenza caratteristica a trazione parallela alle fibre:..... $f_{t,0,k} = 19,20 \text{ N/mm}^2$

Resistenza caratteristica a trazione ortogonale alle fibre:..... $f_{t,90,k} = 0,50 \text{ N/mm}^2$

Resistenza caratteristica a compressione parallela alle fibre:..... $f_{c,0,k} = 24,00 \text{ N/mm}^2$

Resistenza caratteristica a compressione ortogonale alle fibre:..... $f_{c,90,k} = 2,50 \text{ N/mm}^2$

Resistenza caratteristica a taglio:..... $f_{v,k} = 3,50 \text{ N/mm}^2$

Resistenza caratteristica a flessione:..... $f_{m,k} = 24,00 \text{ N/mm}^2$

Coefficiente correttivo K_{mod} :

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Classe durata carico	permanente	lunga	media	breve	istantanea
Classe di servizio 1	0.60	0.70	0.80	0.90	1.10
Classe di servizio 2	0.60	0.70	0.80	0.90	1.10
Classe di servizio 3	0.50	0.55	0.65	0.70	0.90

Dati unione

Unione realizzata con l'utilizzo di una staffa d'acciaio a T con l'ala fissata alla trave di bordo in c.a. per mezzo di connettori chimici e l'anima inserita nel travetto:

La staffa ha dimensioni ($S_s \times H_s \times L_{anima} \times B_{ala}$) 10,00x160,00x140,00x140,00mm.

Materiale staffa: Acciaio S275

Tensione caratteristica di snervamento:..... $f_{yk} = 275,00 \text{ N/mm}^2$

Tensione caratteristica di rottura:..... $f_{tk} = 430,00 \text{ N/mm}^2$

Dati connettore travetti-staffa

Bullone: M22

Diametro:..... $\varnothing = 22,00 \text{ mm}$

Limite "Johanson" per E_{fune} :..... $L_{EF} = 25\%$

Numero :..... $n = 1$ (1 righe e 1 colonne)

Diametro rondella:..... $\varnothing_r = 60,00 \text{ mm}$

Materiale: Classe 8.8 (NTC18/EC3)

Tensione di snervamento:..... $f_{yb} = 640,00 \text{ N/mm}^2$

Tensione di rottura:..... $f_{tb} = 800,00 \text{ N/mm}^2$

Dati connettori staffa-cordolo in c.a.

Bullone: M12 e ancorante chimico tipo WIT-UH 300 WURTH.

Diametro:..... $\varnothing = 12,00 \text{ mm}$

Numero :..... $n = 4$ (2 righe e 2 colonne)

Materiale: Classe 8.8 (NTC18/EC3)

Tensione di snervamento:..... $f_{yb} = 640,00 \text{ N/mm}^2$

Tensione di rottura:..... $f_{tb} = 800,00 \text{ N/mm}^2$

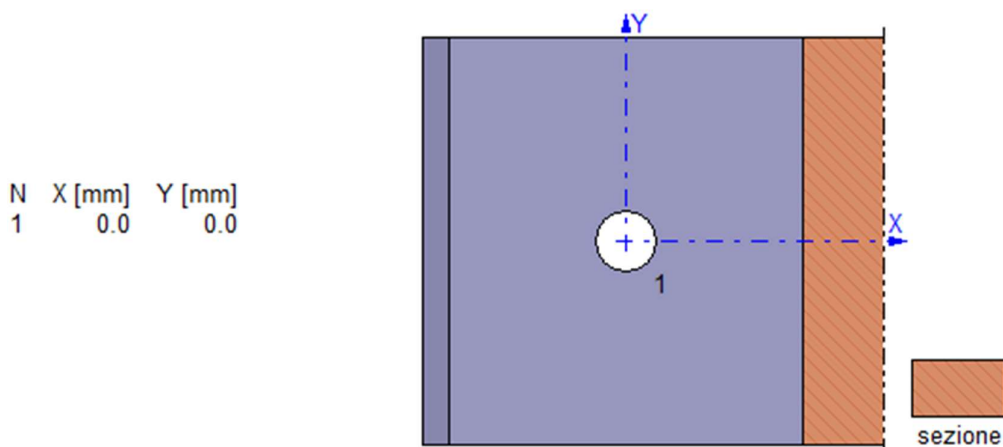
INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Sollecitazioni nella sezione d'attacco dell'elemento:

N.C.D.	V2 [N]	V3 [N]	N [N]	M2 [N.mm]	M3[N.mm]	T[N.mm]
1.1.M	13.380,00	0,00	0,00	0,00	0,00	0,00
1.2.M	6.270,00	0,00	-26.320,00	0,00	0,00	0,00

Nota: la prima colonna della tabella riporta il numero del nodo (N), il numero della combinazione © e l'iniziale della classe di durata del carico (D: Permanente; Lunga durata; Media durata; Breve durata; Istantanea).

Verifica unione travetto-staffa a T



Verifica "lato legno" (Nodo n.1, CMB n.1)

Capacità caratteristica a estrazione del bullone: $F_{ax,Rk} = F_{cr,Rd} = 17.952,70 \text{ N}$

$F_{cr,Rd} = 3 \times f_{c,90ck} \times (\phi_r^2 - \phi_f^2) \times \pi / 4$ capacità di carico rondella

$\phi_f = 23,50 \text{ mm}$ diametro foro

Momento caratteristico di snervamento: $M_{y,Rk} = 0,30 \times t_{fb} \times \phi^{2,6} = 742.181,60 \text{ N.mm}$

Resistenza caratteristica a rifollamento per. alle fibre:

$f_{h,o,k} = 0,082 \times (1 - 0,01 \times \phi) \times \rho_k = 24,62 \text{ N/mm}^2$

Coefficiente di essenza legnosa: $k_{90} = 1,35 + 0,015 \times \phi = 1,68$

Angolo di inclinazione del carico rispetto alle fibre: $\alpha = 13,40^\circ$

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Resistenza caratteristica a rifollamento secondo α :

$$f_{h,\alpha,k} = f_{h,0,k} / (k_{90} \times \sin^2 \alpha + \cos^2 \alpha) = 23,76 \text{ N/mm}^2$$

Equazione di Johansen: piastra di qualunque spessore elemento centrale di una connessione a doppio taglio.

Capacità di carico per piano di taglio: $F_{v,Rk} = \min [F_{v,Rk,f}; F_{v,Rk,g}; F_{v,Rk,h}] = 32.643,60 \text{ N}$

$$F_{v,Rk,f} = f_{h,\alpha,k} \times t \times \emptyset = 33.972,60 \text{ N}$$

$$F_{v,Rk,g} = f_{h,\alpha,k} \times t \times \emptyset \times [(2 + 4 \times M_{y,Rk} / (f_{h,\alpha,k} \times \emptyset \times t^2))^{0.5} - 1] + E_{fune,g} = 32.643,60 \text{ N}$$

$$F_{v,Rk,h} = 2,3 \times (M_{y,Rk} \times f_{h,\alpha,k} \times \emptyset)^{0.5} + E_{fune,h} = 49.787,40 \text{ N}$$

$$E_{fune,g} = \min [L_{EF} \times F_{v,Rk,g}; F_{ax,Rk} / 4] = 4.488,20 \text{ N}$$

$$E_{fune,h} = \min [L_{EF} \times F_{v,Rk,h}; F_{ax,Rk} / 4] = 4.488,20 \text{ N}$$

Resistenza di progetto del bullone per piano di taglio: $F_{v,Rd} = K_{mod} \times F_{v,Rk} / Y_M = 17.409,90 \text{ N}$

Numero efficace bulloni per ogni gruppo par. alle fibre:

Carico parallelo alle fibre:..... $n_{ef,II} = n = 1,00$

Carico perpendicolare alle fibre:..... $n_{ef,T} = n = 1,00$

Carico reale (secondo α):..... $n_{ef,\alpha} = n_{ef,II} + (n_{ef,T} - n_{ef,II}) \times \alpha / 90 = 1,00$

Numero di gruppi par. alle fibre:..... $n_g = 1,00$

Resistenza di progetto del giunto per piano di taglio: $F_{v,G,Rd} = n_{ef,\alpha} \times n_g \times F_{v,Rd} = 17.409,90 \text{ N}$

Forza agente sul giunto per piano di taglio: $F_{v,Ed} = 13.528,30 \text{ N}$

>> $F_{v,Ed} / F_{v,G,Rd} = 0,78 \rightarrow \text{OK}$

Verifica "Lato acciaio" (Nodo n.1, CMB n.2)

Calcolo resistenze

Resistenza a taglio dei bulloni: $F_{vb,Rd} = 0,50 \times f_{tb} \times 2 \times A_{res} / Y_{M2} = 194.628,00 \text{ N}$

Connessione	$F_{b,x,Rd} \text{ [N]}$	$F_{v,x,Rd} \text{ [N]}$	$F_{b,y,Rd} \text{ [N]}$	$F_{v,y,Rd} \text{ [N]}$
1	174.439,70	174.439,70	189.200,00	189.200,00

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Legenda:

$F_{b,x,Rd} = k \times \alpha \times f_{tk} \times \emptyset \times S_s / Y_{M2} \rightarrow$ resistenza a rifollamento anima staffa in direzione x

$F_{v,x,Rd} = \min [F_{vb,Rd}; F_{b,x,Rd}] \rightarrow$ resistenza a taglio di progetto in direzione x

$F_{b,y,Rd} = k \times \alpha \times f_{tk} \times \emptyset \times S_s / Y_{M2} \rightarrow$ resistenza a rifollamento anima staffa in direzione y

$F_{v,y,Rd} = \min [F_{vb,Rd}; F_{b,y,Rd}] \rightarrow$ resistenza a taglio di progetto in direzione y

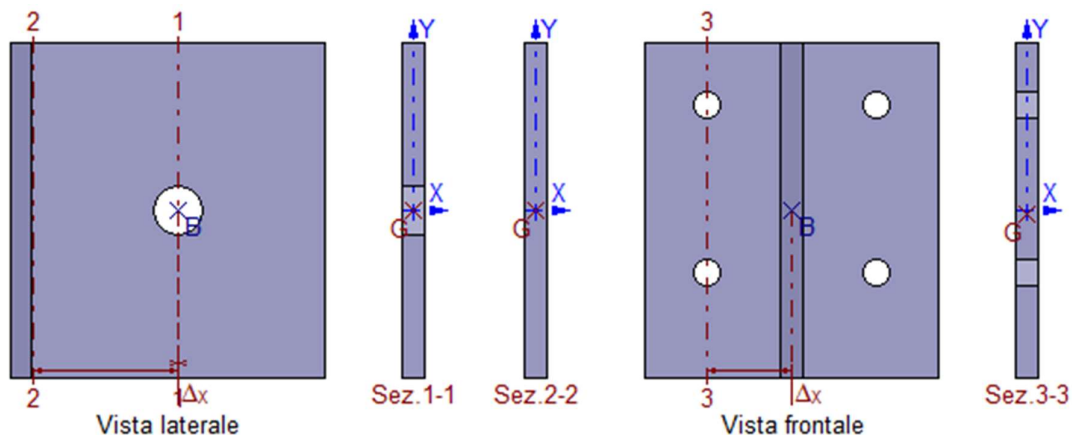
Verifica connettori

$F_{v,Ed} = 27.056,50 \text{ N}$

$F_{v,Rd} = 174.439,70 \text{ N}$

$\gg F_{v,Ed} / F_{v,Rd} = 0,16 \rightarrow \text{OK}$

Verifica staffa a T



Caratteristiche sezioni

Sez.	Δx	Y_G	X_G	A	A_{vy}	A_{vx}	J_{xG}	W_{xG}^*	J_{yG}	W_{yG}^*
	[mm]	[mm]	[mm]	[mm ²]	[mm ²]	[mm ²]	[mm ⁴]	[mm ³]	[mm ⁴]	[mm ³]
1-1	0	0	0	1.365	1.365	-	3.413.333	42.667	-	-
2-2	70	0	0	1.600	1.600	1.600	3.413.333	42.667	13.333	2.667
3-3	40	-1,94	0	1.340	1.340	1.340	2.966.289	36.201	11.167	2.233

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Sollecitazioni massime

Sez.	Nodo CMB	V_y [N]	V_x [N]	N [N]	M_y [N.mm]	M_x [N.mm]
1-1	1.2	-6.270	-	-26.320	-	0
2-2	1.2	-6.270	0	-26.320	0	438.900
3-3	1.2	-3.135	-13.160	0	526.400	-125.400

Tensioni massime

Sez.	τ_{MED} [N/mm ²]	σ_{MAX} [N/mm ²]	σ_{ID} [N/mm ²]	FV	VER
1-1	4,59	-19,28	20,86	0.08	OK
2-2	3,92	-26,74	27,58	0,11	OK
3-3	10,10	-239,17	239,80	0,92	OK

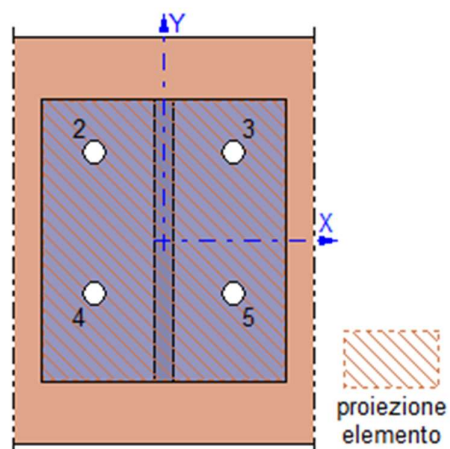
Legenda:

$$FV = \sigma_{ID} / f_d = (f_d = f_{yk} / \gamma_{MO} = 261,90 \text{ N/mm}^2)$$

$$VER \rightarrow FV \leq 1,00$$

Verifica staffa a T-cordolo c.a.

N	X [mm]	Y [mm]
2	-40.0	50.0
3	40.0	50.0
4	-40.0	-30.0
5	40.0	-30.0



INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Dati di ingresso

Materiale base	Calcestruzzo: fessurato C25/30; $f_{ck} = 25.00 \text{ N/mm}^2$, $f_{t,ck,ube} = 30.00 \text{ N/mm}^2$ $h = 150 \text{ mm}$ Intervallo di temperature: 40 °C / 24 °C (Utente) 40 °C / 24 °C (Progettazione)
Armatura	Armatura del calcestruzzo: Normale Armatura di bordo: Armatura di bordo dritta + armatura di sospensione a maglia stretta Armatura per limitare la fessurazione: Disponibile
Condizioni di installazione	Metodo di perforazione: Foro trapanato Modalità di perforazione: Asciutto Flessione dell'ancorante: No Pulizia: Pulizia ad aria compressa (CAC), vedere istruzioni di settaggio

Ancorante:

Art. Nr.	Descrizione	Ø [mm]	l [mm]	$t_{el,max}$ [mm]	VE [Pezzi]
Disponibile su richiesta	Barra filettata F-8.8 x 1000 - Lunghezza dello spezzone: 100 mm - Numero di spezzoni per ogni barra filettata venduta a metro: 9	M12	1000 mm	10 mm	1

Barre filettate commerciali standardizzate con:

- Materiali, dimensioni e proprietà meccaniche secondo la tabella a1 della certificazione della resina corrispettiva.
- Certificazione di controllo 3.1 secondo EN 10204:2004
- Marcatura della profondità di infissione.

Resina / adesivo:

Art. Nr.	Descrizione
5918 500 320	ancorante chimico WIT-UH 300 in cartuccia per iniezione da 320 ml
5918 500 420	ancorante chimico WIT-UH 300 in cartuccia per iniezione da 420 ml
5918 503 825	ancorante chimico WIT-UH 300 in cartuccia per iniezione da 825 ml
5918 504 280	ancorante chimico WIT-UH 300 in cartuccia per iniezione da 280 ml

Tipo e dimensione dell'ancorante WIT-UH 300 + Barra d'ancoraggio a metro/ F M12 selezionato

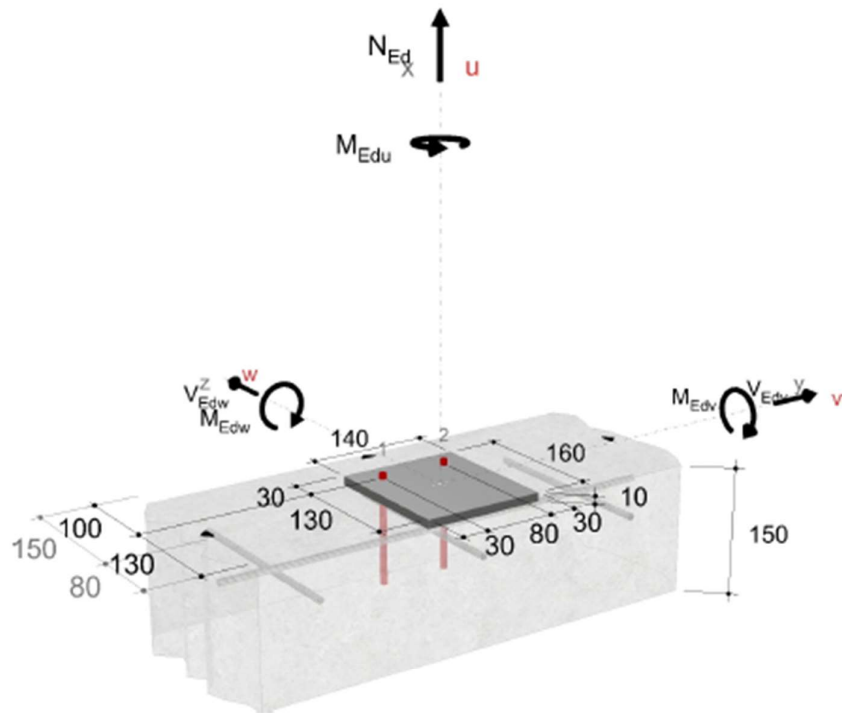
Materiale	F 8.8
Reale profondità di ancoraggio	75 mm



Geometria e sollecitazioni:

RELAZIONE DI CALCOLO

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA



Combinazioni di carico:

#	Nome	N_{Ed} [kN]	V_{Edv} [kN]	V_{Edw} [kN]	M_{Edu} [kNm]	M_{Edv} [kNm]	M_{Edw} [kNm]	Tipo di azione
1		-27.320	0.000	-6.270	0.000	0.440	0.000	Normale
2		0.000	0.000	-13.380	0.000	0.940	0.000	Normale

Verifiche

Forze di ancoraggio risultanti:

Numero dell'ancorante	$N_{Ed,x}^j$ [kN]	$(V_{Ed,y}^{Mx,j})$ [kN]	$(V_{Ed,z}^{Mx,j})$ [kN]	$(V_{Ed,y}^{Vy,j})$ [kN]	$(V_{Ed,z}^{Vy,j})$ [kN]	$V_{Ed,y}^j$ [kN]	$V_{Ed,z}^j$ [kN]	V_{Ed}^j [kN]
1	4.175	0.000	0.000	0.000	-6.690	0.000	-6.690	6.690
2	4.175	0.000	0.000	0.000	-6.690	0.000	-6.690	6.690

	$\Sigma N_{Ed,x}^j$ [kN]	$\Sigma (V_{Ed,y}^{Mx,j})$ [kN]	$\Sigma (V_{Ed,z}^{Mx,j})$ [kN]	$\Sigma (V_{Ed,y}^{Vy,j})$ [kN]	$\Sigma (V_{Ed,z}^{Vy,j})$ [kN]	$\Sigma V_{Ed,y}^j$ [kN]	$\Sigma V_{Ed,z}^j$ [kN]	$ \Sigma V_{Ed}^j $ [kN]
Totale	8.350	0.000	0.000	0.000	-13.380	0.000	-13.380	13.380

Riepilogo:

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Forza	Verifica	Sfruttamento	Condizione
Trazione	Cedimento lato acciaio	9.29 %	comprovato
Trazione	Cedimento combinato per sfilamento e rottura del calcestruzzo	47.83 %	comprovato
Trazione	Rottura del calcestruzzo	40.47 %	comprovato
Taglio	Cedimento lato acciaio, azioni di taglio senza braccio di leva	24.81 %	comprovato
Taglio	Scalzamento del calcestruzzo (Gruppo di ancoranti)	34.49 %	comprovato
Taglio	Rottura del bordo di calcestruzzo	68.17 %	comprovato
Combinazione Trazione/Taglio	Interazione (Cedimento lato acciaio)	7.02 %	comprovato
Combinazione Trazione/Taglio	Interazione (Escluso il cedimento lato acciaio)	89.37 %	comprovato

Calcolo dello spessore della piastra di ancoraggio.:

Non è stato eseguito il dimensionamento della piastra di ancoraggio. E' necessario che l'utente verifichi che la sua rigidità sia sufficiente.

Verifica effettuata con successo!

Osservazioni:

- metodo di verifica: EN1992-4
- Il presente documento è da considerarsi come una progettazione preliminare. Il dimensionamento e l'installazione dei mezzi di unione sono subordinati alla revisione e approvazione da parte del progettista responsabile delle strutture.
- Si prega di osservare le condizioni d'uso del software, in particolare il §4.
- Il numero di articolo degli ancoranti viene riportato nella relativa scheda prodotto.
- Il numero articolo degli accessori viene riportato nella relativa scheda informativa del prodotto. Le istruzioni di posa, invece, nella relativa certificazione.
- The results using the respective design methods are listed. For the details of the verifications please contact your certified structural engineer.
- Le verifiche dettagliate dello Stato Limite di Servizio non vengono riportate. Si raccomanda di richiederle ad un progettista abilitato.
- Il calcolo è valido solo se il foro passante non è più grande di quello indicato nella tabella 4.1 della EN 1992-4. In caso contrario, si veda il cap. 1.1 della EN 1992-4
- La progettazione si basa su numerosi parametri specifici dell'ancorante. Se l'ancorante scelto viene cambiato o se vengono modificati i valori di sollecitazione indicati o i dati geometrici, la verifica non è più valida e deve essere ripetuta con le nuove condizioni. E' necessario rispettare i requisiti e le indicazioni riportate nel Benestare Tecnico Europeo.
- In un gruppo di ancoranti devono essere utilizzati solo i tasselli dello stesso tipo, dimensione e lunghezza.
- La resistenza della resina varia in funzione della temperatura nel breve e lungo termine del materiale base.
- Verificare la resistenza del materiale base selezionato
- Il metodo di progettazione si applica a fissaggi sufficientemente rigidi da rendere effettiva la distribuzione lineare della deformazione.
- L'ipotesi che il fissaggio in esame sia sufficientemente rigido fa parte della sua valutazione tecnica.
- Se ci si discosta dall'ipotesi di piastra di ancoraggio rigida, le forze interne determinate vengono incrementate con un fattore di scale (forze realistiche sull'ancorante/forze lineari sull'ancorante) secondo la teoria dell'elasticità. Si prega di far controllare e verificare questo risultato da un ingegnere strutturista abilitato.
- Per ulteriori informazioni sulla piastra di ancoraggio rigida e sulla sua progettazione, si veda il documento del Prof. Ing. Jan Hofmann.
- La trasmissione dei carichi negli elementi strutturali deve essere verificata secondo EN 1992-4, cap. 7. Nel caso vi sia uno strato di malta di inghisaggio, si presume che non ci siano sacche d'aria sotto la piastra di ancoraggio, che lo strato sia stato preparato in anticipo e che sia completamente indurito.
- Il foro deve essere pulito con aria compressa (CAC) secondo ETA-17/0127.
- Non è stato eseguito il dimensionamento della piastra di ancoraggio. E' necessario che l'utente verifichi che la sua rigidità sia

Verifica dell'armatura aggiuntiva

Per assorbire completamente la flessione sviluppata dal nodo di connessione tra solaio in aggetto e trave in c.a. si predispongono barre di armatura aggiuntiva in ragione di n.1Ø10/ TRAVETTO.

$$N_{Ed,Traz} = 26,32 \text{ kN}$$

$$N_{Rd,traz} = f_{yd} \times A_{\phi 10} = 30,92 \text{ kN}$$

$$f_{yd} = 391,30 \text{ N/mm}^2$$

$$A_{\phi 10} = 79,00 \text{ mm}^2$$

$$N_{Ed,Traz} / N_{Rd,traz} = 0,85 < 1,00 \rightarrow \text{OK}$$

Verifica connettori

Per far collaborare compiutamente la soletta e i travetti in legno si è ipotizzato di posizionare n°3 profili tubolari 33,7x3mm saldati ad un piatto di base avente dimensioni 5x80x100mm. Quest'ultimo vincolato al travetto in legno sottostante, per mezzo di un piatto di dimensioni 5x100x130mm e n°1 bullone M16.

La configurazione del sistema ipotizzato si configura come una mensola, dove l'azione di trazione applicata in precedenza agisce come un taglio applicato ad una certa distanza dalla base del profilo:

$$N_{Ed,Traz} = V_{Ed} = 26,32 \text{ kN}$$

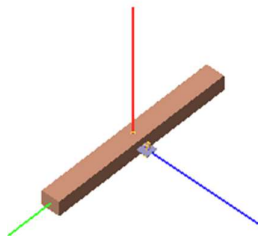
$$M_{Ed} = V_{Ed} \times b = 1,184 \text{ kN.m}$$

$$b = 4,50 \text{ cm}$$

Inoltre, si ipotizza che la sollecitazione si ripartisca equamente sui connettori in progetto:

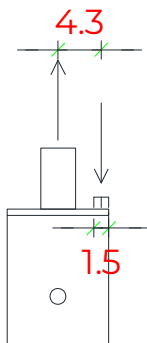
$$V_{Ed} = 26,32 / 3 = 8,77 \text{ kN}$$

$$M_{Ed} = 0,395 \text{ kN.m}$$



INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Il momento così calcolato può a sua volta essere scomposto in una coppia di forze uguale e contraria:



$$T = C = M_{Ed} / x = 9,19 \text{ kN}$$

$$x = 4,30 \text{ cm}$$

Classe di servizio 2

L'opera è caratterizzata da un'umidità del materiale in equilibrio con l'ambiente a una temperatura di 20°C e un'umidità relativa dell'aria circostante che supera l'85% solo per poche settimane all'anno.

Coefficiente di sicurezza utilizzato

$$Y_M = 1,50$$

Trave lato 3+

Dimensioni sezione ($B_{el} \times H_{el}$):.....100x80mm

Legno: GL24h – UNI EN 14080:2013

Essenza: conifere

Massa volumica caratteristica:..... $\rho_k = 385,00 \text{ daN/mc}$

Massa volumica media:..... $\rho_m = 420,00 \text{ daN/mc}$

Resistenza caratteristica a trazione parallela alle fibre:..... $f_{t,0,k} = 19,20 \text{ N/mm}^2$

Resistenza caratteristica a trazione ortogonale alle fibre:..... $f_{t,90,k} = 0,50 \text{ N/mm}^2$

Resistenza caratteristica a compressione parallela alle fibre:..... $f_{c,0,k} = 24,00 \text{ N/mm}^2$

Resistenza caratteristica a compressione ortogonale alle fibre:..... $f_{c,90,k} = 2,50 \text{ N/mm}^2$

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Resistenza caratteristica a taglio:..... $f_{v,k} = 3,50 \text{ N/mm}^2$

Resistenza caratteristica a flessione:..... $f_{m,k} = 24,00 \text{ N/mm}^2$

Coefficiente correttivo K_{mod} :

Classe durata carico	permanente	lunga	media	breve	istantanea
Classe di servizio 1	0.60	0.70	0.80	0.90	1.10
Classe di servizio 2	0.60	0.70	0.80	0.90	1.10
Classe di servizio 3	0.50	0.55	0.65	0.70	0.90

Dati unione

Unione realizzata con l'utilizzo di una piastra d'acciaio inserita nel legno nel lato dell'elemento continuo:

La piastra ha dimensioni ($S_p \times H_p \times L_p$) 5,00x100,00x130,00 ed è inserita nel travetto per 130,00mm.

Materiale staffa: Acciaio S275

Tensione caratteristica di snervamento:..... $f_{yk} = 275,00 \text{ N/mm}^2$

Tensione caratteristica di rottura:..... $f_{tk} = 430,00 \text{ N/mm}^2$

Dati connettore travetti-staffa

Bullone: M16

Diametro:..... $\emptyset = 16,00 \text{ mm}$

Limite "Johanson" per E_{fune} :..... $L_{EF} = 25\%$

Numero :..... $n = 1$ (1 righe e 1 colonne)

Diametro rondella:..... $\emptyset_r = 40,00 \text{ mm}$

Materiale: Classe 8.8 (NTC18/EC3)

Tensione di snervamento:..... $f_{yb} = 640,00 \text{ N/mm}^2$

Tensione di rottura:..... $f_{tb} = 800,00 \text{ N/mm}^2$

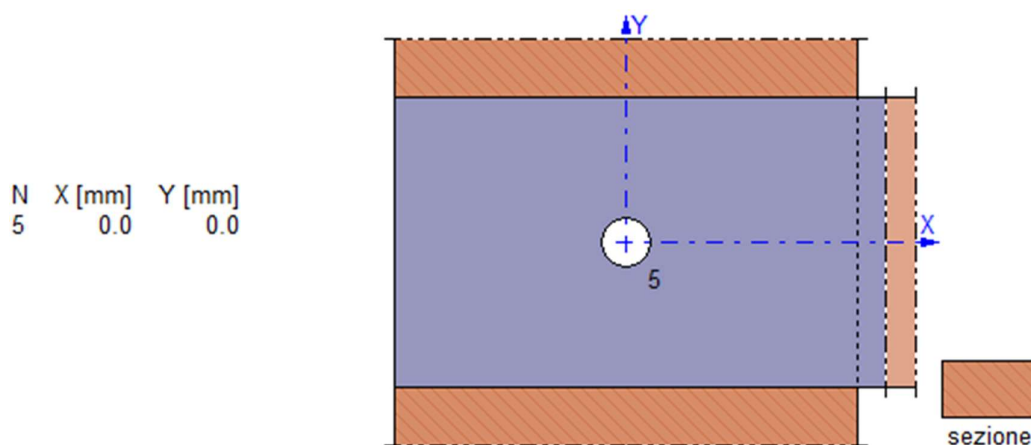
INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Sollecitazioni nella sezione d'attacco dell'elemento:

N.C.D.	V2 [N]	V3 [N]	N [N]	M2 [N.mm]	M3[N.mm]	T[N.mm]
1.1.M	0,00	8.770,0	9.190,00	0,00	0,00	0,00

Nota: la prima colonna della tabella riporta il numero del nodo (N), il numero della combinazione © e l'iniziale della classe di durata del carico (D: Permanente; Lunga durata; Media durata; Breve durata; Istantanea).

Verifica unione travetto-piastra



Verifica "lato legno" (Nodo n.1, CMB n.1)

Capacità caratteristica a estrazione del bullone: $F_{ax,Rk} = F_{cr,Rd} = 2.915,80 \text{ N}$

$F_{cr,Rd} = 3 \times f_{c,90ck} \times (\phi_r^2 - \phi_f^2) \times \pi / 4$ capacità di carico rondella

$\phi_f = 17,00 \text{ mm}$ diametro foro

Momento caratteristico di snervamento: $M_{y,Rk} = 0,30 \times t_{fb} \times \phi^{2,6} = 324.282,30 \text{ N.mm}$

Resistenza caratteristica a rifollamento per. alle fibre:

$f_{h,0,k} = 0,082 \times (1 - 0,01 \times \phi) \times \rho_k = 26,52 \text{ N/mm}^2$

Coefficiente di essenza legnosa: $k_{90} = 1,35 + 0,015 \times \phi = 1,59$

Angolo di inclinazione del carico rispetto alle fibre: $\alpha = 46,34^\circ$

Resistenza caratteristica a rifollamento secondo α :

$f_{h,\alpha,k} = f_{h,0,k} / (k_{90} \times \sin^2 \alpha + \cos^2 \alpha) = 20,26 \text{ N/mm}^2$

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

Equazione di Johansen: piastra di qualunque spessore elemento centrale di una connessione a doppio taglio.

Capacità di carico per piano di taglio: $F_{v,Rk} = \min [F_{v,Rk,f}; F_{v,Rk,g}; F_{v,Rk,h}] = 15.970,80 \text{ N}$

$$F_{v,Rk,f} = f_{h,\alpha,k} \times t \times \emptyset = 21.883,00 \text{ N}$$

$$F_{v,Rk,g} = f_{h,\alpha,k} \times t \times \emptyset \times [(2 + 4 \times M_{y,Rk} / (f_{h,\alpha,k} \times \emptyset \times t^2))^{0.5} - 1] + E_{fune,g} = 15.970,80 \text{ N}$$

$$F_{v,Rk,h} = 2,3 \times (M_{y,Rk} \times f_{h,\alpha,k} \times \emptyset)^{0.5} + E_{fune,h} = 24.311,50 \text{ N}$$

$$E_{fune,g} = \min [L_{EF} \times F_{v,Rk,g}; F_{ax,Rk} / 4] = 728,90 \text{ N}$$

$$E_{fune,h} = \min [L_{EF} \times F_{v,Rk,h}; F_{ax,Rk} / 4] = 728,90 \text{ N}$$

Resistenza di progetto del bullone per piano di taglio: $F_{v,Rd} = K_{mod} \times F_{v,Rk} / Y_M = 8.517,70 \text{ N}$

Numero efficace bulloni per ogni gruppo par. alle fibre:

Carico parallelo alle fibre:..... $n_{ef,II} = n = 1,00$

Carico perpendicolare alle fibre:..... $n_{ef,T} = n = 1,00$

Carico reale (secondo α):..... $n_{ef,\alpha} = n_{ef,II} + (n_{ef,T} - n_{ef,II}) \times \alpha / 90 = 1,00$

Numero di gruppi par. alle fibre:..... $n_g = 1,00$

Resistenza di progetto del giunto per piano di taglio: $F_{v,G,Rd} = n_{ef,\alpha} \times n_g \times F_{v,Rd} = 8.517,70 \text{ N}$

Forza agente sul giunto per piano di taglio: $F_{v,Ed} = 6.351,60 \text{ N}$

>> $F_{v,Ed} / F_{v,G,Rd} = 0,75 \rightarrow \text{OK}$

Verifica "Lato acciaio" (Nodo n.1, CMB n.1)

Calcolo resistenze

Resistenza a taglio dei bulloni: $F_{vb,Rd} = 0,50 \times f_{tk} \times 2 \times A_{res} / Y_{M2} = 100.370,10 \text{ N}$

Connessione	$F_{b,x,Rd} \text{ [N]}$	$F_{v,x,Rd} \text{ [N]}$	$F_{b,y,Rd} \text{ [N]}$	$F_{v,y,Rd} \text{ [N]}$
1	68.800,00	68.800,00	68.800,00	68.800,00

Legenda:

$F_{b,x,Rd} = k \times \alpha \times f_{tk} \times \emptyset \times S_s / Y_{M2} \rightarrow$ resistenza a rifollamento anima staffa in direzione x

$F_{v,x,Rd} = \min [F_{vb,Rd}; F_{b,x,Rd}] \rightarrow$ resistenza a taglio di progetto in direzione x

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

$F_{b,y,Rd} = k \times a \times f_{tk} \times \emptyset \times S_s / Y_{M2} \rightarrow$ resistenza a rifollamento anima staffa in direzione y

$F_{v,y,Rd} = \min [F_{vb,Rd}; F_{b,y,Rd}] \rightarrow$ resistenza a taglio di progetto in direzione y

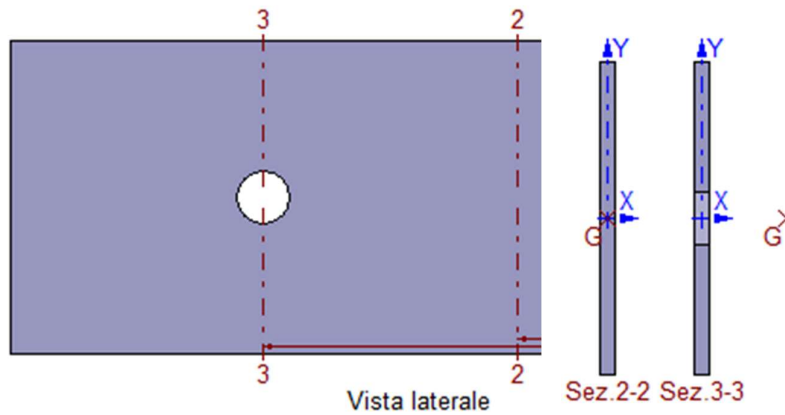
Verifica connettori

$F_{v,Ed} = 12.703,10 \text{ N}$

$F_{v,Rd} = 68.800,00 \text{ N}$

$\gg F_{v,Ed} / F_{v,Rd} = 0,19 \rightarrow \text{OK}$

Verifica staffa a T



Caratteristiche sezioni

Sez.	Δ_x	Y_G	X_G	A	A_{wy}	A_{vx}	J_{xG}	W_{xG}^*	J_{yG}	W_{yG}^*
	[mm]	[mm]	[mm]	[mm ²]	[mm ²]	[mm ²]	[mm ⁴]	[mm ³]	[mm ⁴]	[mm ³]
1-1	-	-	-	-	-	-	-	-	-	-
2-2	50	0	0	500	500	500	416.667	8.333	1.042	417
3-3	130	0	26.79	415	415	-	416.667	8.333	-	-

Sollecitazioni massime

Sez.	Nodo CMB	V_y [N]	V_x [N]	N [N]	M_y [N.mm]	M_x [N.mm]
1-1	1.1	-	-	-	-	-

INTERVENTI LOCALI SU FABBRICATO ESISTENTE A MOLLIA

2-2	1.1	8.770	0	9.190	0	-438.500
3-3	1.1	8.770	-	9.190	-	-1.140.100

Tensioni massime

Sez.	τ_{MED} [N/mm ²]	σ_{MAX} [N/mm ²]	σ_{ID} [N/mm ²]	FV	VER
1-1	-	-	-	-	-
2-2	17,54	71,00	77,23	0,29	OK
3-3	21,13	158,96	163,12	0,62	OK

Legenda:

$$FV = \sigma_{ID} / f_d = (f_d = f_{yk} / \gamma_{M0} = 261,90 \text{ N/mm}^2)$$

$$VER \rightarrow FV \leq 1,00$$

Verifica dei profili tubolari

Sollecitazioni agenti:

$$V_{Ed} = 26,32 / 3 = 8,77 \text{ kN}$$

$$M_{Ed} = 1,184 / 3 = 0,40 \text{ kN}$$

Verifica di resistenza a taglio:

$$V_{c,Rd} = \frac{A_v \cdot f_{yk}}{\sqrt{3} \cdot \gamma_{M0}} ; A_v = 2A / \pi$$

$$V_{Rd} = 27,87 \text{ kN}$$

$$A = 2,89 \text{ cm}^2$$

$$f_{yk} = 275,00 \text{ N/mm}^2$$

$$A_v = 184,08 \text{ mm}^2$$

$$V_{Ed} / V_{Rd} = 0,31 < 1,00 \rightarrow \text{OK}$$

Verifica di resistenza a flessione:

$$M_{el,Rd} = \frac{W_{el,min} \cdot f_{yk}}{\gamma_{M0}}$$

$$M_{Rd} = 0,53 \text{ kN.m}$$

$$W_{el} = 2,04 \text{ cmc}$$

$$f_{yk} = 275,00 \text{ N/mm}^2$$

$$M_{Ed} / M_{Rd} = 0,75 < 1,00 \rightarrow \text{OK}$$

Verifica a compressione legno-piastra

La coppia di forze generata dal momento flettente della connessione tra connettori e travetto:

$$T = C = 9,19 \text{ kN} = 9.190,00 \text{ N}$$

Sviluppa una pressione tra le due superfici di contatto della piastra in acciaio e del travetto in legno, in particolare:

$$\sigma_{MAX} = C / (L_1 \times L_2) = 7,66 \text{ N/mm}^2$$

$$L_1 = 80,00 \text{ mm}$$

$$L_2 = 15,00 \text{ mm (superficie in compressione della piastra soggetta a momento)}$$

$$\sigma_{amm,acc} = 261,90 \text{ N/mm}^2$$

$$\sigma_{amm,legno} = 14,57 \text{ N/mm}^2$$

$$\sigma_{MAX} < \sigma_{amm,acc} \rightarrow \text{OK}$$

$$< \sigma_{amm,legno} \rightarrow \text{OK}$$

CONCLUSIONI

Il sottoscritto Ing. Stefano Vantaggiato, iscritto all'albo degli Ingegneri della Provincia di Milano al n° 31571, quale progettista delle opere strutturali

DICHIARA

che tutte le opere strutturali sono state calcolate e progettate a norma della Scienza delle Costruzioni ed in osservanza delle vigenti disposizioni di Legge.

Dichiara inoltre che tutti gli elaborati allegati sono sufficienti per individuare i lavori da eseguirsi e che i materiali di cui si prevede l'impiego, nonché le relative dosature, sono idonei in relazione alle sollecitazioni assunte a base del calcolo.

Luogo e data

Milano, Luglio 2024

Il Tecnico

Dott. Ing. Stefano Vantaggiato

